## APPENDIX MR-2022

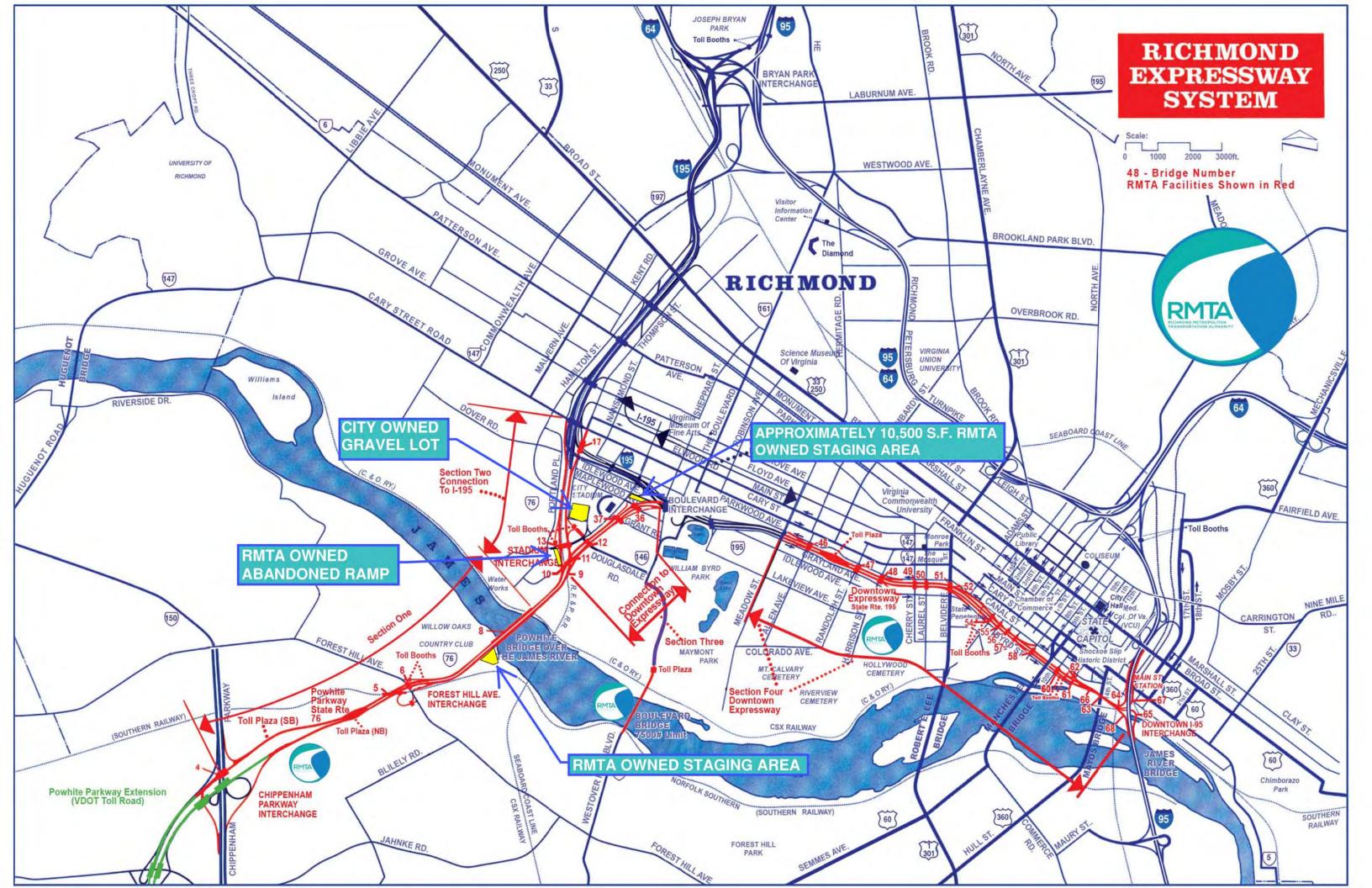
### **RECORD DRAWINGS**

### RICHMOND METROPOLITAN TRANSPORTATION AUTHORITY (RMTA)

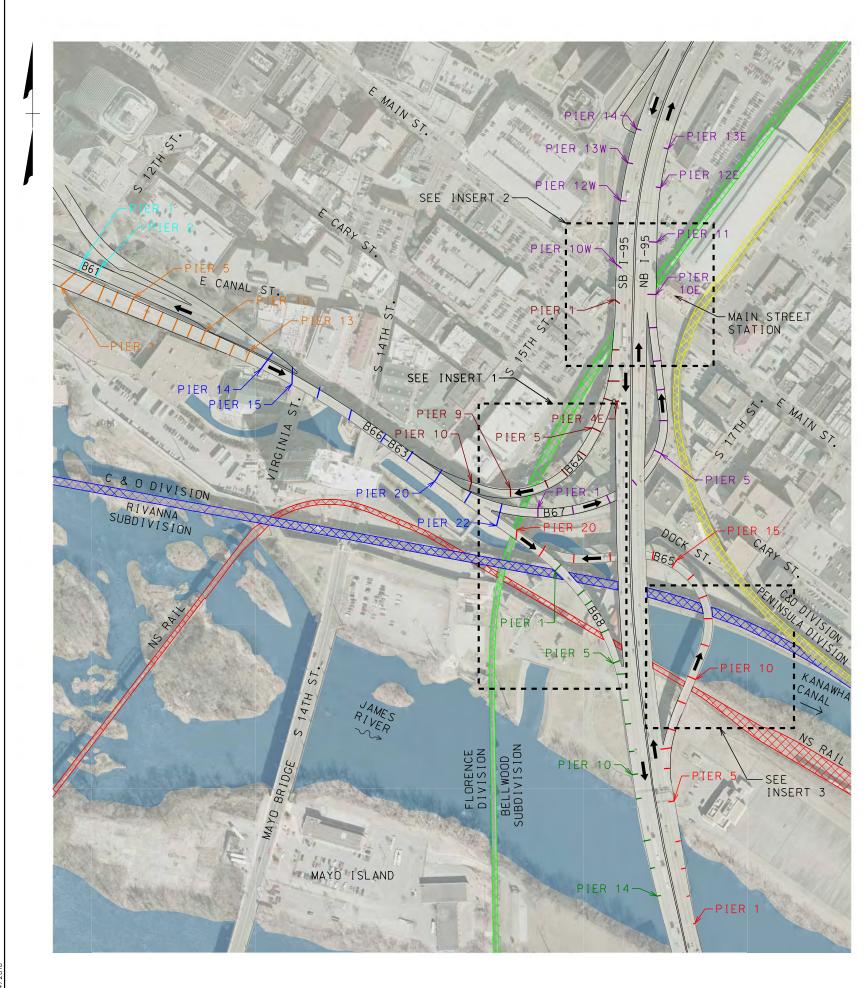
RMTA BRIDGES 6, 13, 48, 55 RMTA Box Culverts 1827 and 1831

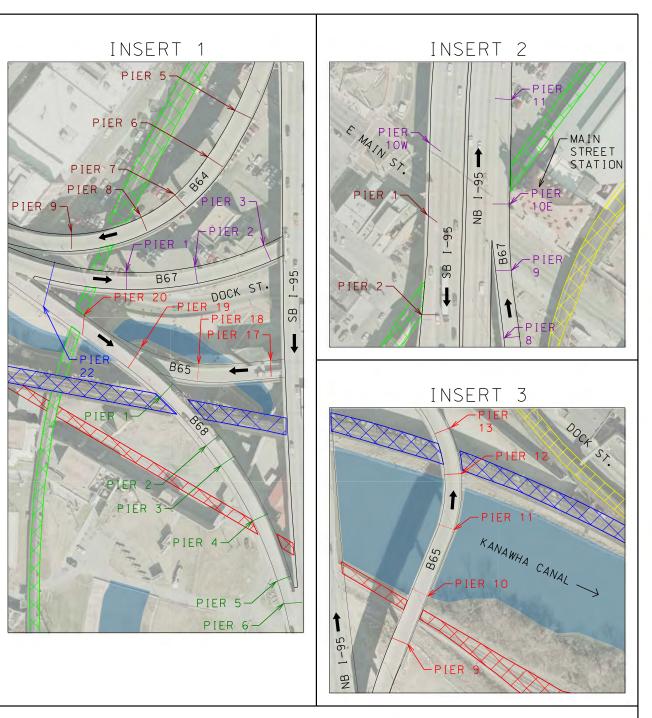
(NOTE: Additional sheets of the As-built Plans are Available upon Request to the Engineer)

## RMTA System Map



## **I-95 Ramps Pier Location Exhibit**







Pier Map-DR2016.dg 3 / / 2016 NOTES:

- 1) PIER NUMBERS BASED ON AS-BUILT DRAWINGS FROM CONTRACTS C-10 AND C-11.
- 2) RAILROAD LIMITS AND PIER LOCATIONS BASED ON AERIAL PHOTOGRAPHY.
- 3) THIS EXHIBIT IS FOR REFERENCE ONLY. REFER TO AS-BUILT DRAWINGS FOR EXACT PIER LOCATIONS.
- 4) BRIDGE 63 IS ON BOTTOM, BRIDGE 66 IS ON TOP.

 $R {\rm ichmond}\, M {\rm etropolitan}\, T {\rm ransportation}\, A {\rm uthority}$ 



#### I-95 RAMPS PIER LOCATION EXHIBIT

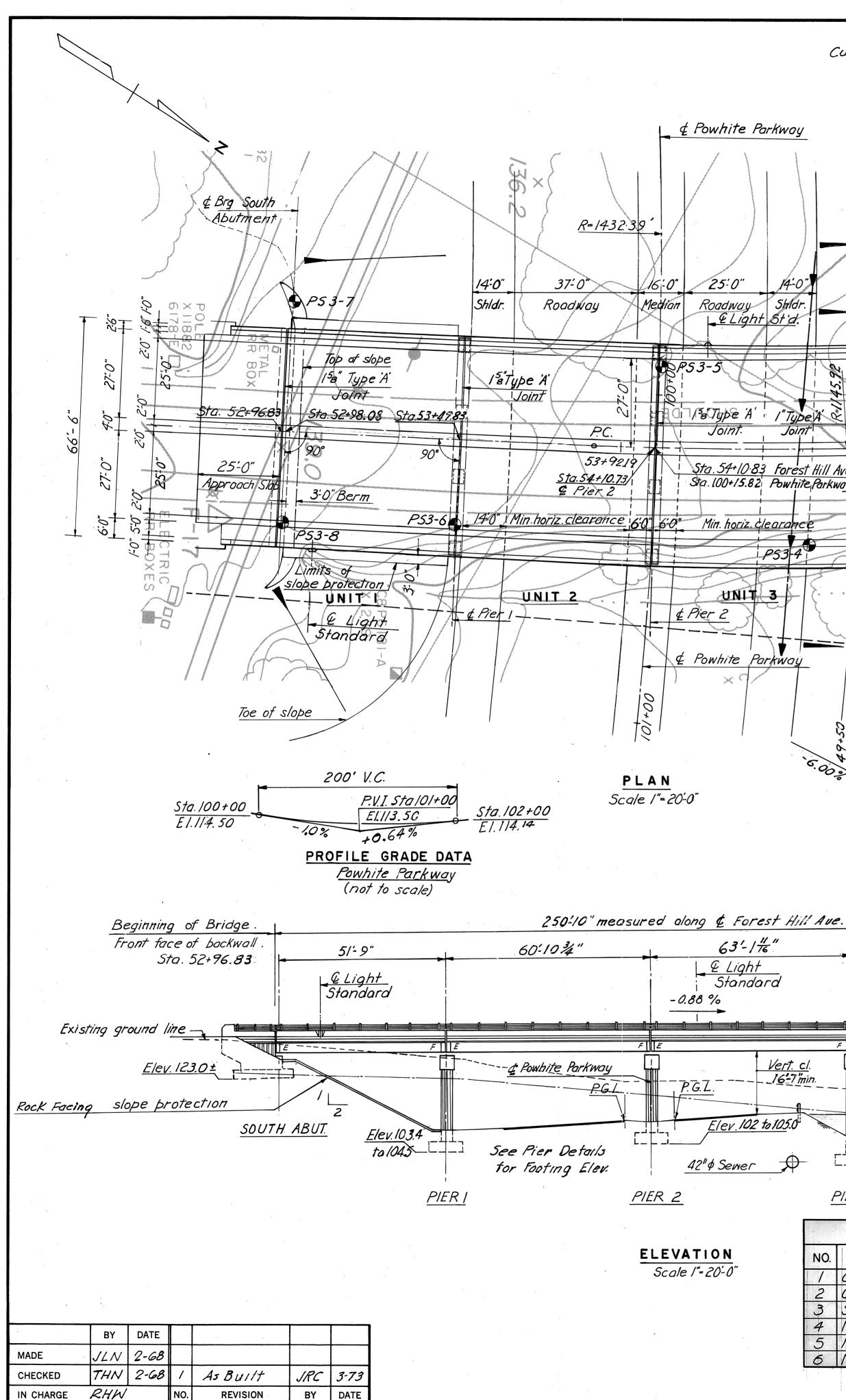
2900 S. QUINCY STREET, SUITE 200 ARLINGTON, VIRGINIA (703) 824–5100

cale:

## Bridge 6

(Forest Hill Avenue over Powhite Parkway - VA State Rte. 76)

**Record Set Plans** 



BY DATE RHW REVISION NO.

CURVE DATA: D=5°00' R=1145.92' GENERAL \_ & Powhite Parkway ROADWAY - Approx. Toe of Slope CAPACITY & Relocated SPECIFICA Powhite Creek ¢ Brg. North / Abutment ● PC4-3 25:0" 14-0 Roadway Shidr. E Light St'd. Existing culvert - PS3-1 to be removed. Scupper 1<sup>3</sup>8" Type A joint DATUM Sta. 54+73.87 Sta. 55+ 46.41 Sto. 55+ 47,66 € € P.G.L. 15 Type A' TEMPERA l"Type A & Joint Forest Hill Ave. Joint. totida DIMENSIO Scupper 55+00 Sta. 54+10.83 Forest Hill Ave Sta. 100+15.82 Powhite Parkway 25-0" Approach Slab 30 Berm EXCAVATIO - P53-2 P53-4 P\$3-3 FOUNDATIO UNIT 4 Standard UNIT 3 See sh.12 for Scupper Details ¢ Pier 3 — Approx. location exist. 12" Gas line CONCRETE & Powhite Parkway X Approx. Toe of \$ lope Line of sewer (To be removed) 300' V.C. 500° V.C. 6 6.00. -0.88% × 1.48% P.V.T. 51 El. 141: PROFILE GRADE DATA STEEL NOT Forest Hill Ave. (not to scale) End of Bridge Front face of backwall. Sto. 55+47.66 63'-1"" 72-6 16" <u>E Light</u> Standard <u>& Light</u> Standard BENCH M Finished ground line -Finished ground line, W.side <u>Vert. cl.</u> \_<u>16-7"min.</u> HW Elev. 115.8 ---<u>E Relocated</u> Powhite Creek BORINGS Existing ground line -Elev.10800-Elev. 102 to 105.0 Approx. line of rock along Elev. 107.8 \_\_\_\_\_ east side of bridge 42"ø sewer NORTH ABUT. Elev. 96 to 98.5 Invert Level (to be removed.) <u>PIĖR 3</u> Rock Facing Slope Protection above Dry Riprop Class 1 7 Pier I Details 16 Utility Suppor INDEX 

 17
 Approach Slat

 18
 Boring Logs

 51
 Standard Shi

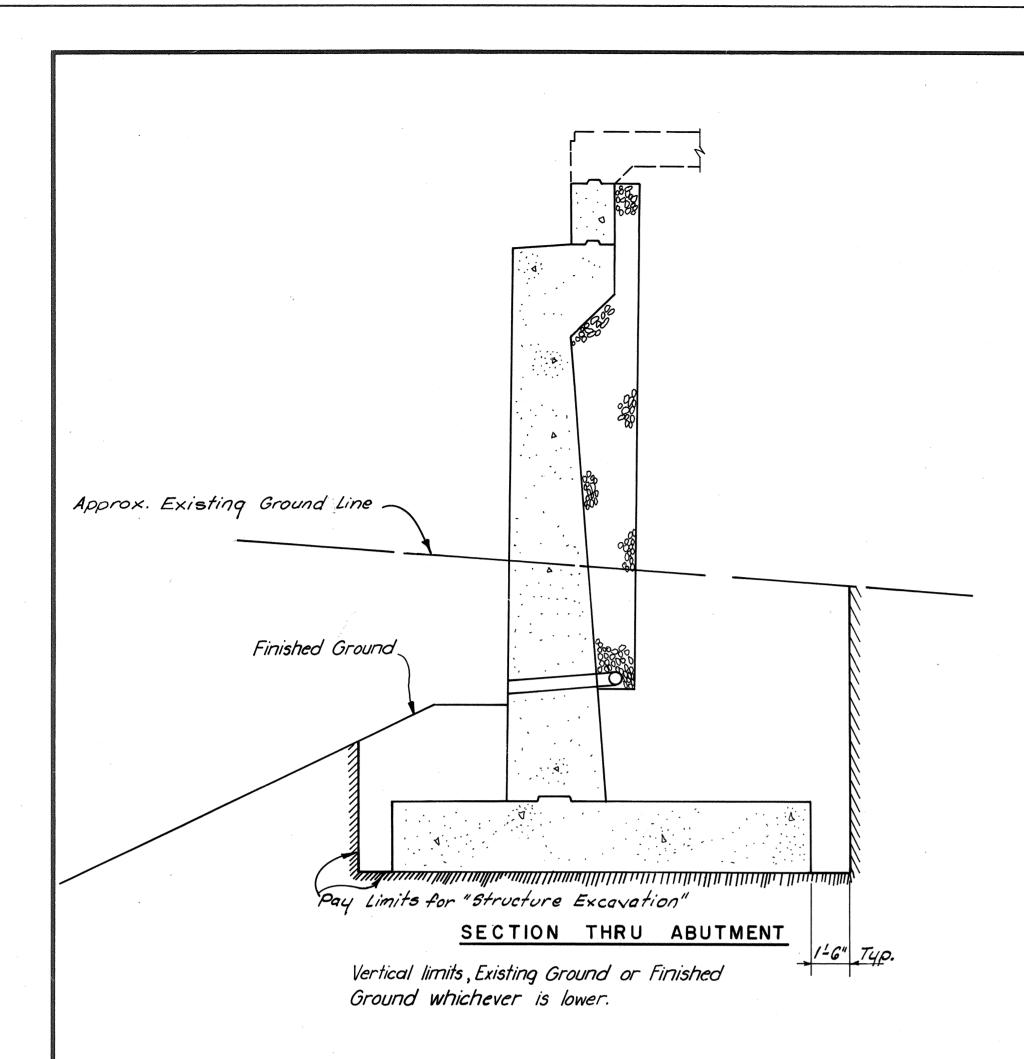
 53
 Standard Alu

 57
 Standard Arci

 58
 Standard Arch

 8 Pier 2 & 3 Details DESCRIPTION NO. 9 Framing Plan 10 Typ. Transverse Section General Plan and Elevation II Deck Plans Quantities and Misc. Details 12 Scupper Details 13 Electrical Detail South Abutment Details 3 North Abutment Details 4 14 Joint Details 15 Conduit Installation Details North Abutment Details (I) 6 North Abutment Details (2)

					RICHMOND EXPRESSWAY	SVSTEM	,
				SECTION	PROJECT	SYSTEM SHEET NO.	TOTAL SHEETS
				2	POWHITE PARKWAY	151	18 <b>8</b>
NOTE	<u>'S:</u>						· · · · · · · · · · · · · · · · · · ·
			-		alk. One l'-6" c		
	Dead Load- Live Loads- L	Includes 15/b. HS20-44 load	s. per sq. ft. f ling and B.P.I	for futu R. modii	ure wearing su fied for military	rface. vehicle.	5.
TONS:	cations, 1970 DESIGN-A.A.S modified by WELDING- 19	5.H.O. Standar Special Desig	rd Specifican an Provisions Specification	tions for s. as for W	Poad and Bridge - Highway Bridge lelded Highway Society.	s, 1961,	• 
	CONTRACT	SPECIAL PRO	NISIONS				
	Specificatio are necesso CITY OF RICI	ary to make i	tract Special these plans of	Provisi Complete	ons referred to e.	above	
URE		temperature for movemen			plan is 68° F. The	e tempe	ra-
s:					nd vertically unle	ess othe	e <i>r-</i>
<b>V.</b>	as Structure	Excavation.	All excavation	on abov	late shall be cla these limits sh ncluded in the S	all be	al
VS:	and special cations, and	attention is	called to sec act Special f	tion 40	material shall b 1.05 of the Gener s, concerning pre	a spec	///-
NOTES:	Class A-3. A. fillet unless of low-slump downgrade	Il exposed ed otherwise n concrete, and movement of n	ges and corr oted. Care in d/or other m newly placed .	ners sho n the me neans si slab con	All other concrete all have a 3/4" co ethod of vibration hall be employed perete (When gradie	hamfer n, the us d to pre	or se vent r 2%)
	Finishing con and the Con	ncrete <mark>surface</mark> tra <mark>ct Spe</mark> cia	es: See the St Provisions f	andard or types	Architectural D and details.	etail Sh	peets
	on the detaile noted. Clear d as noted on t	ed drawings a distance betwe	are to be to th en reinforcing bar laps sha	he Cente g steel c	nde 40. All reinf. bar ars of bars unless a and face of concre and face of concre	nth <b>erw</b> ise ete shall	e be
5:	Structural st	eel shall con	form to A.S.T	T. M. Spec	ification A36 ex	cept as .	noted.
	DOITS SHAIL DE	ections shall A 7/8" diamete ification A-32.	er unless oth	h high s erwise	trength bolts. Hi noted and shall c	gh strer. Tonform	ngth to
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1	in highway plan			1667			
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	y Indicates lo	cation of $2^{1/2}_{2}\phi$	cased hole bon	ring.			
	•						
	-				SBUIL		
	Bridge Abut. Slope Pro. Det.		RICH	RICHMO	METROPOLITAN IND EXPRESSWAY		RITY
itectu	. Det.(2 Rail) Iral Details			POV	HILL AVENUI WHITE PARKWA BRIDGE B-06	AY (	
Tectul	ralDetails	ļ	5			LEVA	
	•		MATEO,	Associate	Enginopro		
					MMEN & BERGENDOFF	NTRACT NO.:	

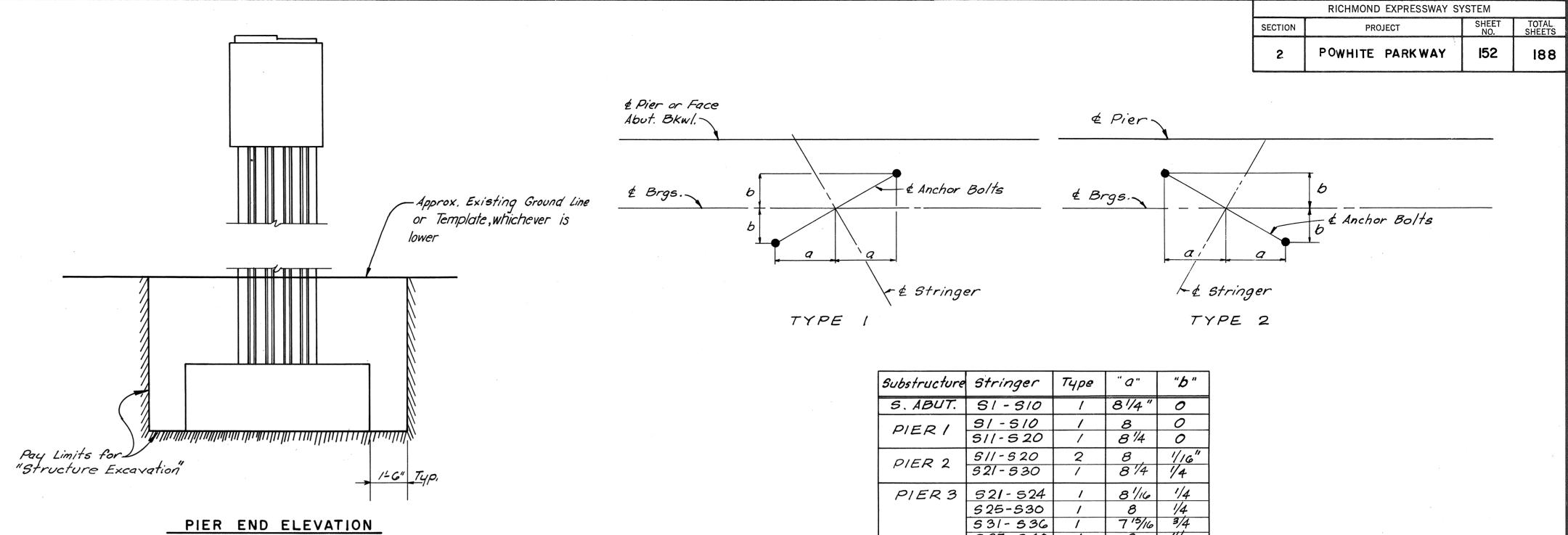


#### ESTIMATED QUANTITIE

	Structure Excavation	(a)	Reinforcing Stee		Aluminum Br. Railing (2 Rail)			,		Rock Face Slope Protection	Asphalt Damp- proofing	Underdrain G"ø Pipe	Conc.A Br. Appr. Slab
	<i>C.Y.</i>	, C.Y.	LBB,	Lbs,	L. F.	L, F,	L. F.	<i>L.F.</i>	C.Y.	S.Y	<i>G.</i> Y.	L.F,	C. Y.
Superstructure		535.2 (b)	120,415	498,775	501.0	305.0	606.0	1,830.0					
South Abutment	389	1944	7,066		27.0				32	438	97	122	
Pier I	102	90.5	27,970		-			·					
Pier 2	165.6	94.6	26,700										×
Pier 3	158.4	100.5	29,849										
North Abutment	5.77	5-18.2	37,909		46.0				103	146	309	132	
Approach Slabs			35,687							_			166
Total	1,392	998.2(a) 535.2(b)	285,596	498,775	574.0 (c)	305.0	606.0	1,830.0	135	584	406	25:4	166

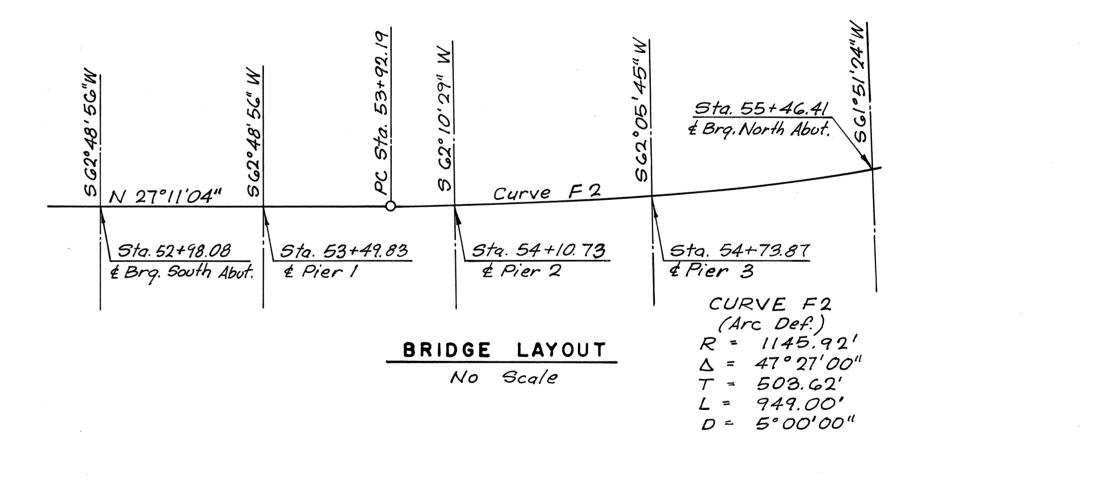
(a) Class A3, unless noted. (b) Class A4 (c) Includes that portion on abutments and wingwalls.

	BY	DATE				
MADE	R.5.5 A.J.P.	2-68	2	As Built	JRC	3-73
CHECKED	Т. Н. N.	2-68	/	General	J.G.V.	10-70
IN CHARGE R.	H. W.		NO.	REVISION	BY	DATE



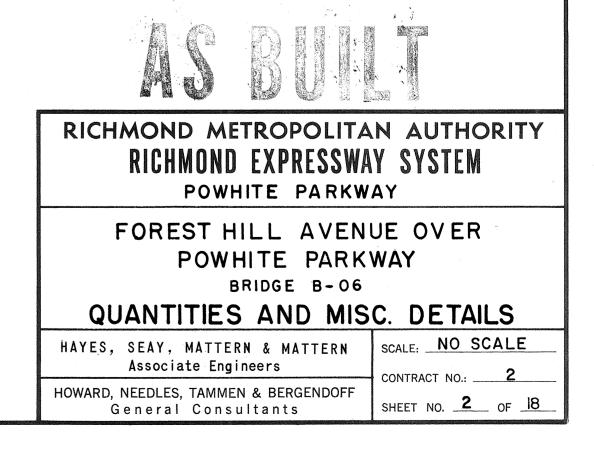
,	C	
,	J	

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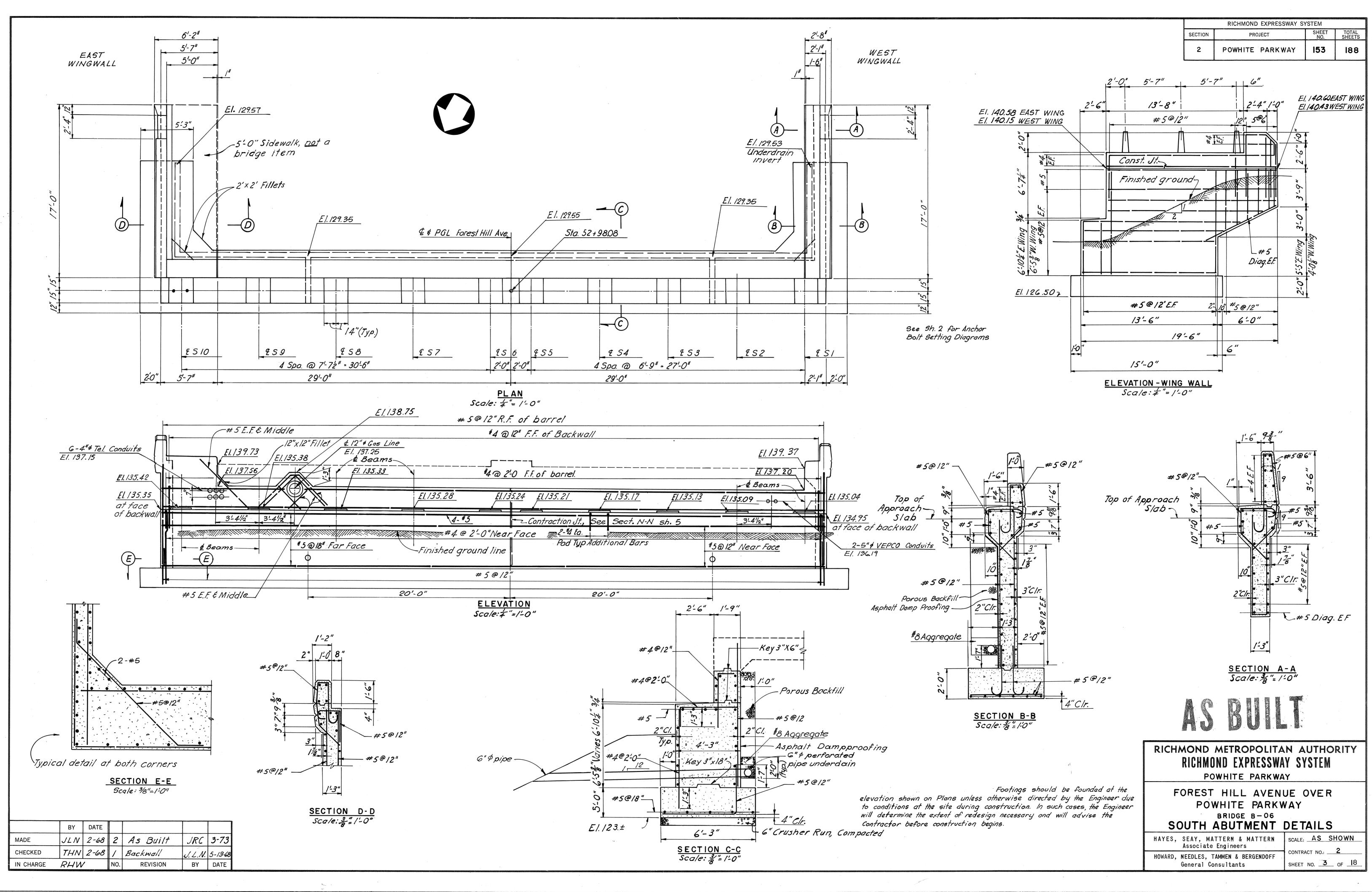


Substructure	Stringer	Тчре	" a "	"b"
S. ABUT.	51-510	1	81/4 "	0
PIERI	51-510	1	8	0
FIERT	511-520	/	8 4	0
PIER 2	511-520	2	8	"/16"
FILK	521-530	/	8 1/4	1/4
PIER 3	521-524	1	81/16	'/4
	525-530	/	8	1/4
	531-536	1	7 15/16	3/4
	537-540	1 ·	8	"/16
N. ABUT.	531-534	1	8 3/16	15/16
	535	1	81/4	7/8
	536-540	1	8'/4	"/16

#### ANCHOR BOLT SETTING DIAGRAMS

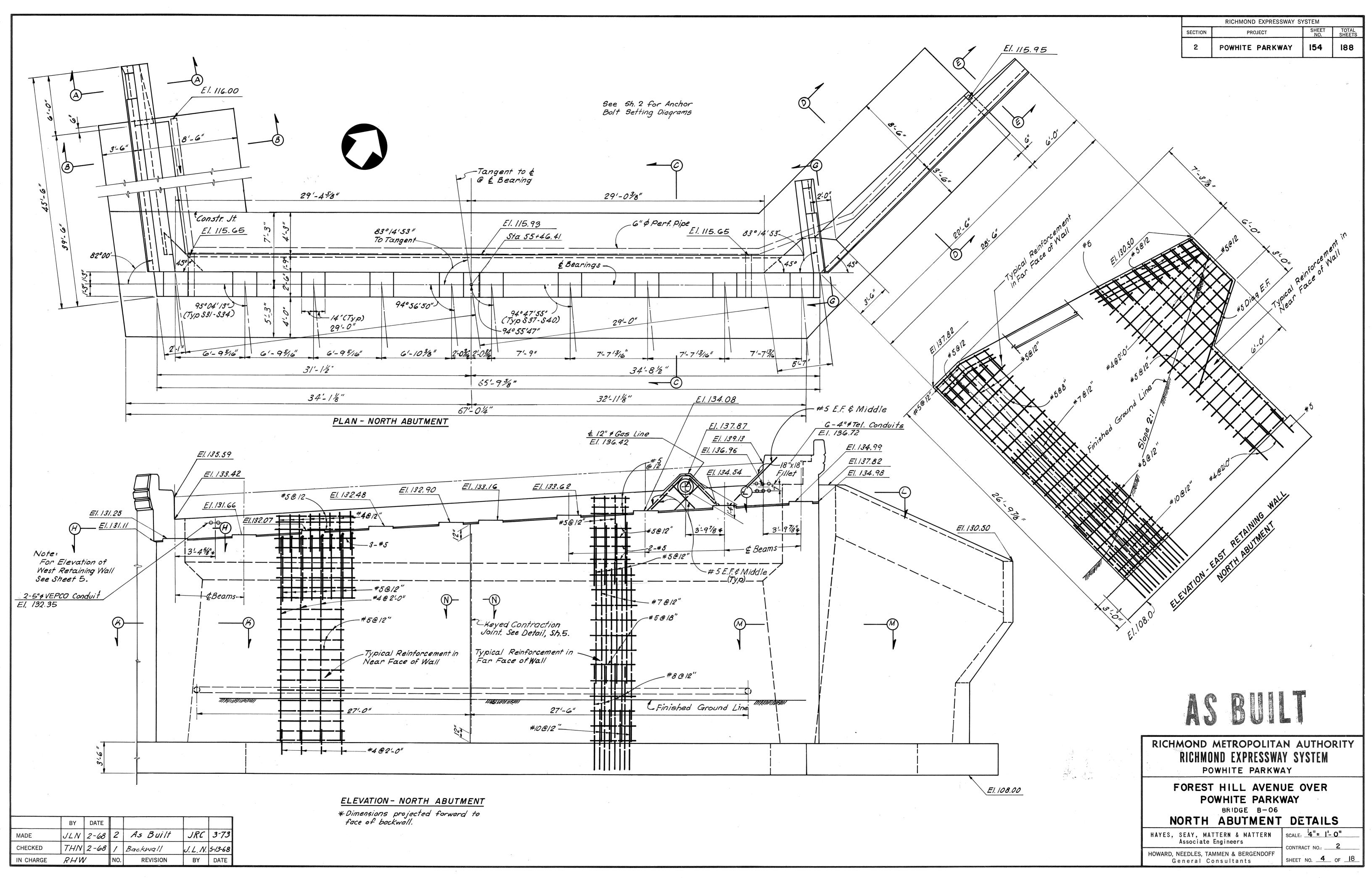


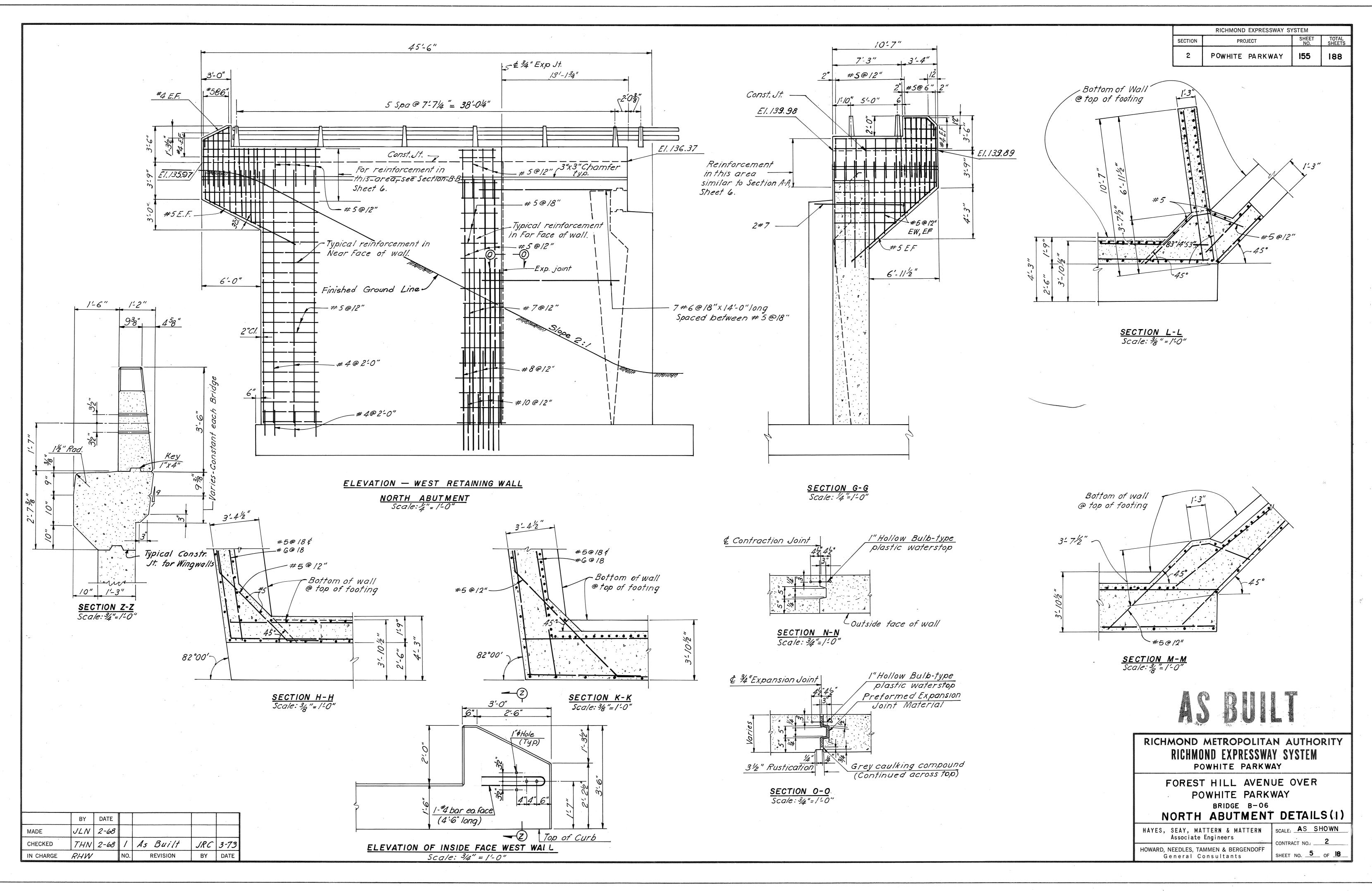
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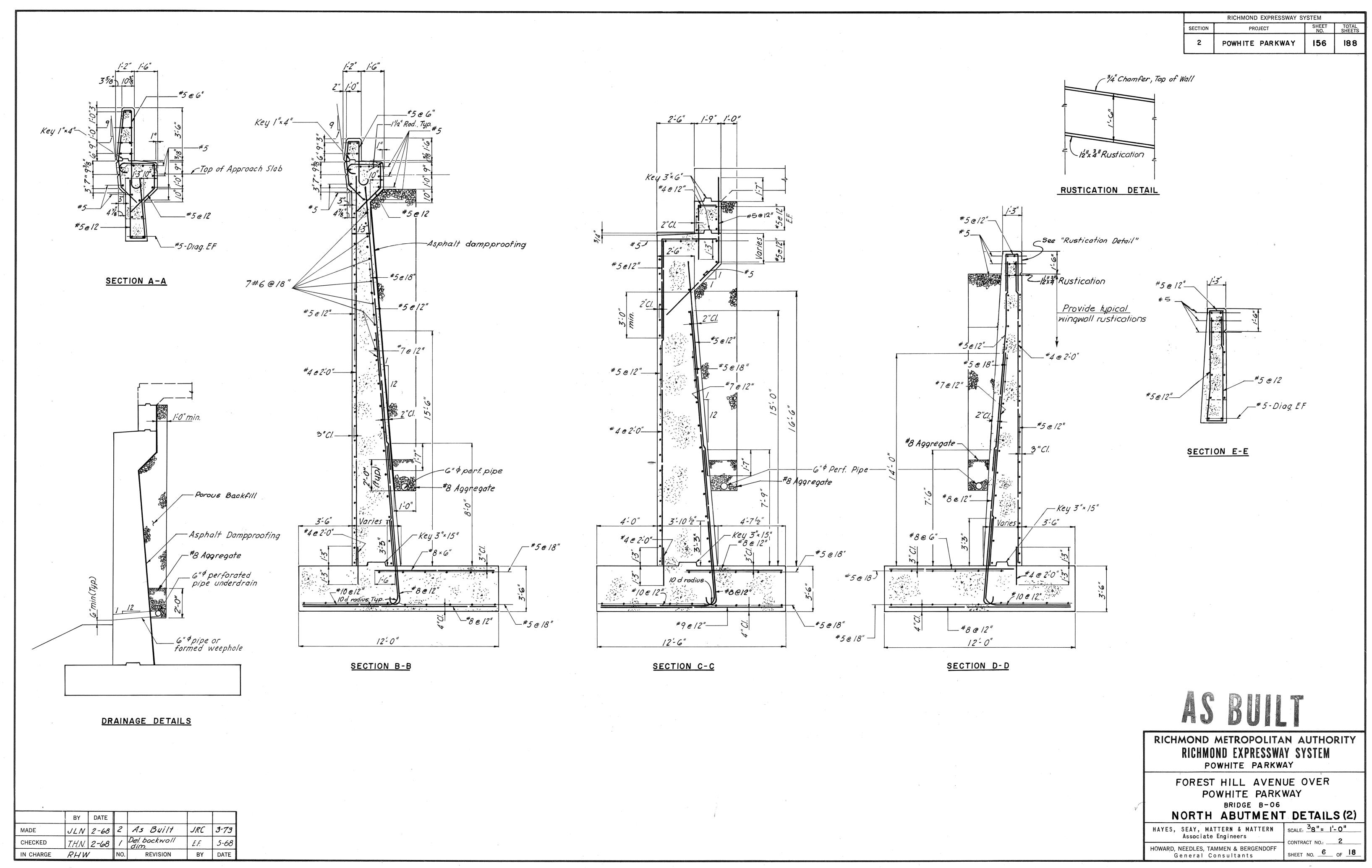


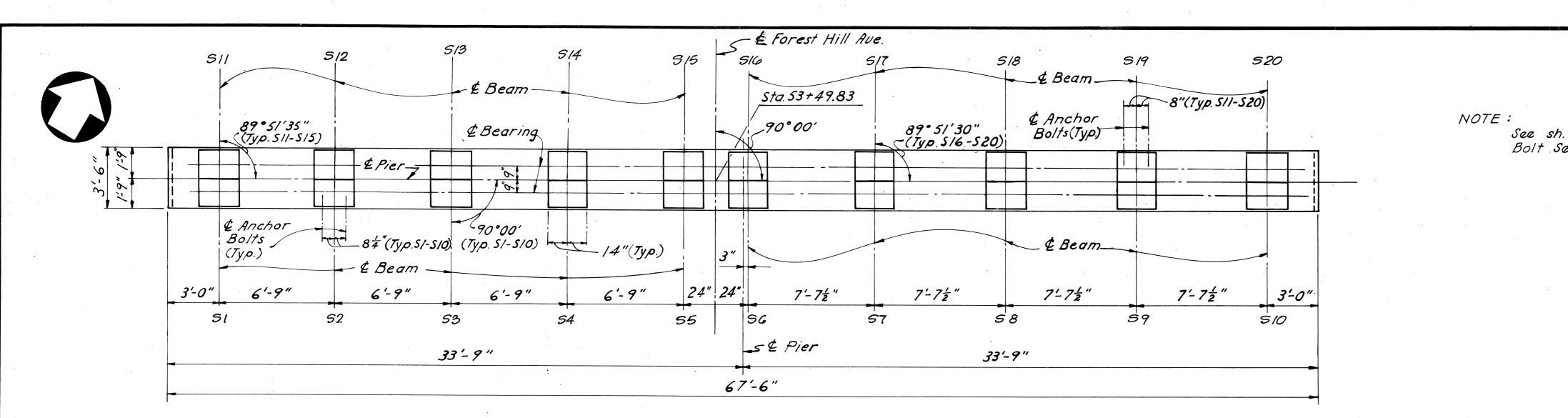
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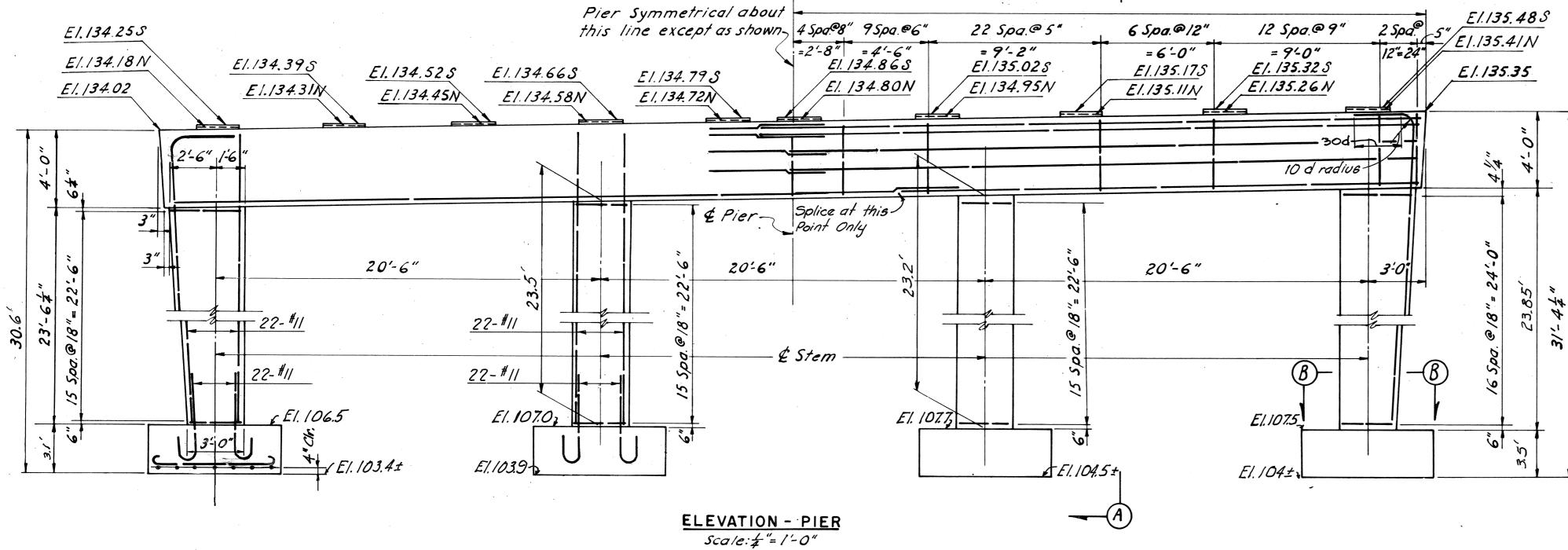
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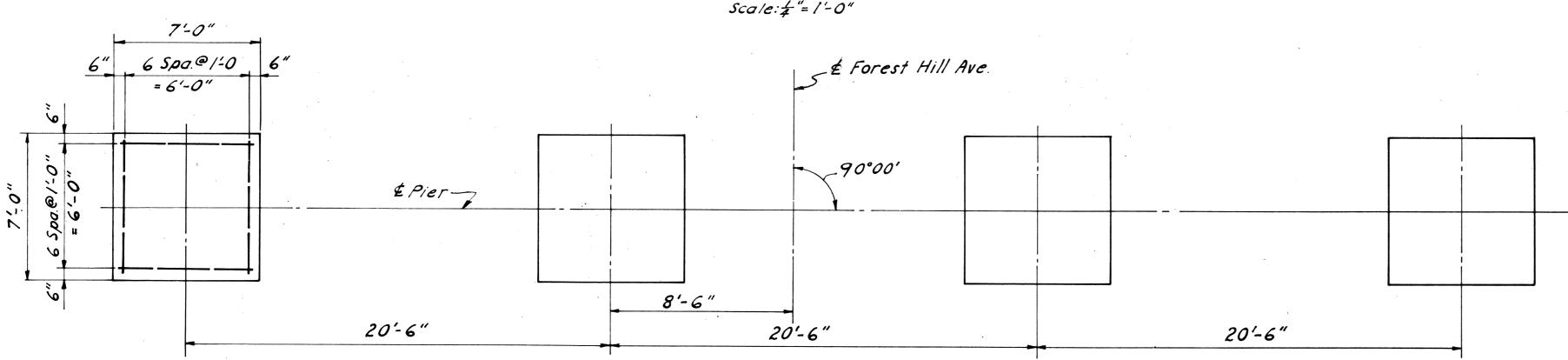










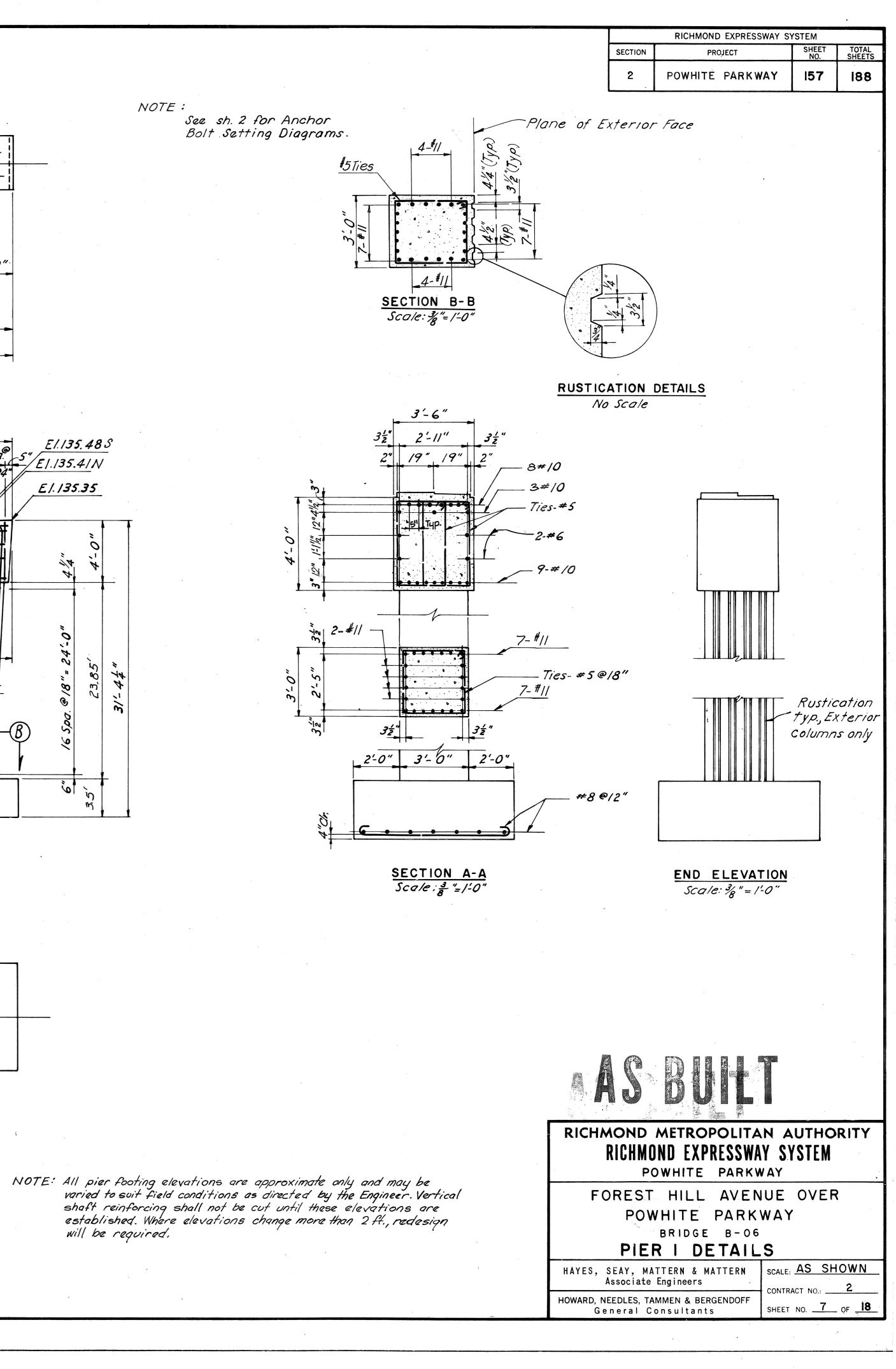


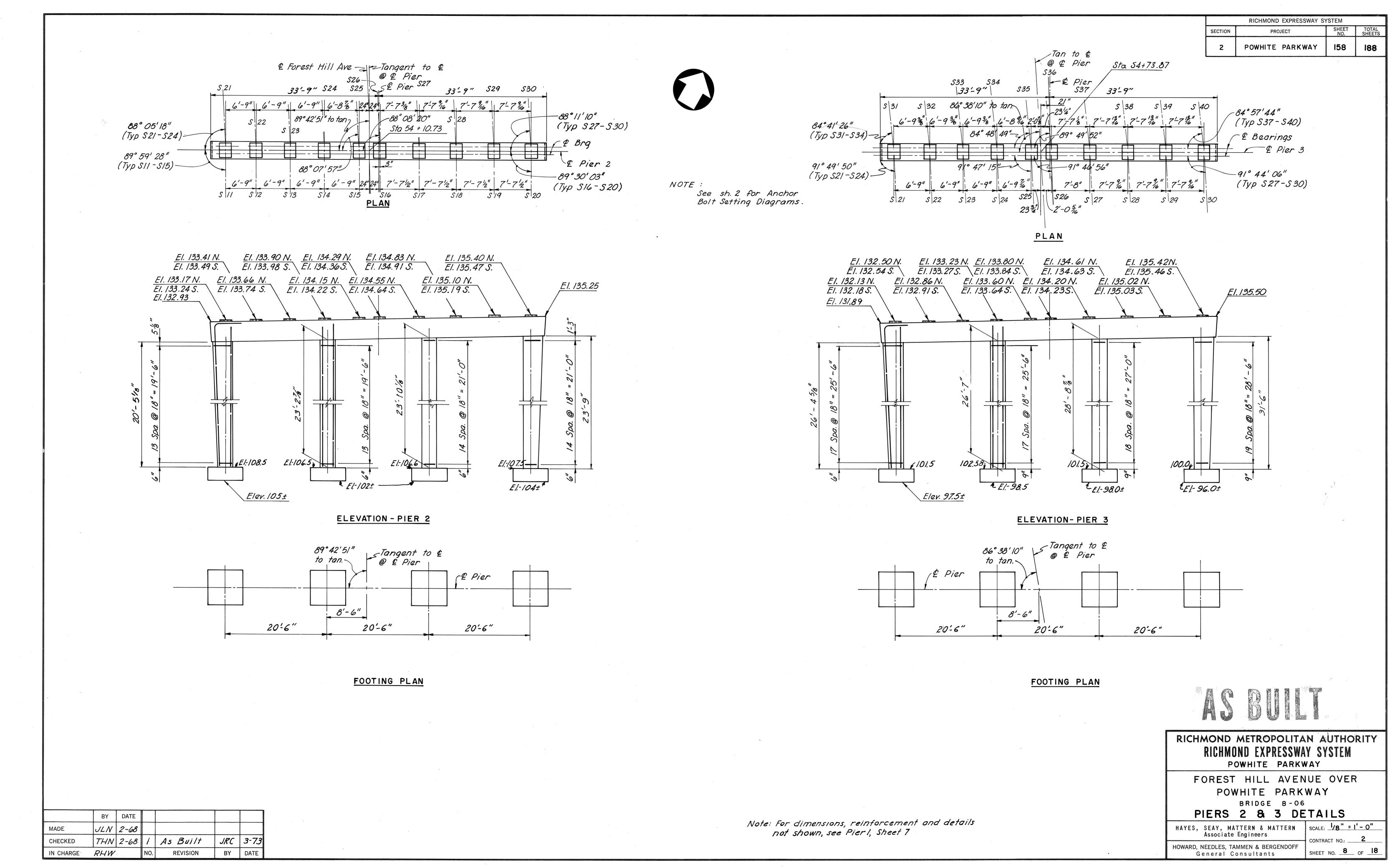
	BY	DATE				
MADE	JLN	2-68				
CHECKED	THN	2-68	1	As Built	JRC	3-73
IN CHARGE	RHW	/	NO.	REVISION	BY	DATE

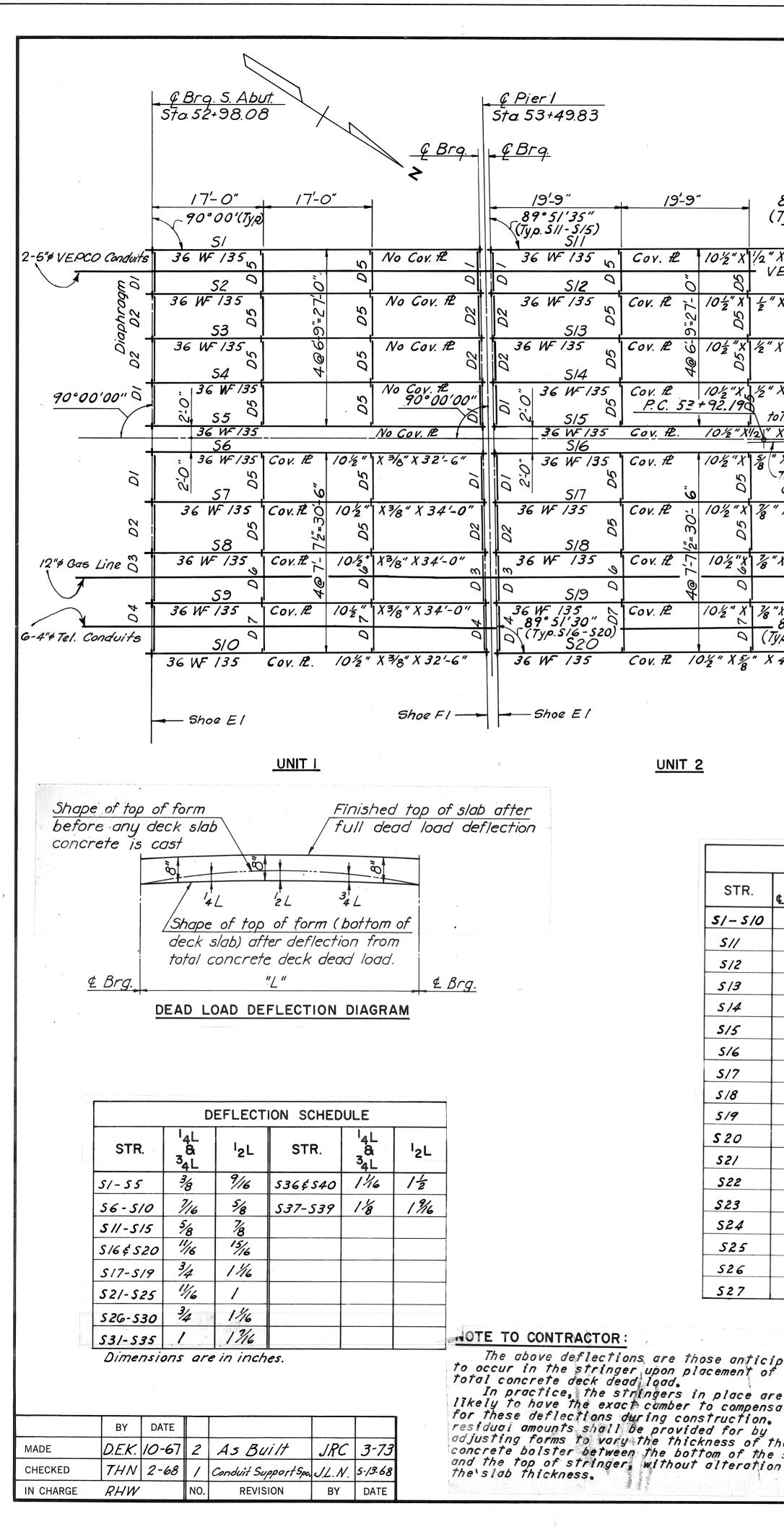
 $\frac{\text{FOOTING PLAN}}{Sco/e:\frac{1}{4}} = 1'-0''$ 

PLAN Scale: #"= 1'-0" ---(A

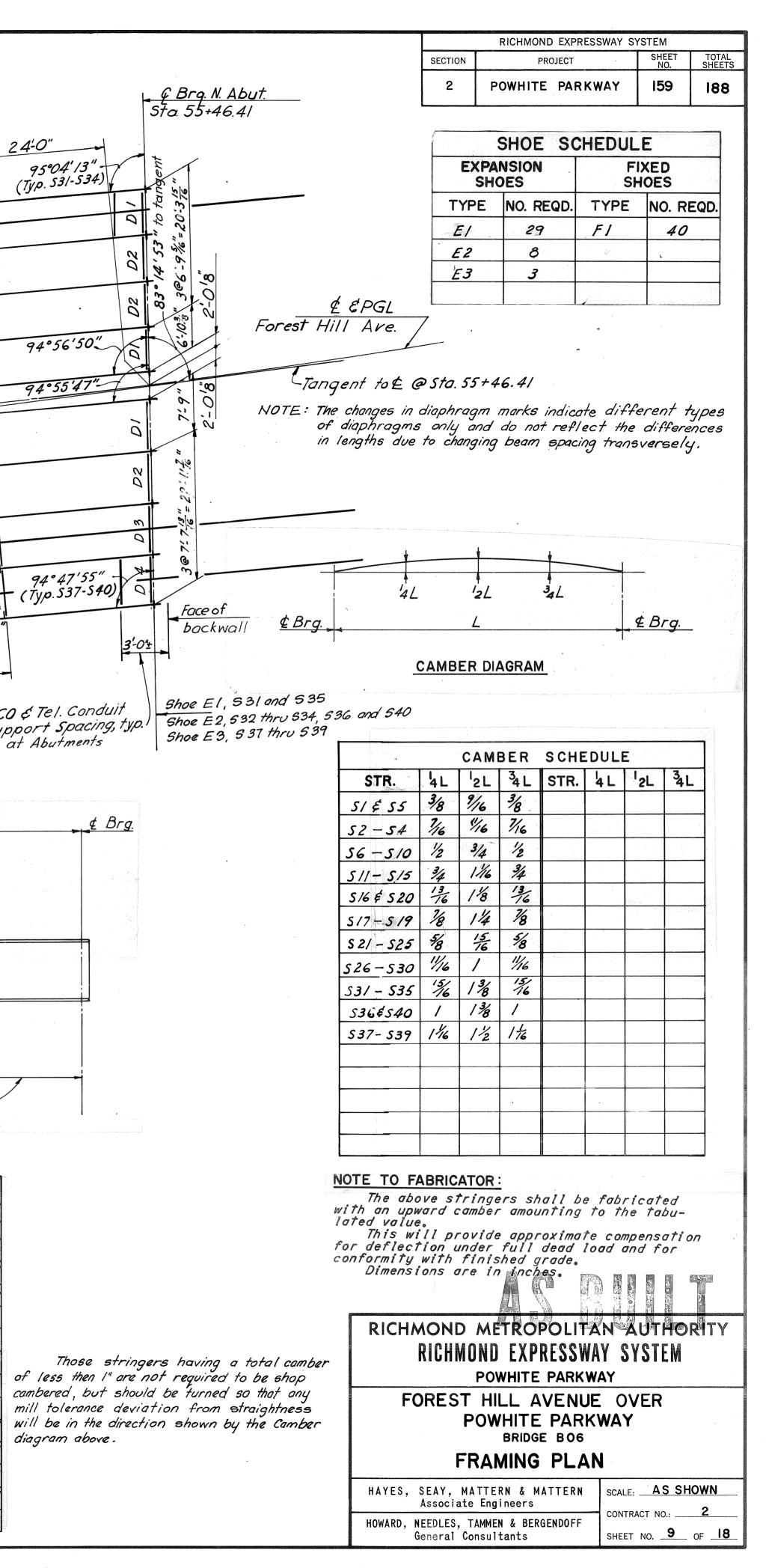
will be required.



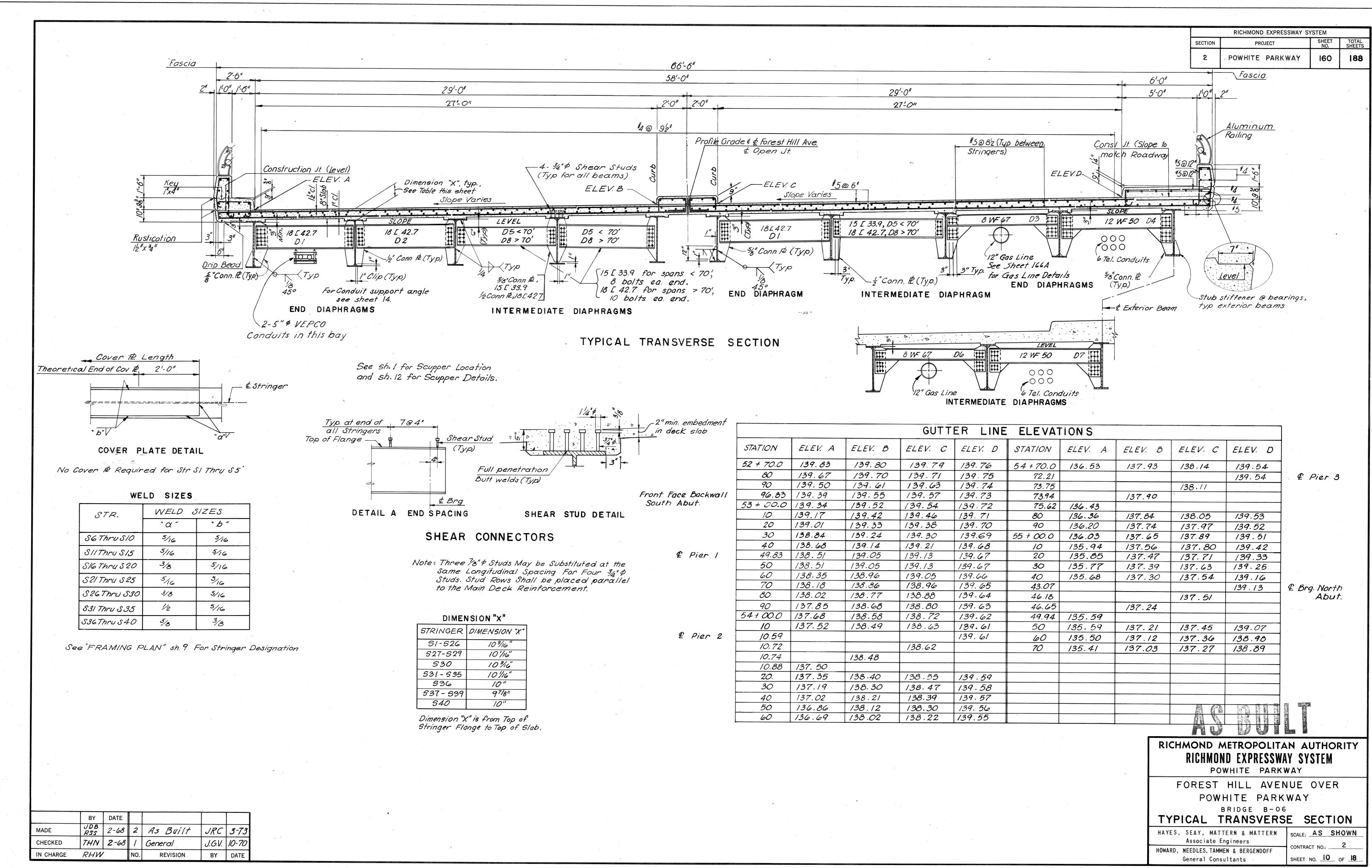


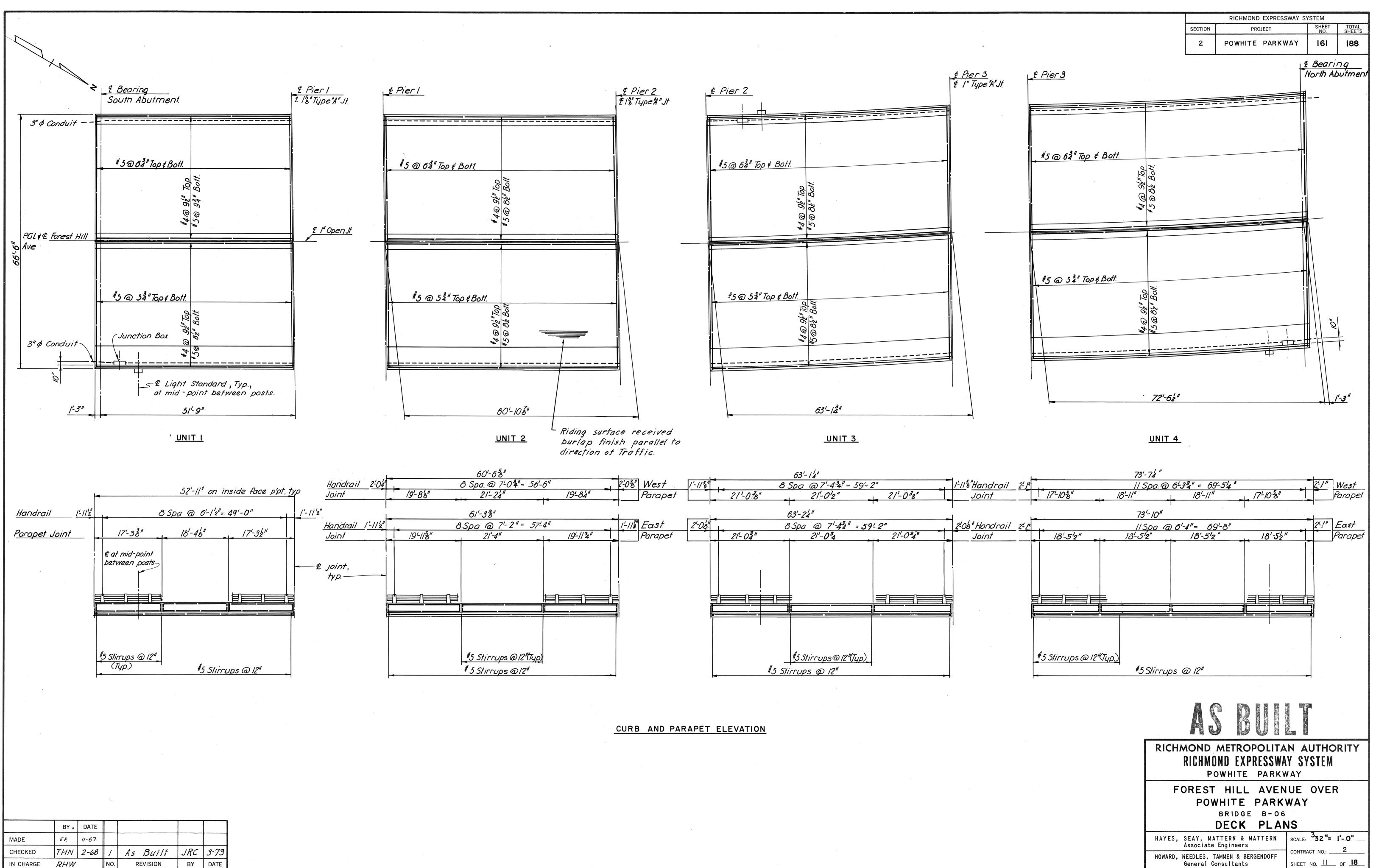


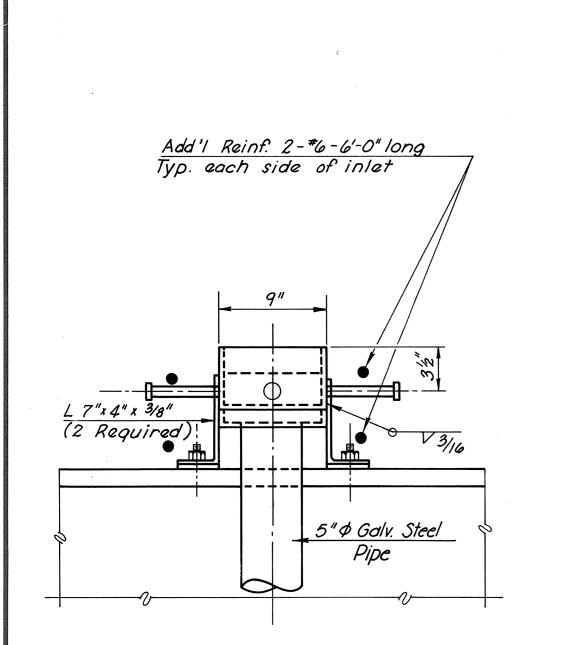
	<u>6 Pier 2</u> Sta. 54+10. 73			- 4	<u>: Pier 3</u> Sta 54+73.87						
<u>é Brq.</u> 9" typ.	<u> </u>		<u> </u>	<b>'</b>	<u>&amp; Brq.</u>		24	1-0"			24
89° 29' 58" (Тур. SII- SIS)	20'-6" - 88°05'/8" - (Typ. 521-524)	20-6" 521	9/*49*58 (Typ. 52/-52	4)	84°41'26" (Typ. 5.31-534)		531 ov. Æ 10	0%"X/"X	52'-6	200	(
"X 40'-6" VEPCO	36 WF /35	52/ Cov. # 10%"X % * X 43 : 6" S22	9="20		36 W 150						
-"X 4/'-0" Q	02	Соч. # 10½"Х 5%" X 44-0 523	<u> </u>	38'/0" tangen	4 0% 4 0% 4 0 19 36 WF /5 C	0	ov. 12 10	21			
" X 4/'-0"	36 WF /35 88 88 2 2 36 WF /35 5 36 WF /35 5	Cov. # 10½ X 5% X 44'-0 524 Cov. # 10½ "X 5% X 44'0		8	80 36 WF /50 80	8	ov. 12 10	25			
" X 4/'-0" 89°42'5'" toTang@&Pier " X 40'-6"	88°08'20"	S25 Cov. # 101/2"X12"X43'-0 S26	6″		36 WF /50 36 WF /6	C	5 ov. # 10 v. # 102	536 536 537 536	X 52 X 53-	-6 -6 -6 -6 -6	
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ζ" <i>X 44'-6"</i> η Ο	6/2 36 WF 150 6/2	Cov. ₽ 10½"X 3 "X 45'-C S29	3 Sec		E 36 WF 170		COV. Æ 1			2-0"	- (
8"X 44'-6" - 89° 30'03" (Typ 516-520) 7	( <i>Typ. S27-S30</i>	Cov. £ 10½"X 3"X 45'- 530 Cov. £ 10½"X 78"X 46	<u> </u>	30)7	84° 57' 44 (Typ. 537-54 36 WF 16	0) °.	Cov. R		14 X :	53'-6"	
X 43'-0"	36 WF /35 20'-6"	20-6"	(Тцр.)	) 2'-6"	2'6"( <i>Typ.</i> ) 24'-0"			24-0		EPC	+ 0
Shoe Fl →		thru 526 and 530 thru 529	VEPCO	) & Tel. C	Shoe Fl Conduit	_				SUP	at
	FRAMING PLAN	<u>UNIT 3</u>		Brg	ring at Piers,		<u>U</u> ngth L	NIT 4			ana sa
	Scale: <sup>3</sup> 3'2= '-0"		에 가 가 가 가 가 가 지않는 것 같은 것 같이. 같은 것 같은 물질 것		0.12 , 0.22 , 0.32	0.4L		(Symm. stringe	-	É	
STRINGER LENGTH	T	PACINIC						en nge	., ,	•	
LENGTH	G. O.IL O.2L O.3L O.			T			ļ				
5/'-0"		0" 12'2"									
59'-0%" 59'-1¾"	7" 8" 9½" 1 6½" 7½" 9"		ee Detail "A" on sh. 10								
59'-2#6"		11 14									
59'-3%"		11" 14"			-	×				~	
59'-4'2"		1/2" 14 2				Cov	er Pla	ate			
59'-5 <sup>1</sup> /6" 59'-6 <sup>1</sup> /6"		10 13 13 12 1 10 11 12 12 1				iee Fr	aming	Plan			1
59'-7 <u>7</u> 6" 59'-8'8"	52" 62" 8" 1	0" 12'2"		1	en en la construcción de la constru La construcción de la construcción de La construcción de la construcción d	INGER	<i>Dimen</i> ELE	:VAT 10			
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61'- 75/8 "		0'2" 13'2"		\$ 30 \$ 3/	61'-8'8" 71'-8'16"	6" 7"	1			13 <sup>1</sup> / <sub>2</sub> " 15"	
61'- 734"	52" 62" 8" 1	0" 12 2*		S 32	71-8%	62"		1		/ <del>5</del> /4"	
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			and a second	1	71'-916"	6'2"	71/2"	9" 1		14"	01 Ca
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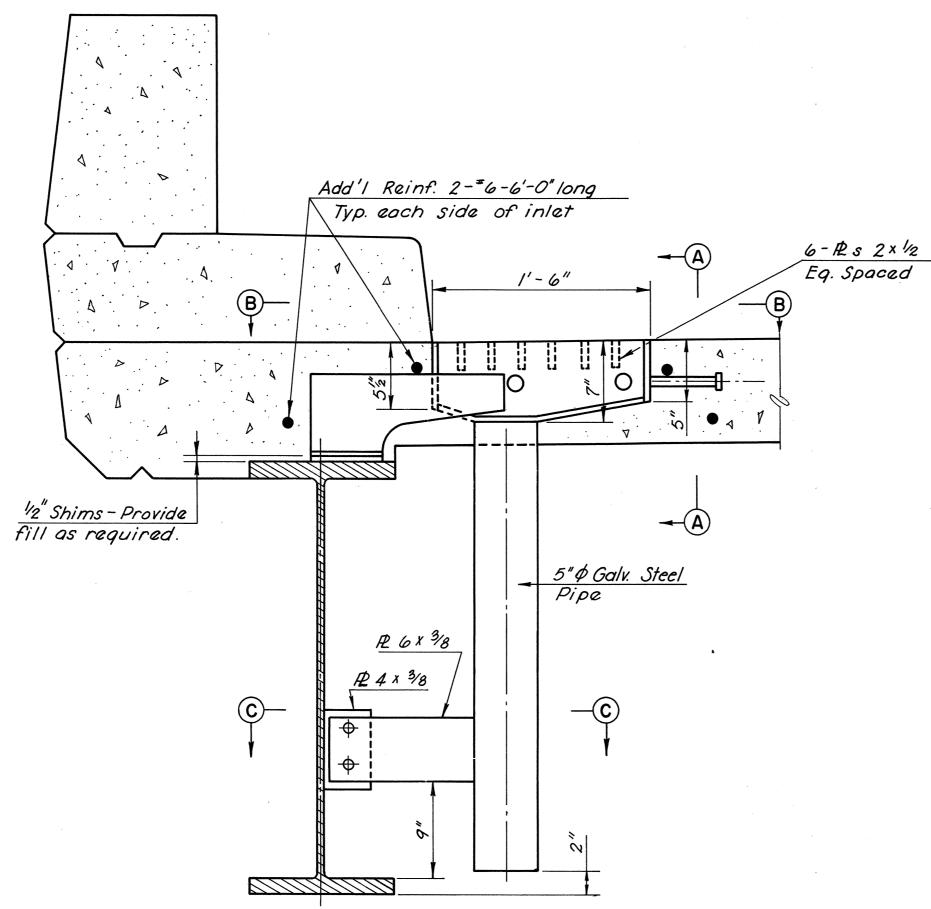
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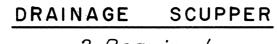




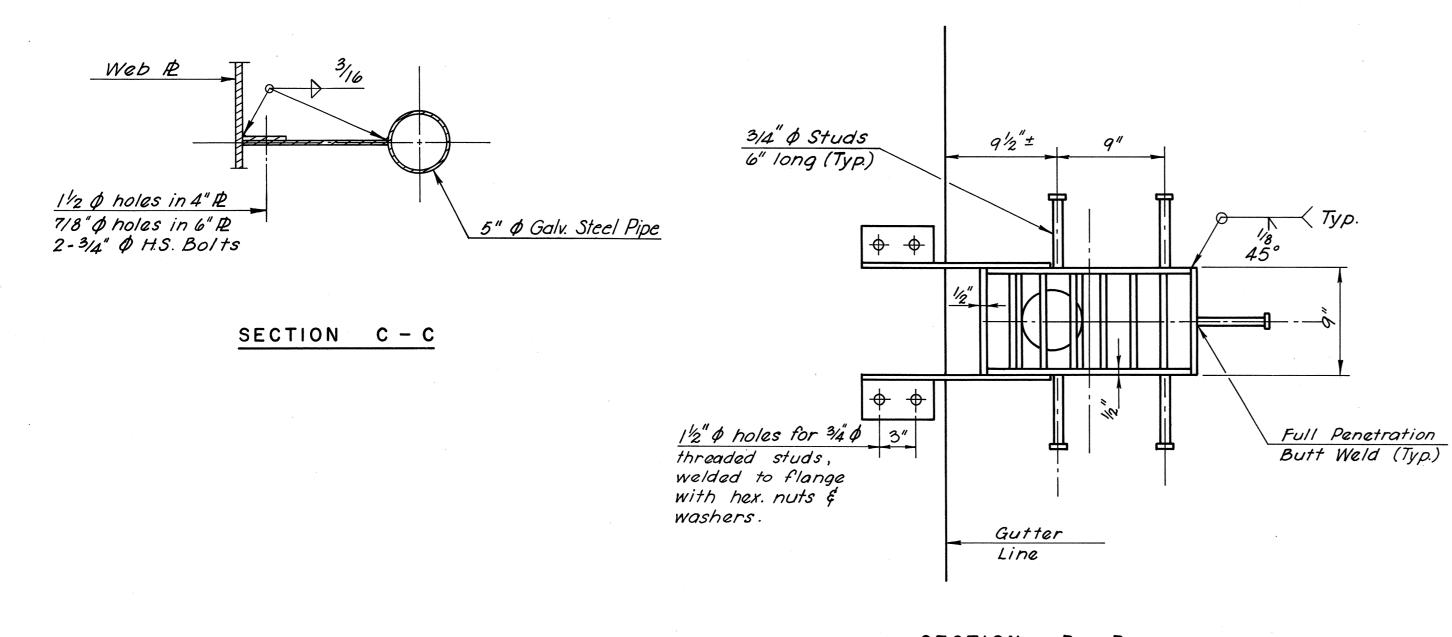


SECTION A - A





2 Required



-						
	BY	DATE				
MADE	SCH	2-68	2	As Built	JRC	3-73
CHECKED	RHW	2-68	1	General	J.G.V.	10-70
IN CHARGE	RHW		NO.	REVISION	BY	DATE

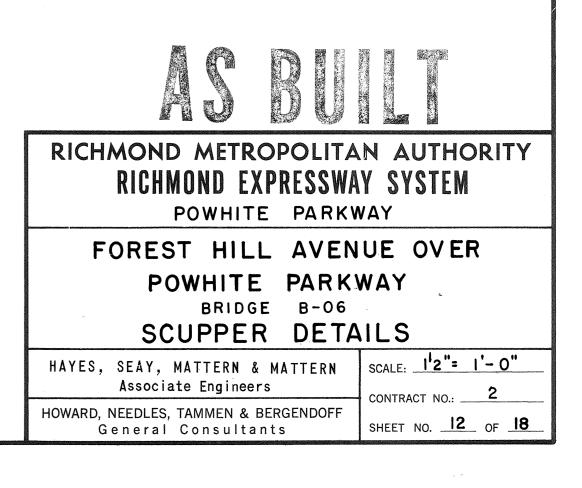
SECTION B - B

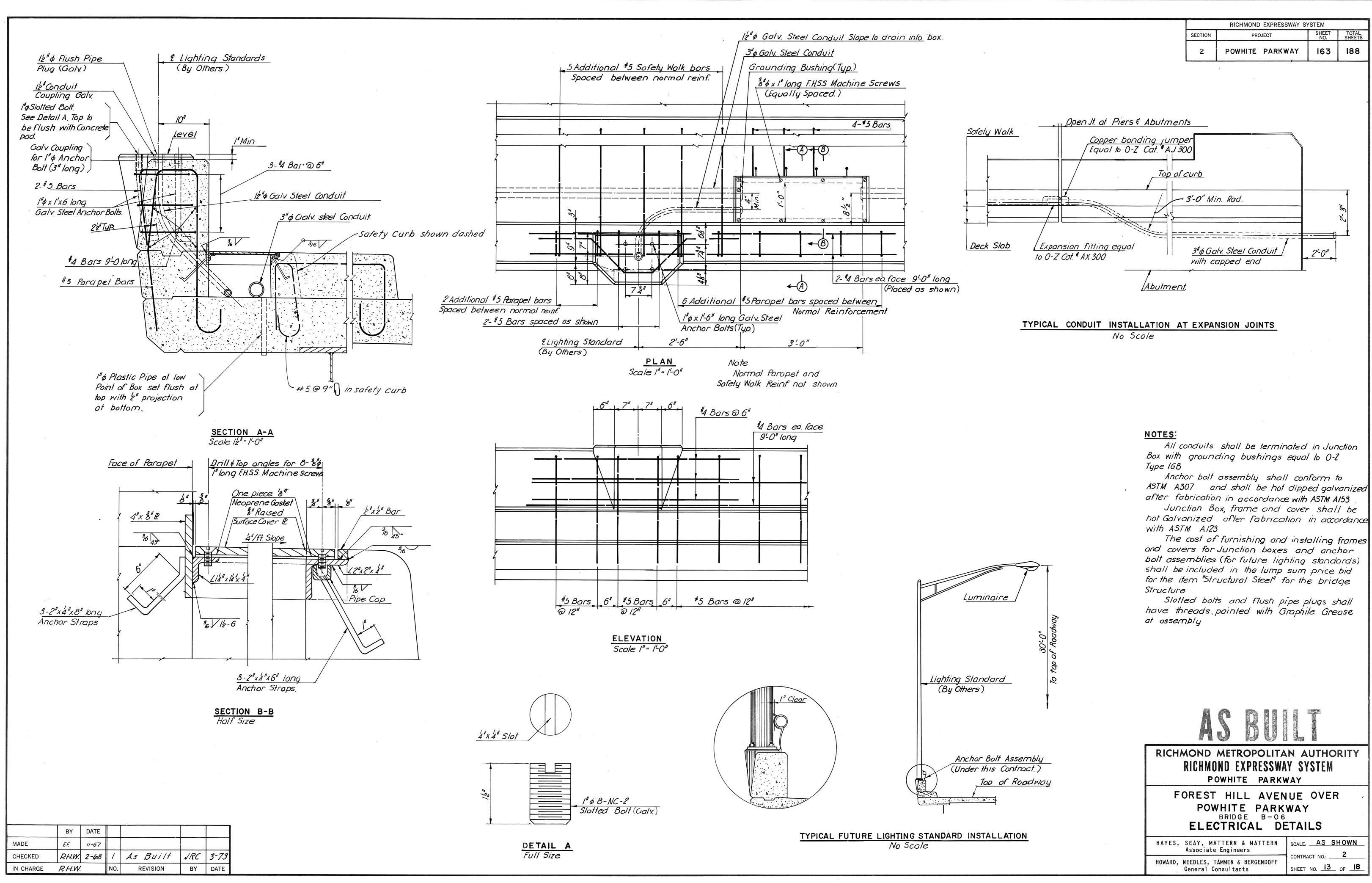
	<b>YSTEM</b>	RICHMOND EXPRESSWAY SY		
T TOTAL SHEETS	SHEET NO.			
188	162	POWHITE PARKWAY	2	

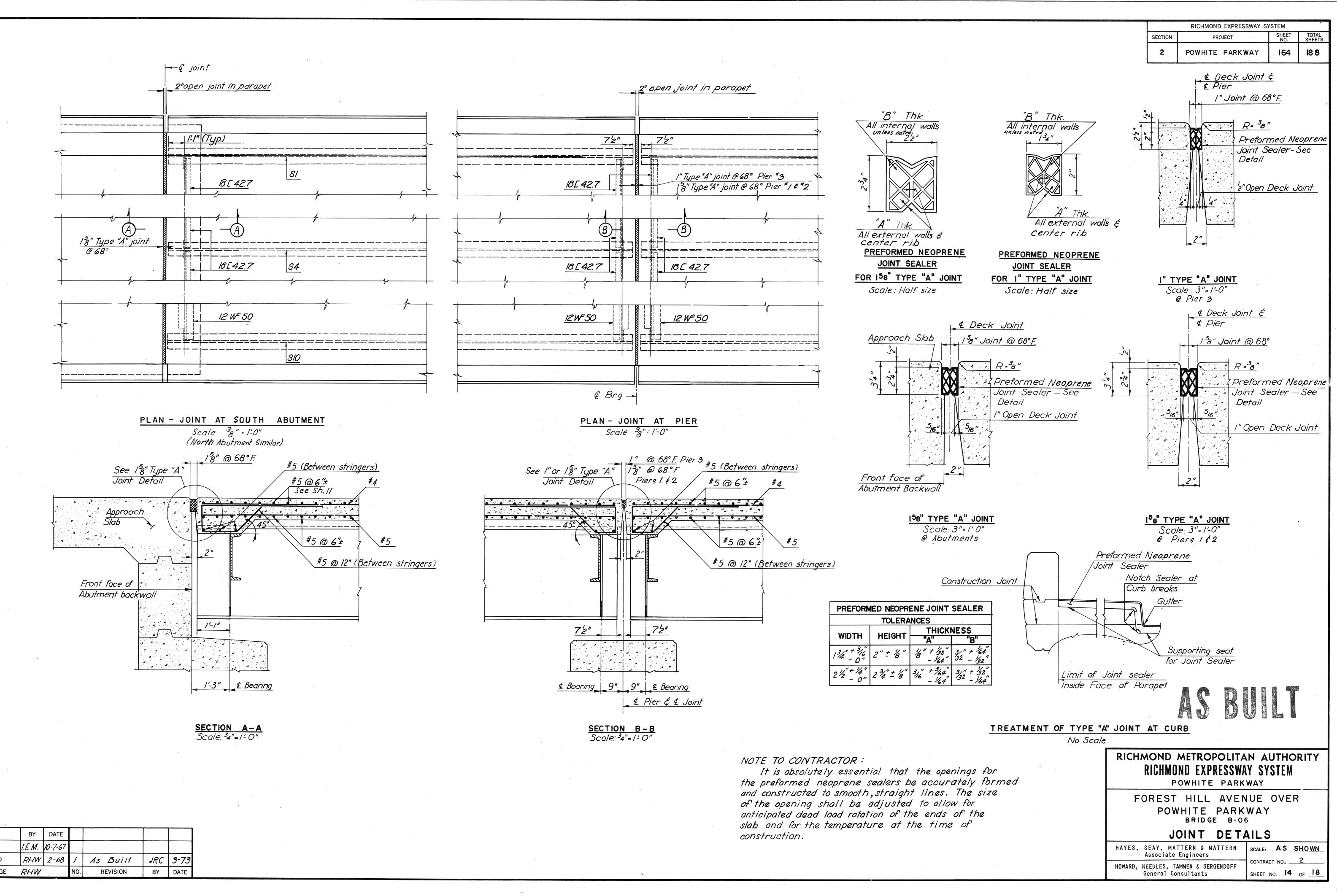
#### NOTES

For General Notes . see sheet No. I For location of scuppers , see sheet No. I Scuppers are to be fabricated from 1/2" thick plates conforming to A.S.T.M. A36 Galvanized Steel Pipe to be in accordance with A.S.T.M. A333 - Grade 9.

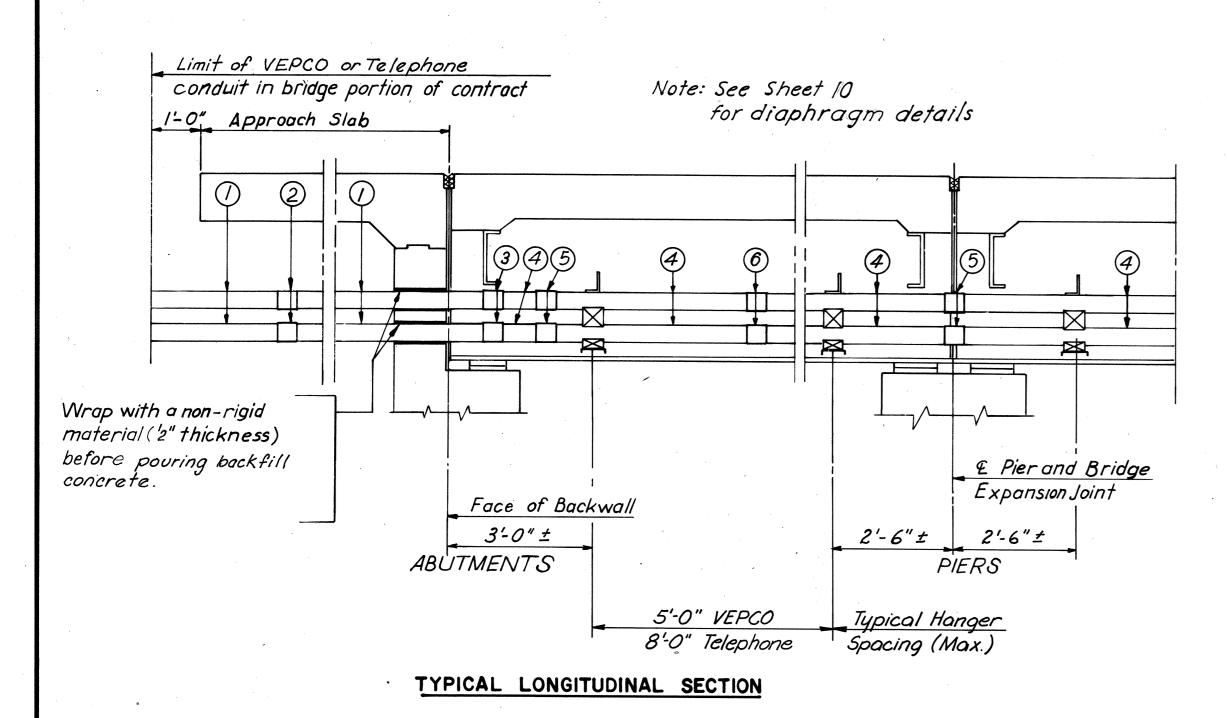
All drainage metalwork shall be hot-dip galvanized after fabrication in accordance with the following specifications : Scuppers - A.S.T.M. A123 Pipe - A.S.T.M. A53 Cost of all drainage metalwork to be included in the bid price for "Structural Steel" Cut normal Deck Reinf. at scupper as required.

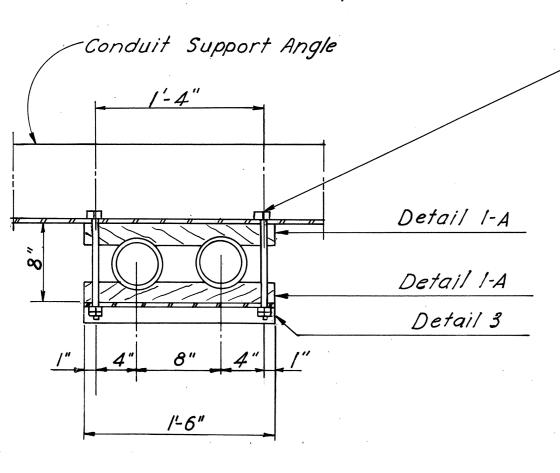






	BY	DATE				
MADE	T.E.M.	10-7-67				
CHECKED	RHW	2-68	1	As Built	JRC	3-73
IN CHARGE	RHW		NO.	REVISION	BY	DATE



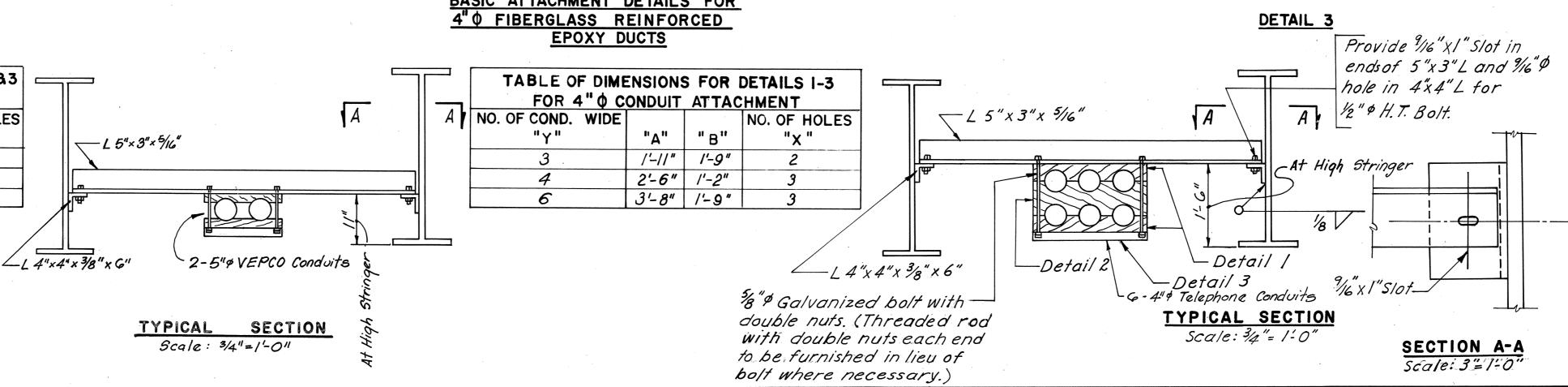


58" Ø Galv. bolt with double nuts(Threaded rod with double nuts each end to be furnished in lieu of bolt where necessary.)

VEPCO Conduits

# BASIC ATTACHMENT DETAILS FOR 5"¢ CEMENT ASBESTOS CONDUITS

TABLE OF DIME FOR 5"¢ C			
NO. OF COND. WIDE	"A"	"B"	NO. OF HOLES
2	/'-6"	/-4"	2
4	2'-10"	/-4"	3
6	4'-2"	2-0"	3

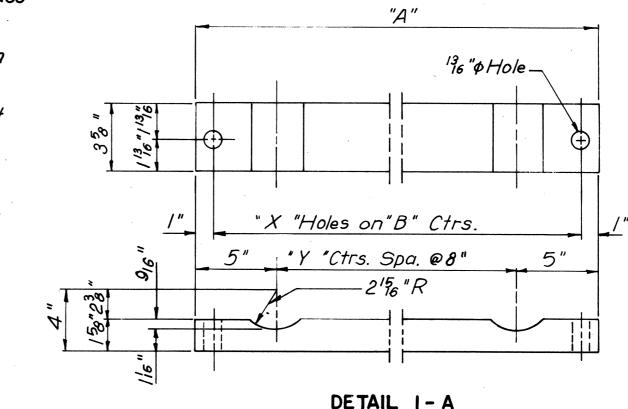


·	BY	DATE	3	As Built	JRC	3-73
MADE	A.P.	11-67	2	General	H.B.W.	11-70
CHECKED	S.B.P.	11-67	1	Support Spc.	J.L.N.	5-/3-68
IN CHARGE	R.H.W.		NO.	REVISION	BY	DATE



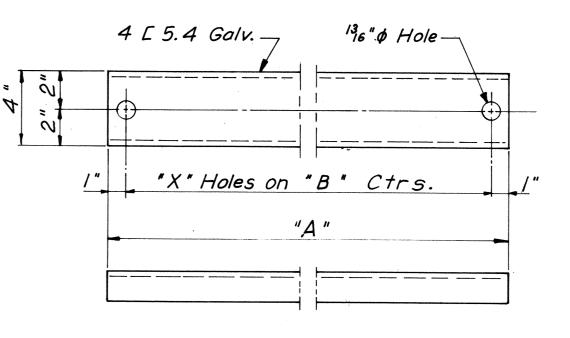
$\bigcirc$	Galvanized steel pipe
2	Pipe Coupling
3	Straight adaptor to connect galvanized steel pipe to cement asbestos conduit.
4	Heavy wall cement asbestos conduit.
(5)	Heavy wall cement asbestos conduit expansion coupling.
6	Heavy wall cement asbestos or plastic tapered coupling.
TELEPH	ONE
$\bigcirc$	Galvanized steel pipe
$\bigcirc$	Pipe coupling
3	Straight adaptor to connect galv. steel pipe to fiberglass

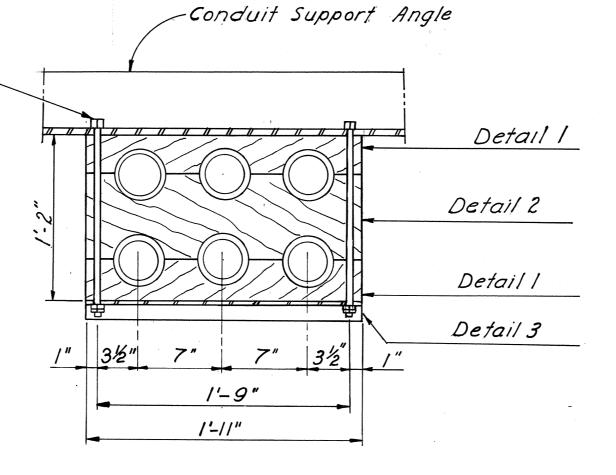
- e steer pipe to fiberglass reinforced cpoxy duct 4 Fiberglass reinforced epoxy duct, 2 layer construction
- (4 pass) with nominal wall thickness of .066 inch 5 Expansion coupling for fiberglass reinforced epoxy duct 6
  - Coupling for fiberglass reinforced epoxy duct



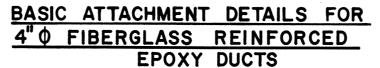


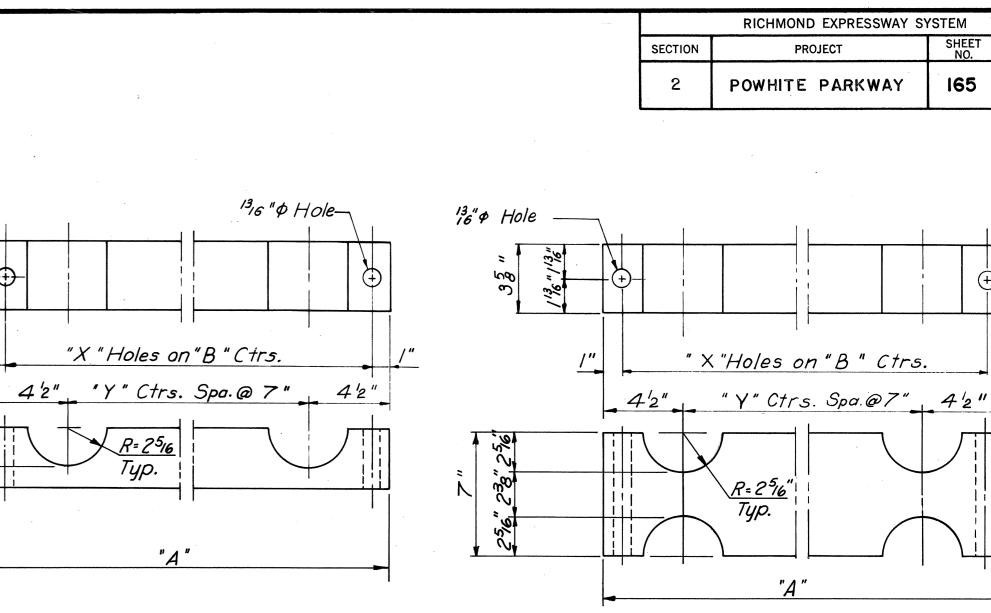
35





Telephone Conduits







DETAIL I-A Wood separator for 5" & cement a sbestos conduit

#### <u>NOTES :</u>

Select conduit lengths so that coupling locations do not coincide with hanger locations. Inside edge of all conduit ends shall be chamfered.

DETAIL 2

Wood separator for 4 " fiberglass reinforced epoxy duct

TOTAL SHEETS

188

(+)

Fittings, in accordance with manufacturers'

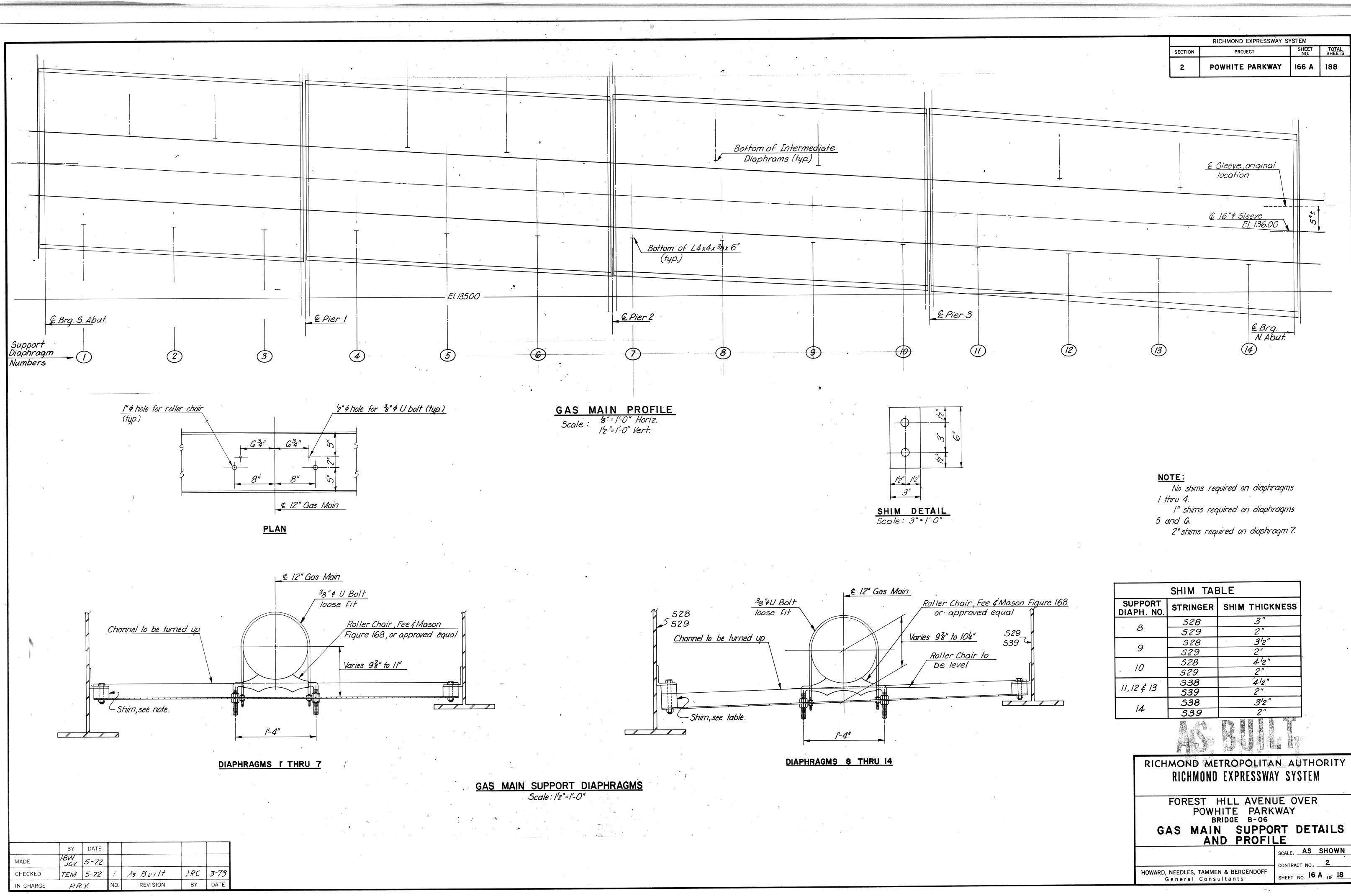
recommendations, shall be used when joining conduits at field-cut joints.

Splicing of steel pipe by welding will not be permitted. Threaded couplings must be used.

Pipe and hardware to be galvanized in accordance with provisions of AASHO MIII (ASTM A123) Specifications. Nuts on all hanger bolts or rods are to be tightened to a snug assembly only and locked. Threads are to be crimped.

Wood separators shall be made from treated Southern Yellow Pine, Douglas Fir or Western Larch. Treatment to be in accordance with Virginia Department of Highways Road and Bridge Specifications, 1970.

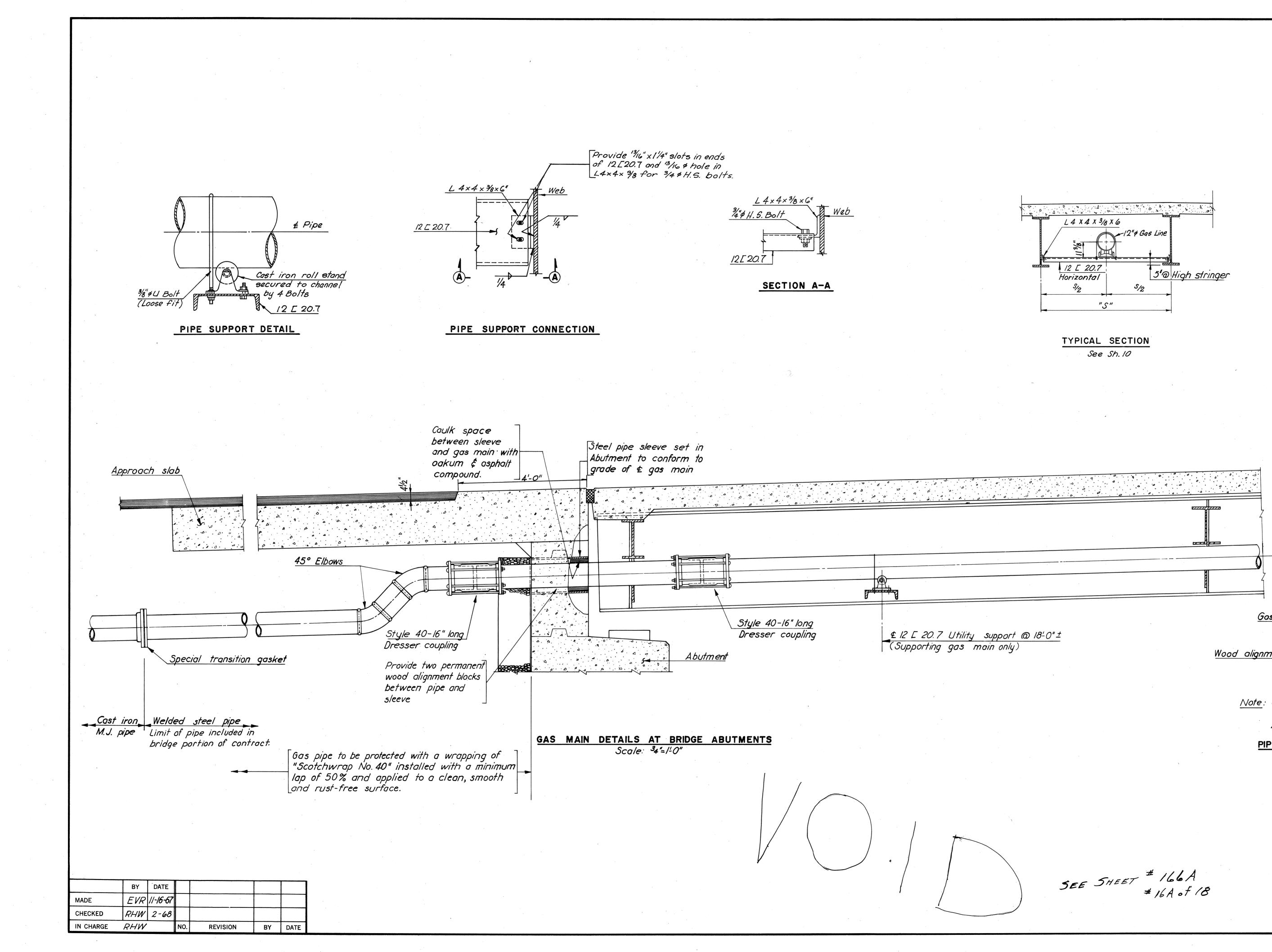
RICHMOND METROPOLITA	N AUTHORITY		
RICHMOND EXPRESSWA	Y SYSTEM		
POWHITE PARKV	VAY		
FOREST HILL AVENUE OVER POWHITE PARKWAY BRIDGE B+06			
CONDUIT INSTALLATIO	ON DETAILS		
HAYES, SEAY, MATTERN & MATTERN Associate Engineers	SCALE: AS SHOWN		
HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants	CONTRACT NO.: <u>2</u> SHEET NO. <u>15</u> OF <u>18</u>		



NOTE	•	
No	shims	1
	•	

SHIM TABLE				
SUPPORT DIAPH. NO.	STRINGER	SHIM THICKNESS		
0	528	3"		
8	529	2"		
0	528	3'2"		
9	529	2"		
10	528	4'2"		
. 10	529	2"		
11 12 \$ 12	538	412"		
11,12 <i>¢</i> 13	539	2"		
14	538	312"		
14	539	2"		

FOREST HILL AVENU POWHITE PARK BRIDGE B-06 GAS MAIN SUPPO AND PROFII	WAY RT DETAILS
	SCALE: AS SHOWN CONTRACT NO.: 2
HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants	SHEET NO. 16 A OF 18
	• • • •



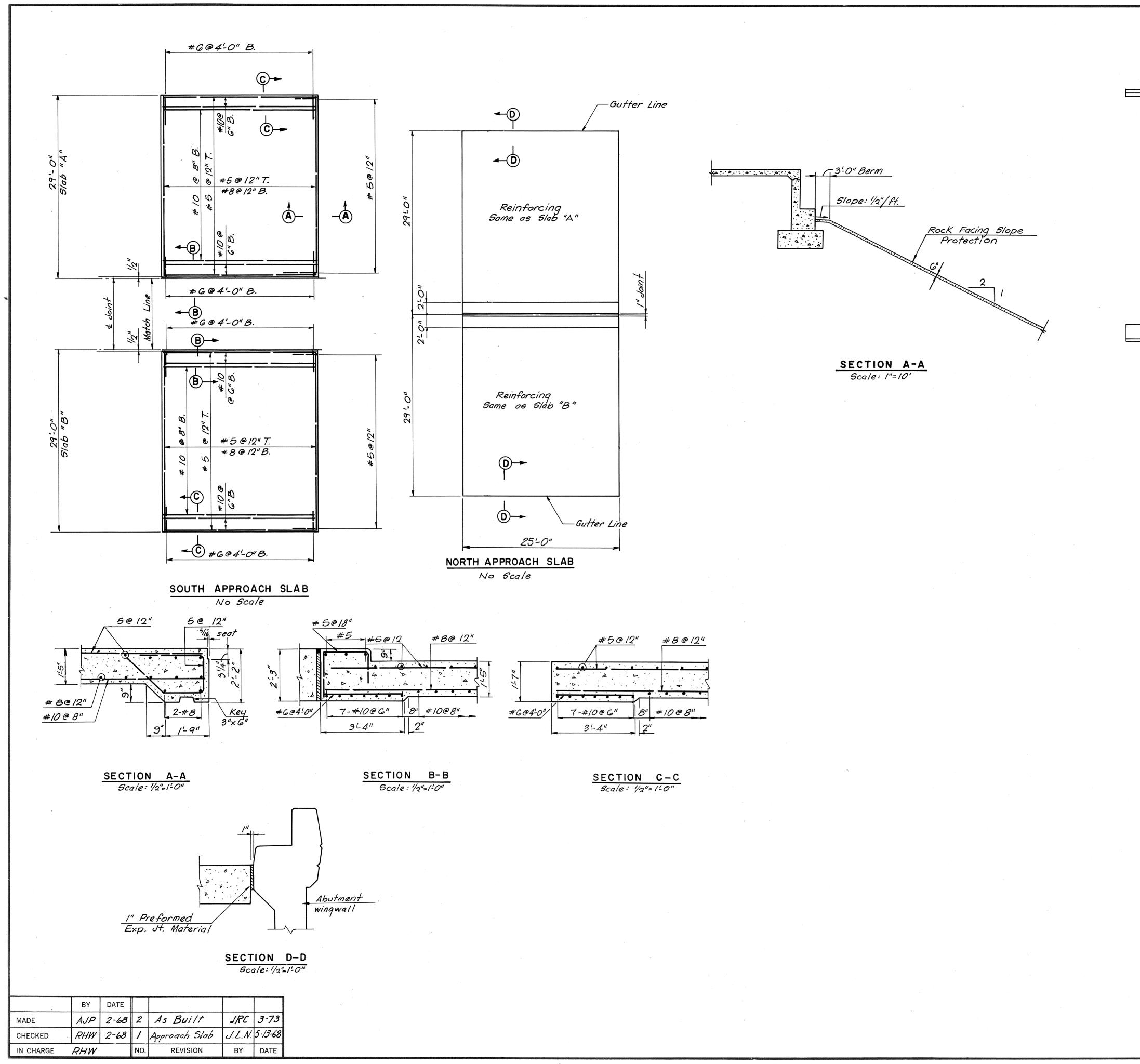
-	RICHMOND EXPRESSWAY SYSTEM			
	SECTION	PROJECT	SHEET NO.	TOTAL SHEETS
	2	POWHITE PARKWAY	166	188

Gas main O.D. ⊈ Gos moin ¢ ⊈ Pipe sleeve Wood alignment blocks

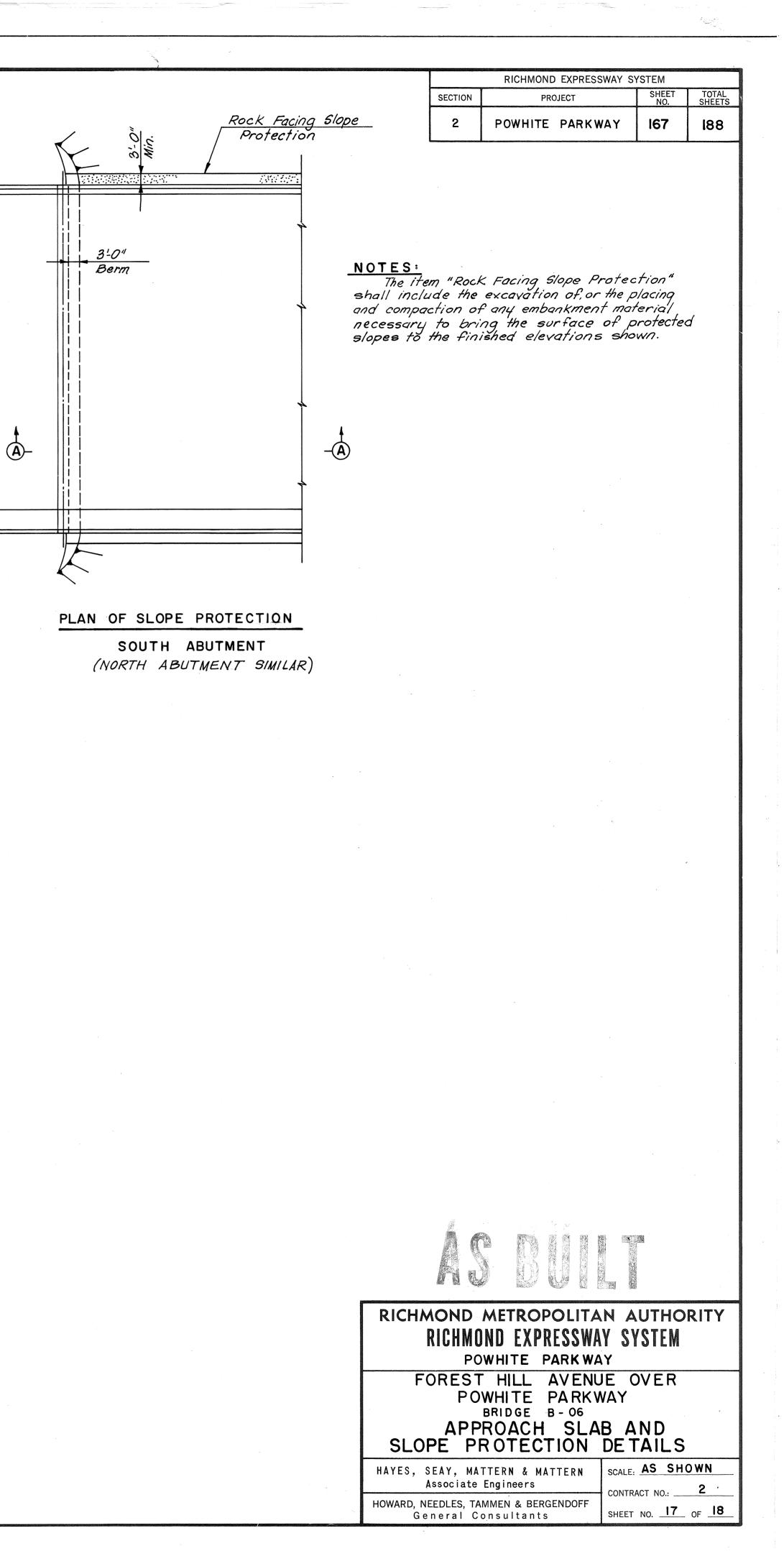
<u>Note</u>: Steel pipe sleeve to be hot-dip galvanized after fabrication in accordance with ASTM A/23

PIPE SLEEVE DETAIL FOR GAS MAIN

RICHMOND METROPOLITAN AUTHORITY RICHMOND EXPRESSWAY SYSTEM POWHITE PARKWAY FOREST HILL AVENUE OVER POWHITE PARKWAY BRIDGE B-06 UTILITY SUPPORT DETAILS AT BRIDGE ABUTMENTS SCALE: AS SHOWN HAYES, SEAY, MATTERN & MATTERN Associate Engineers CONTRACT NO.: 2 HOWARD, NEEDLES, TAMMEN & BERGENDOFF General Consultants SHEET NO. 16 OF 18



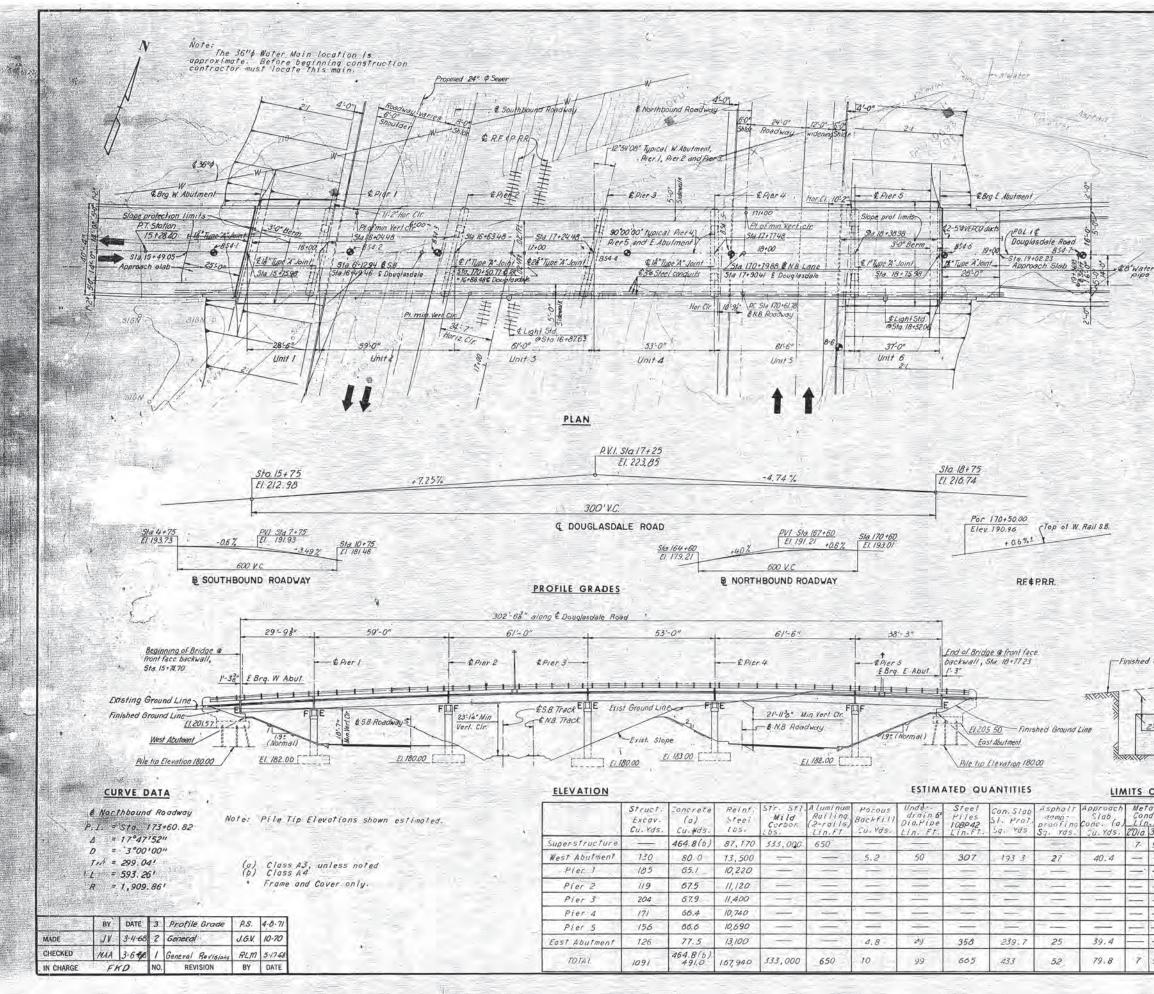
	<u>#5@12"</u> #8@12"
/"/	7-#10@G" 8" #10@8" 3'-4" 2"



## Bridge 13

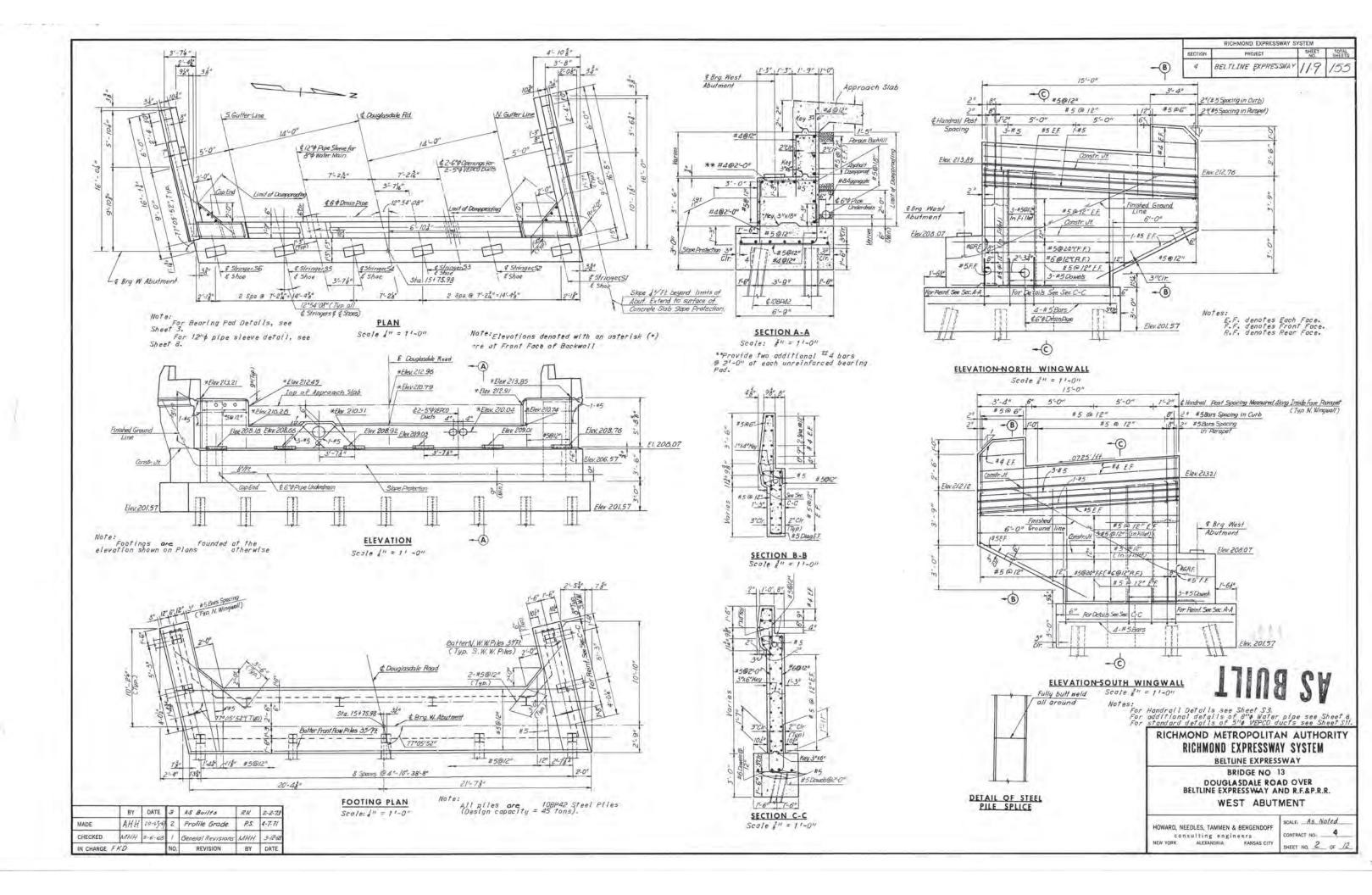
## (Douglasdale Road over I-195 Connector / Powhite Parkway - VA State Rte. 76 and CSX Railroad)

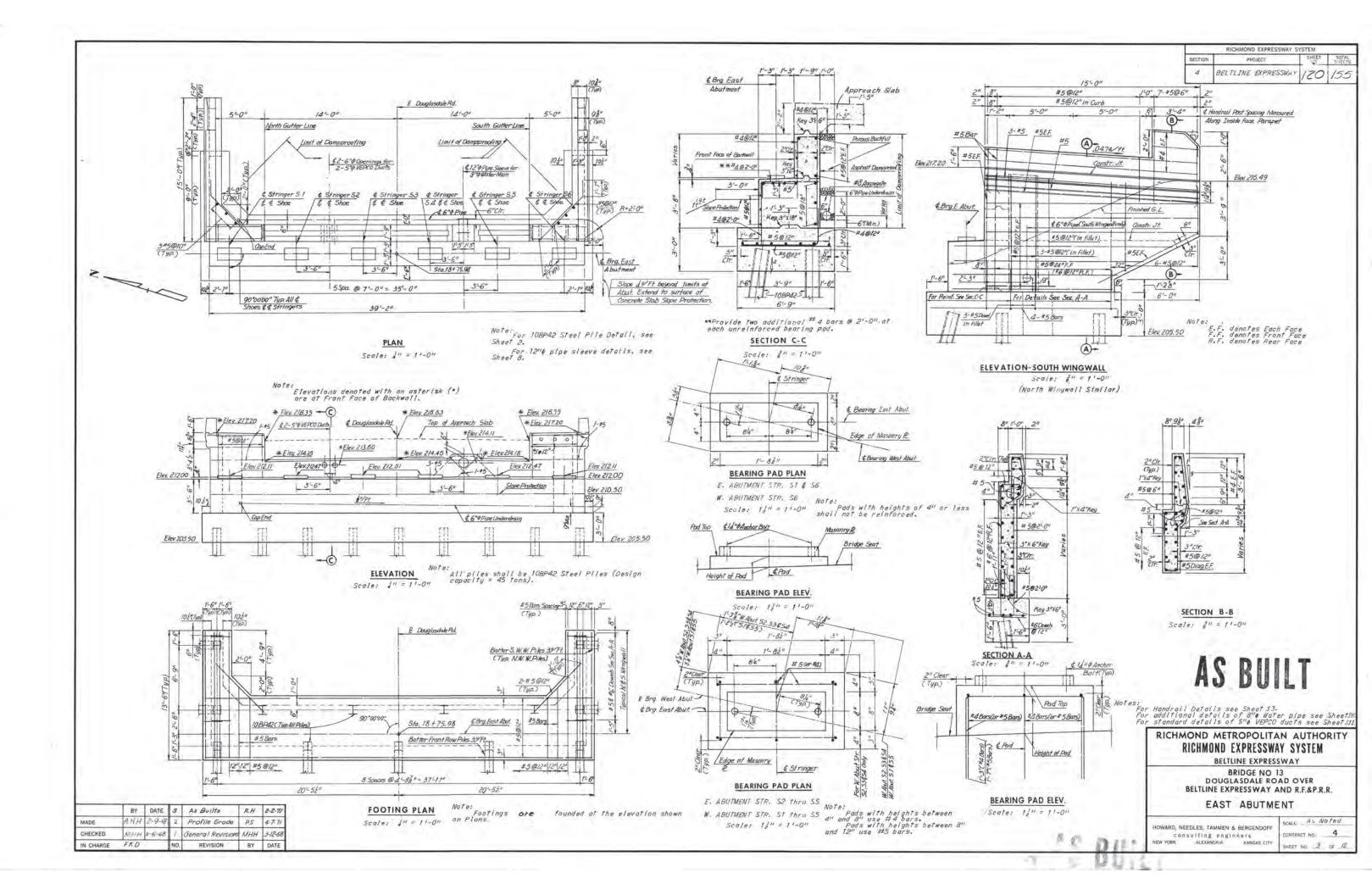
**Record Set Plans** 

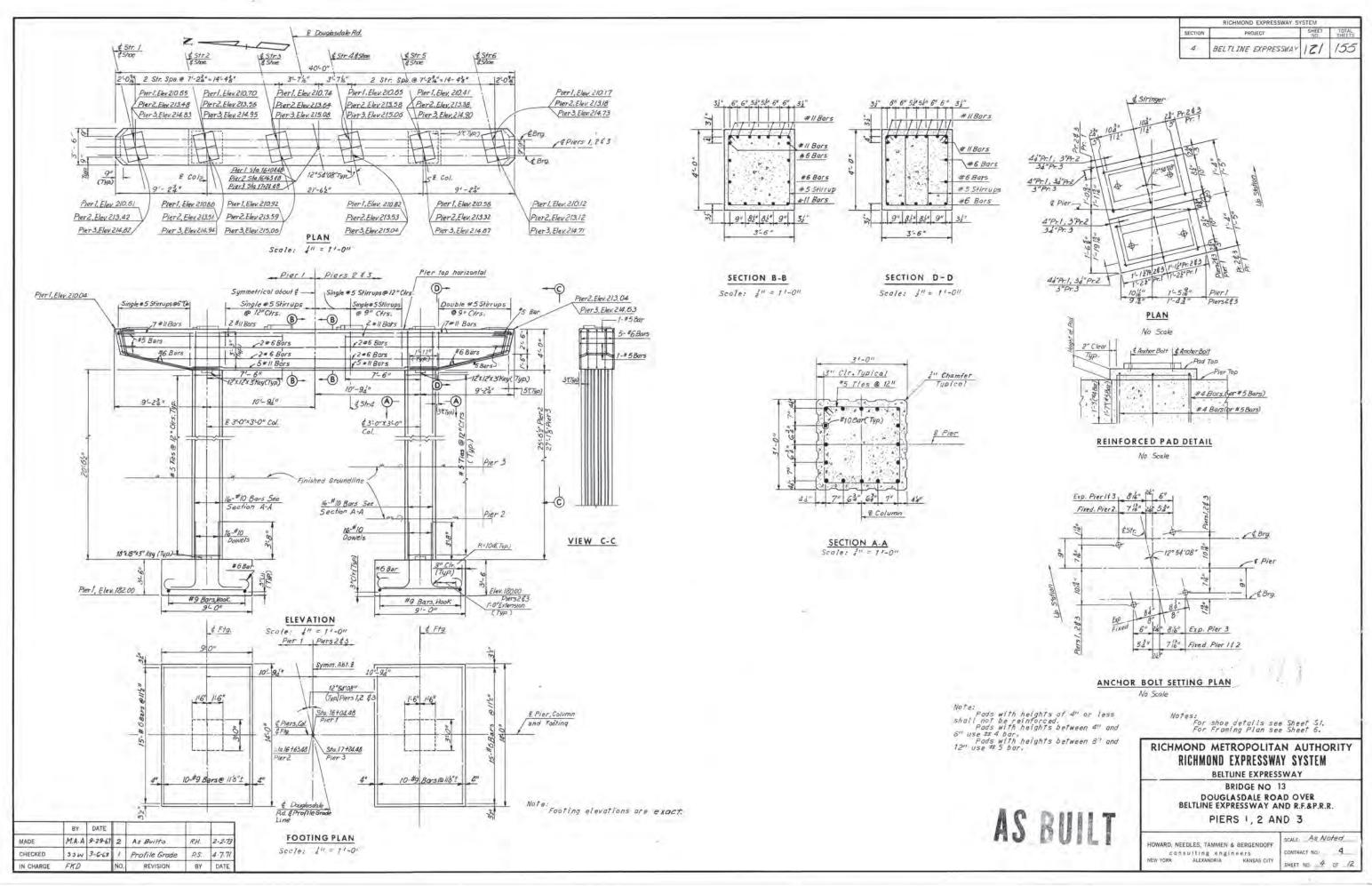


-						
	1			RICHMONO EXPRESSWAY SYSTEM SECTION PROJECT TOTAL SECTION PROJECT TOTAL		
				4. BELTLINE EXPRESSIONAY /18 155		
	GENERAL I	VOTES				
	ROADWAY: CAPACITY		One 20'-O" clear roadway. Two 5'-O" sidewalks . Dead Load includes 15 ibs. per sq.+ft. for future wearing surface. Live Loads - HS20-44 loading.			
	SPECIFICATIONS: GENERAL: Virginia Department of Highway Road and Brid Specifications 1970. SSIGN: A.A.S.H.O. Standard Specifications for Highwa Bridges 1961, modified buspecial Design Provisions. MELDING: 1869 Standard Specifications for Weided Highway and Raliway Bridges of the American Weided Society.					
	14 		Specifico	SPECIAL PROVISIONS ations and Contract Special Provisions referred ore necessary to make these plans complete.		
	DA TUM:		City of H			
$\overline{d}^{\mu}$	TEMPERA TO	IRE :	The norma The tempe	of temperature referred to on the plan is 68°7. For dure range for movement is 0°F to 120°F.		
r	DIMENSION	IS:	All dimen	nsians are measured horizontally and vertically		
	EXCAVATIO	W:	Excavation be classi above the	thermise noted. In below subgrade and cut slope template shall fied as Structure Excavation. All excavation ise limits shall be classified as Regular in and is not included in the Structural		
	FOUNDATIC	WS:		shall rest on firm material. Foundation shall be dry and special attention is called in 401.05 of General Specifications and to act Special Provisions, concerning preparation itians for footings.		
CONCRETE NOTES;			Concrete other conn and corne otherwise the use o be employ placed sl Finishing fectural l	in superstructure shall be Class A4. All crefe shall be Class A3. All exposed edges res shall have a "chamber of filet unless noted. Care in the method of vibration, f low slumb concrete, and or other means shall ed to prevent downgrade mavement of newly ab concrete. Concrete Surfaces. See the Standard Archi- betal Sheets and the Contract Special Fra- or types and details.		
			distance l	orcing steel shall conform to A.S.T.M. A615,Grade 40 orcing bor dimensions on the detailed drawings nfers of bors unless otherwise noted. Elect between reinforcing steel and face of comreie as noted on the plans. All bor logs shall be ers of the smaller digmeter bar unless otherwise		
	STEEL NOT		Structur o tion A36 All field bolts. H otherwise Specific o	l steel shall conform to A.S.T.M. Specifico- except as noted. connections shall be mode with high strength igh strength boils shall be fi diameter unless noted and shall conform to A.S.T.M. fion A 325.		
	LIV CIT MAIN		A-16 Copp Elev. 213	erweld rod N.W. of R.R. Bridge on Douglasdale.		
				denotes 2%"@ Cased hole INDEX Sheet		
	Ground Line       GENERAL PLAN AND ELEVATION       Sheet         Ground Line       GENERAL PLAN AND ELEVATION       1         MEST ABUTMENT       2       2         DIF       CANDA       1         PIERS 4 AND 5       5         FRAMING PLAN       6         DECK PLAN       7         PIERS 4 AND 5       5         FRAMING OFTAILS       6         DOT MIN.       6         DOT MIN.       6         DOT MIN.       7         STANDARD SHOE DETAILS       10         BORING LOCS       11         STANDARD SHOE DETAILS       12         STANDARD SHOE DETAILS       13         STANDARD SHOE DETAILS       14         BORING LOCS       14         STANDARD SHOE DETAILS       15         STANDARD SHOE DETAILS       14         STANDARD COCCUMUNT RAILING DETAILS       54         STANDARD COCS       54         STANDARD COCONDUIT TINSTALLATION DETAILS       54         STANDARD CONDUIT INSTALLATION DETAILS       511					
duit	Sonduit 5"Dia VEP 20	Water Main 8Dia	Junction			
3"Dia.	lin. Et.	Lin.Ft.	BOX LOS	RICHMOND METROPOLITAN AUTHORITY		
914	704	358	360	RICHMOND EXPRESSWAY SYSTEM		
22	-	n	-	BELTLINE EXPRESSWAY BRIDGE NO 13		
4				DOUGLASDALE ROAD OVER		
-				BELTLINE EXPRESS WAY AND R.F.&P.R.R.		
-		10	1244	GENERAL PLAN AND ELEVATION		
914	704	358	360	HOWARD, NEEDLES, TAMMEN & BERGENDOFF consulting engineers CONTRACT NO. 9. New YORK ALEXANDRIA KANSAS, CITY SHEET NO. 7. OF 72		
1	1110000		1	CONCELLING OF THE ASS		

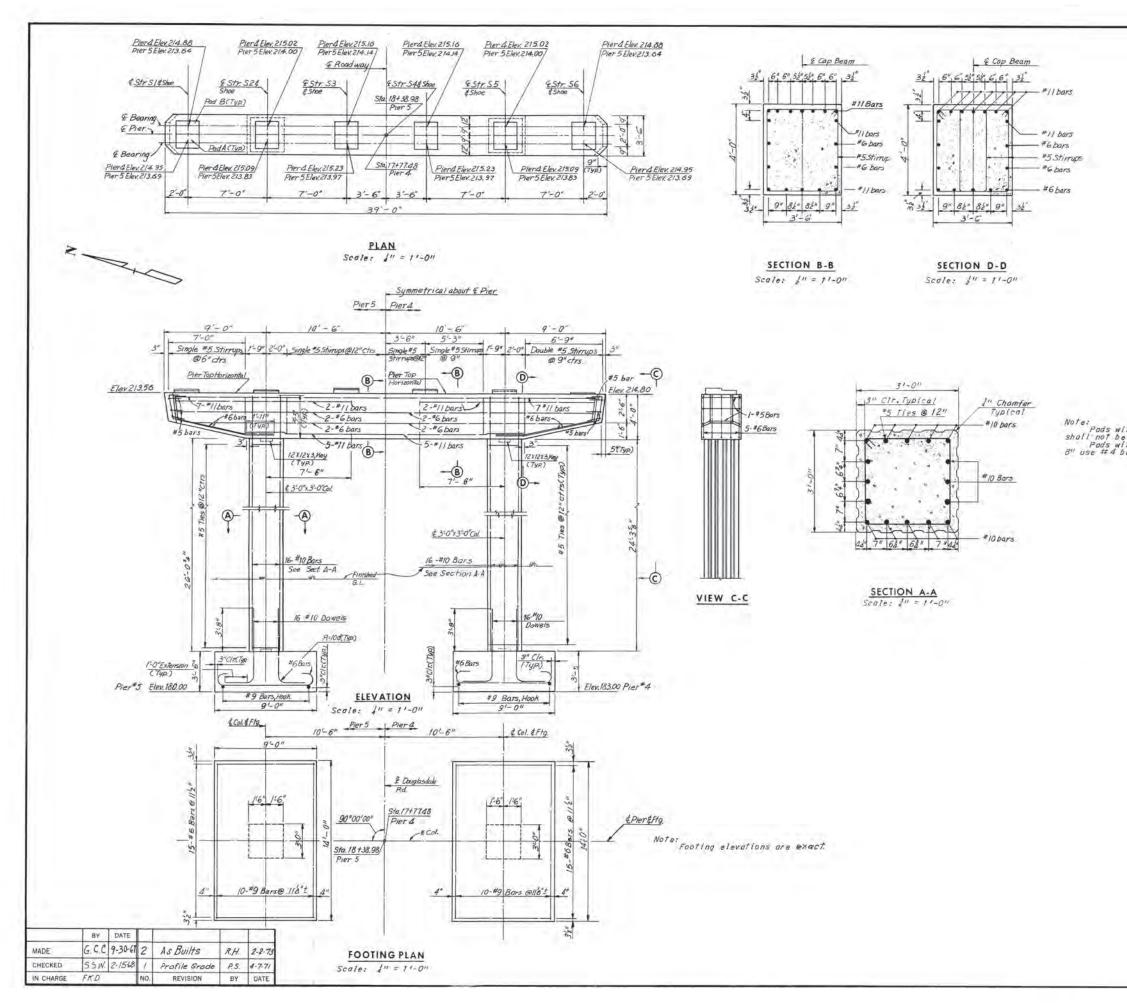
报告 消息

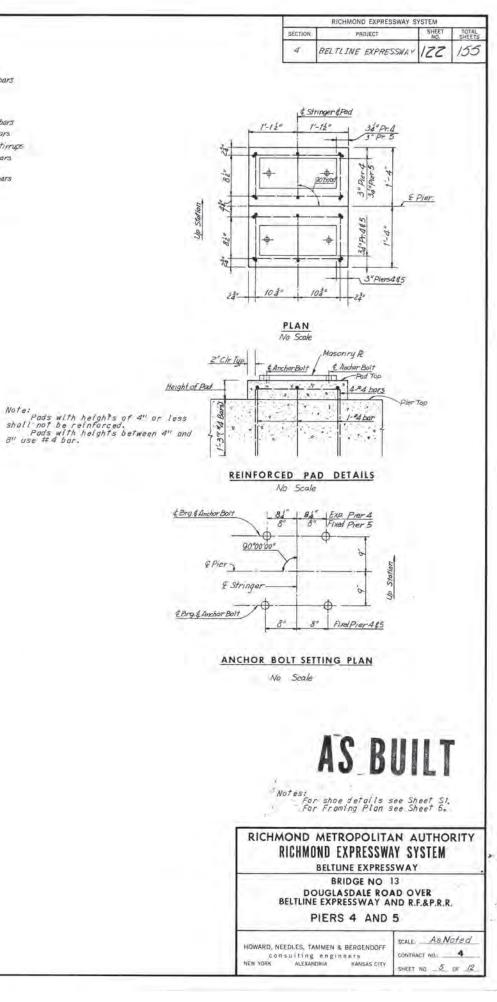


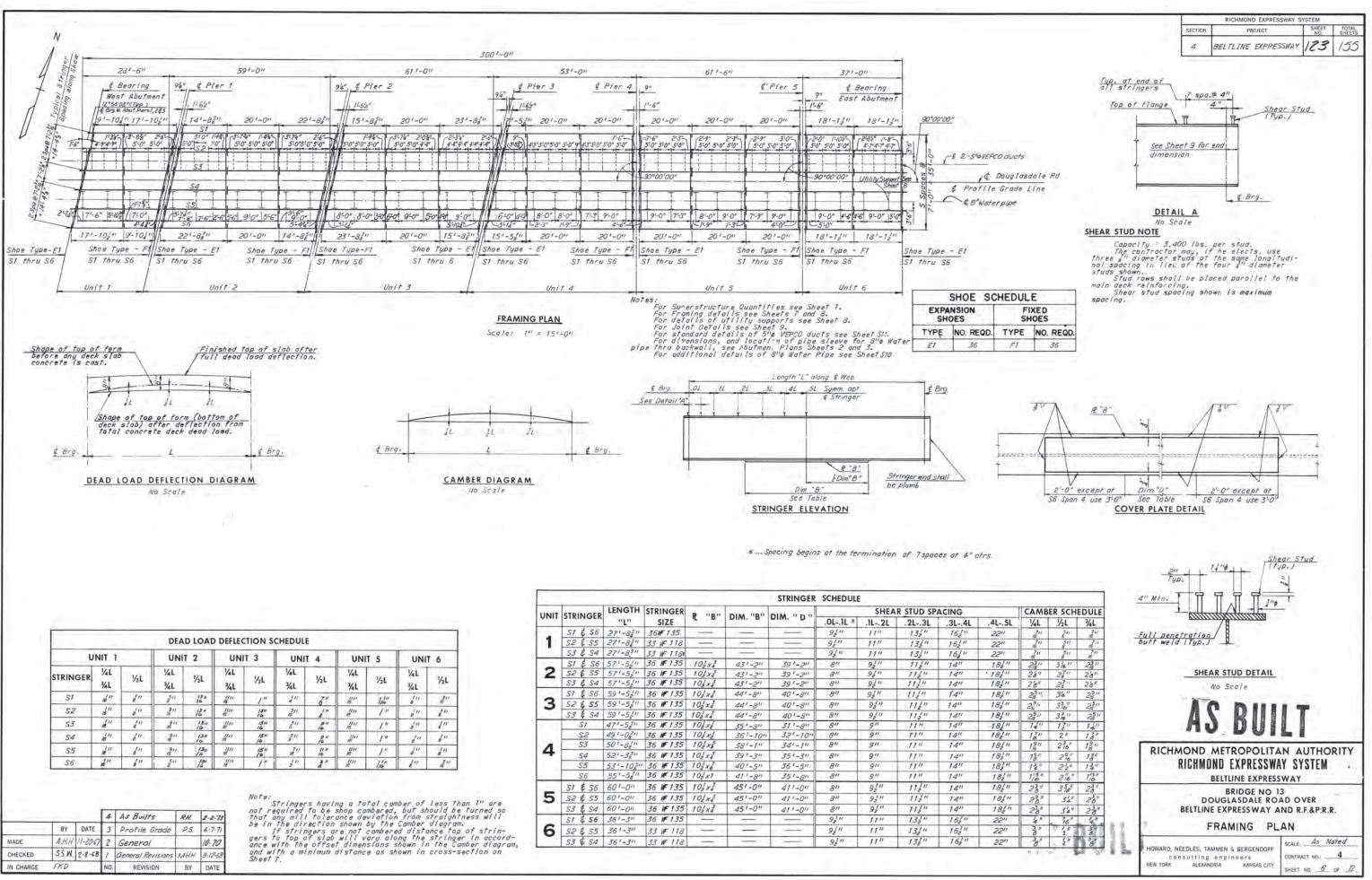




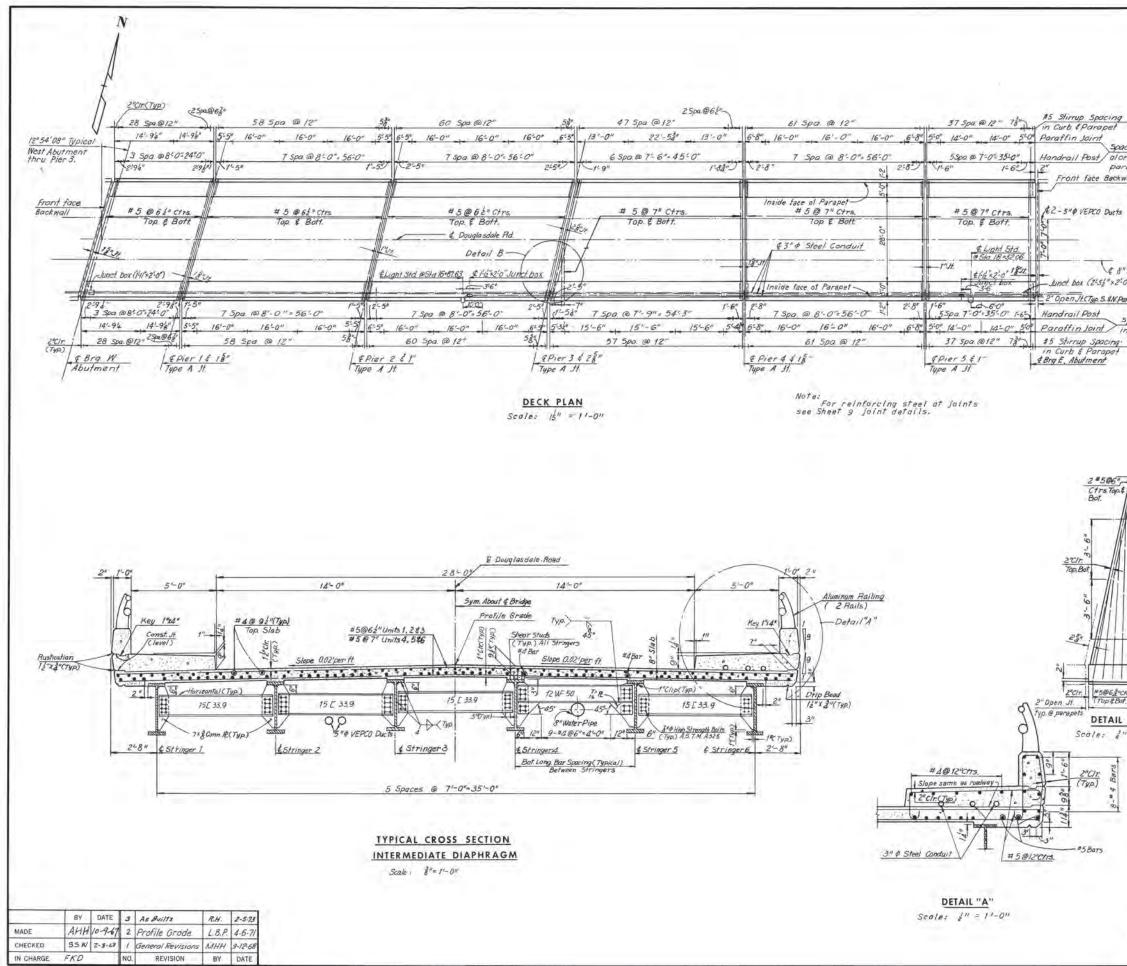
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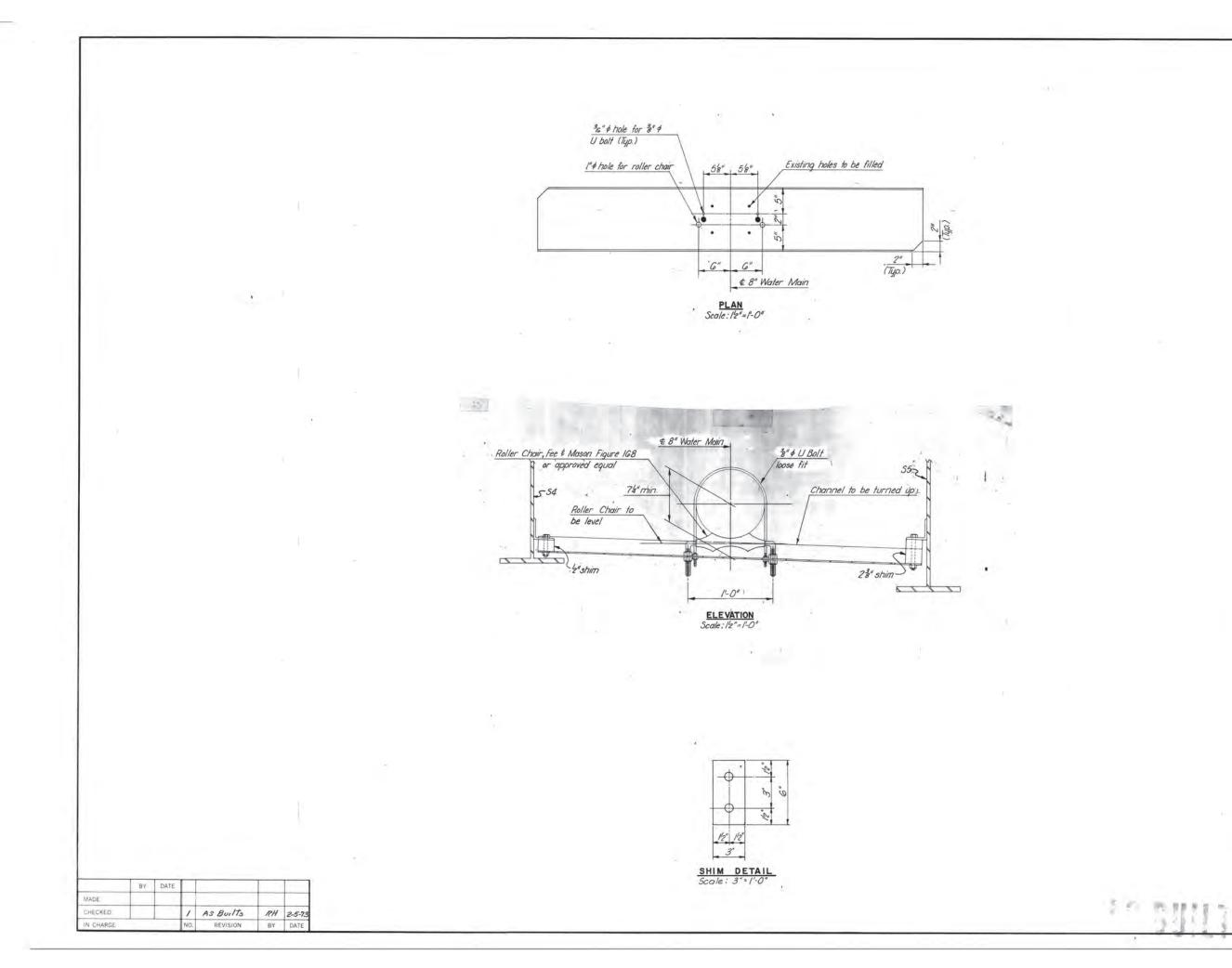




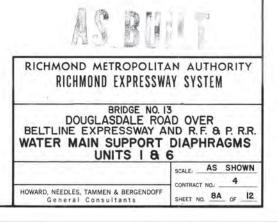
			4	As Builts	RH.	2-2-73
	BY	DATE	.3	Profile Grade	P.S.	4.7.71
MADE	A.H.H	11-20-67	2	General		10.70
CHECKED	5.5.W.	2-8-68	1	General Revisions	MHH	3-12-68
IN CHARGE	FKD	1.1	NO.	REVISION	BY	DATE

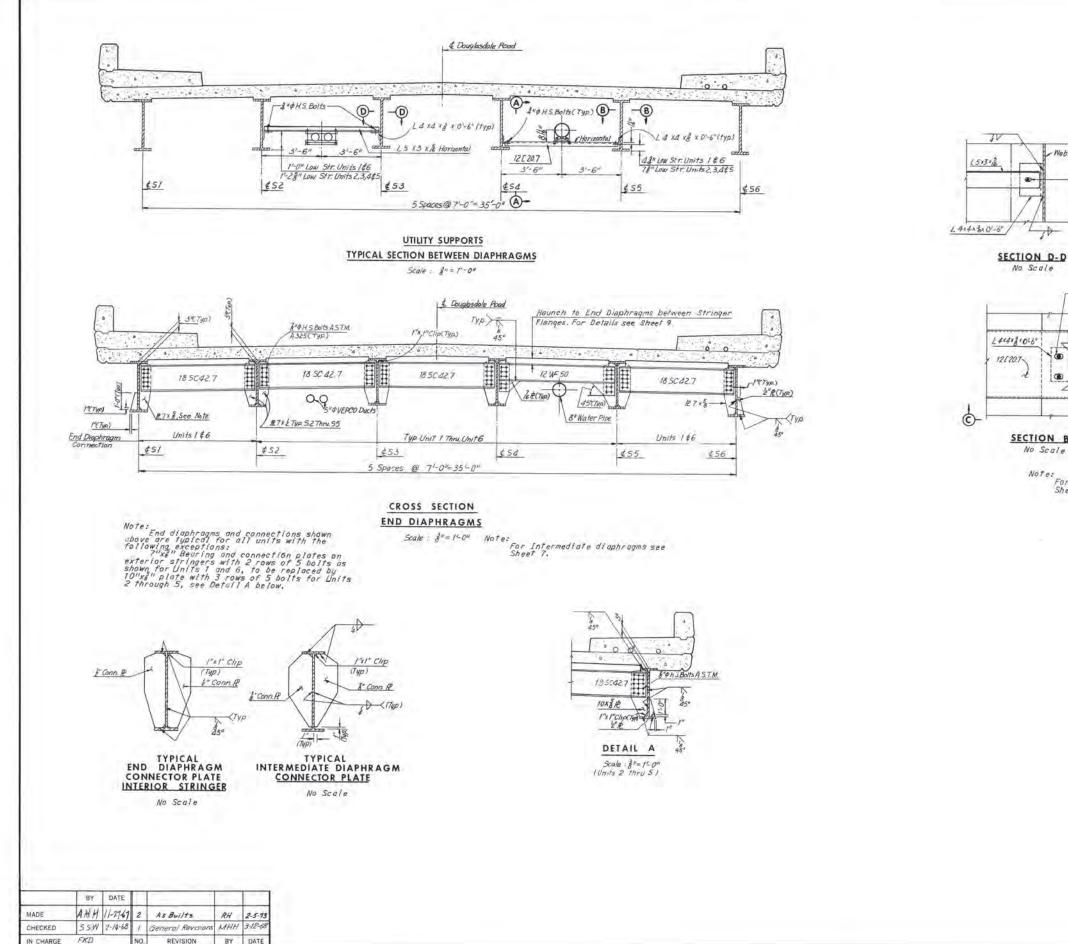


	RICHMOND EXPRESSWAY SYSTEM SECTION PROJECT SHEET TOTAL NO. SHEETS
	4 BELTLINE EXPRESSWAY 124 155
	100
	TABLE OF ELEVATIONS
	STATION ELEV. A ELEV. B
	<u>15+50.00</u> 211.17 210.89 60.00 211.89 211.61
	70.00 212.62 212.34
g measured	71.49 212.45
inside face <u>Symmetrical about</u> Douglasdale Road.	ETERSO -
Elev B	Elex. A 80.00 213.34 213.06
0.02%	90.00 214.03 213.75
Contract of the second s	16+00.00 214.67 214.39
	01.28 214.47
GUTTERLINE ELEVATION DI	07.60
Sofference Elevation Di	10,00 215.28 215.00
ter Pipe.	20.00 215.84 215.56 30.00 216.37 216.09
ier pipe	<u>30.00</u> 216.37 216.09 40.00 216.85 216.57
et)	50.00 217.30 217.02
<u>ur &gt;</u>	50.00 217.70 217.42
icing measured along de face Parapet	<u> </u>
as face for apor	66.69 217.67
	70.00 218.07 217.79
	80.00 218.40 218.12 90.00 218.68 218.40
	17+00.00 218.93 218.65
	10.00 219.13 218.85
	20.00 219.30 219.02 21.28 — 219.03
	21.28 - 219.03 24.48 219.36 - 219.03
	27.69 - 219.12
	<u>30.00</u> 219.42 219.14 40.00 219.51 219.25
ter ter	40,00 219,51 219,23 50,00 219,55 219,27
<del>1.</del>	60.00 219.56 219.28
10	70.00 219.52 219.24
N restringers	77.48 219.47 219.19 80.00 219.45 219.17
	90.00 219.33 219.05
il l	18+00.00 219.18 218.90
	10.00 218.99 218.71
10 m	20.00 218.75 218.47 30.00 218.48 218.20
	38.98 218.20 217.92
	40.00 218.16 217.88
1 Manual 1	50.00 217.81 217.53 60.00 217.42 217.14
A Stringer 6	70.00 216.97 216.69
1-60	77.23 216.63 216.35
00	80.00 216.50 216.22 90.00 216.03 215.75
	19+00.00 215.55 215.27
- To	
<u>2-5</u> *	
1'-0" Note	
	For Superstructure Quantifies see Sheet 1. For Graming and Utility support details see Sheet 8.
	For Joint details see Sheet 9. For Lighting details see Sheet S4. For details of transmission ducts see Sheet S4.
	For details of transmission ducis see Sheef 34.
	RICHMOND METROPOLITAN AUTHORITY
	RICHMOND EXPRESSWAY SYSTEM
	BELTLINE EXPRESSWAY
	BRIDGE NO 13
	DOUGLASDALE ROAD OVER
	BELTLINE EXPRESSWAY AND R.F.&P.R.R.
B 60 5000 10.0 0.0 0.0	DECK PLAN
A V VIZ BUBLI T	SCALE. As Noted
A REAL PROPERTY AND A REAL PROPERTY A REAL PROPERTY AND A REAL PRO	
ASSBUILT	HOWARD, NEEDLES, TAMMEN & BERGENDOFF CONSULTING ENGINEETS NEW YORK ALEXANDRIA KANSAS CITY



RICHMOND EXPRESSWAY SYSTEM					
SECTION	PROJECT	SHEET NO:	TOTAL		
4	BELTLINE EXPRESSIVAY	11.51	1		

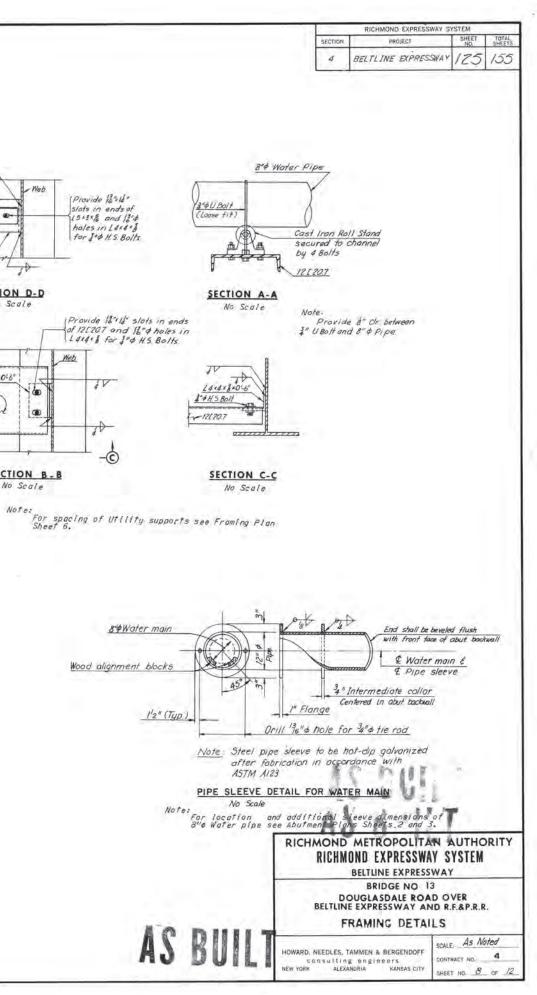


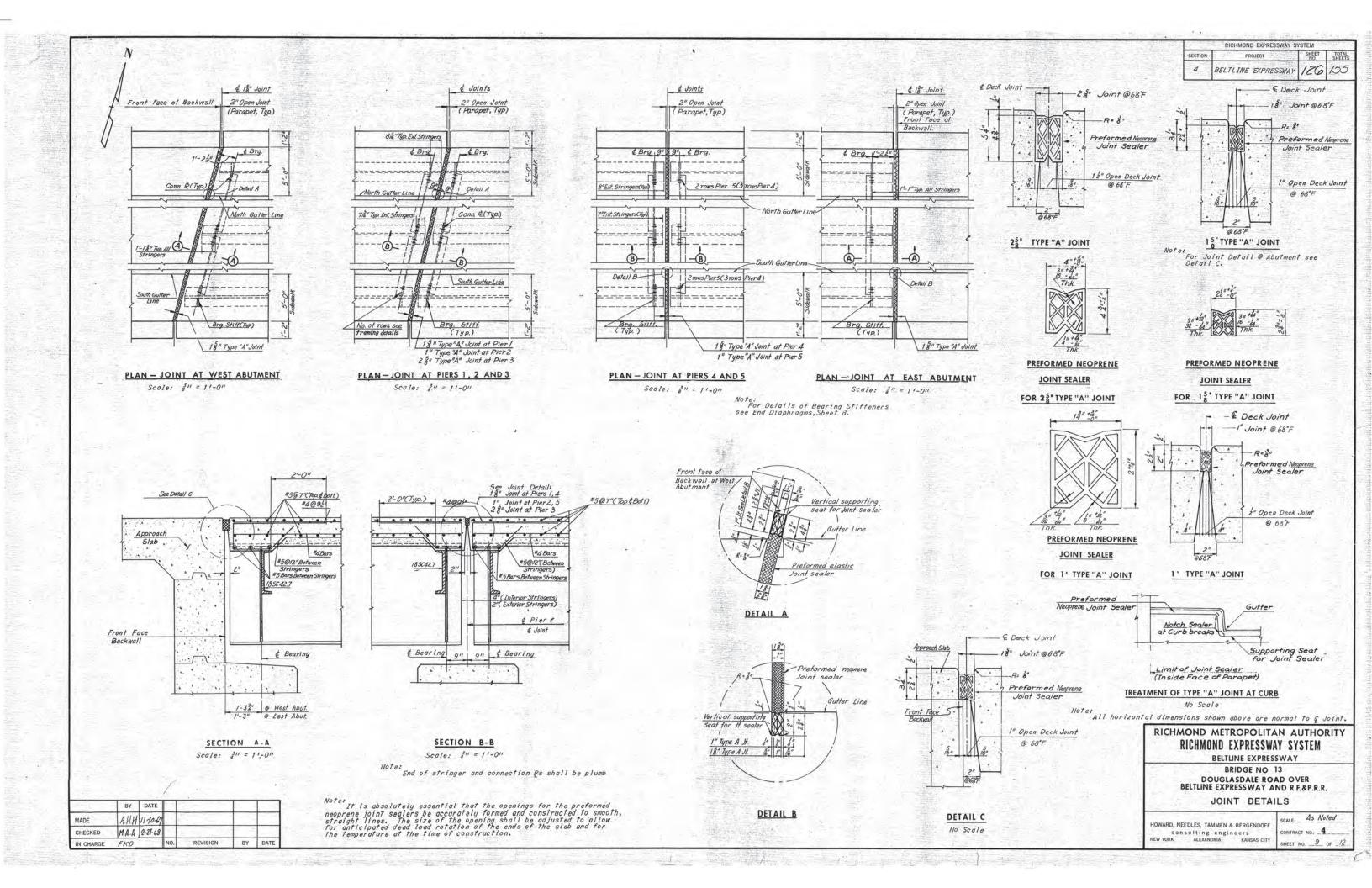


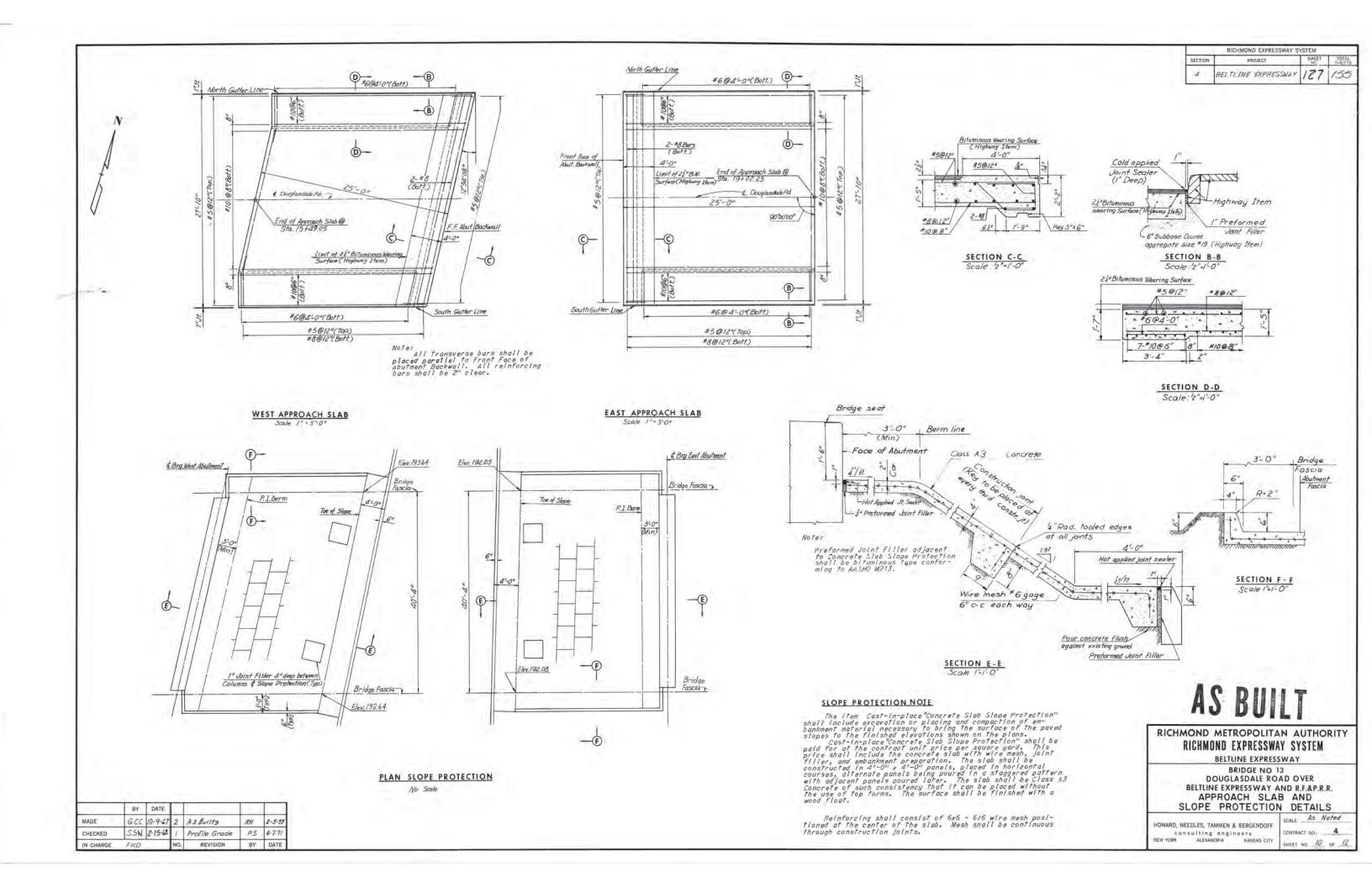
SECTION B.B No Scale

λa. 0

Web



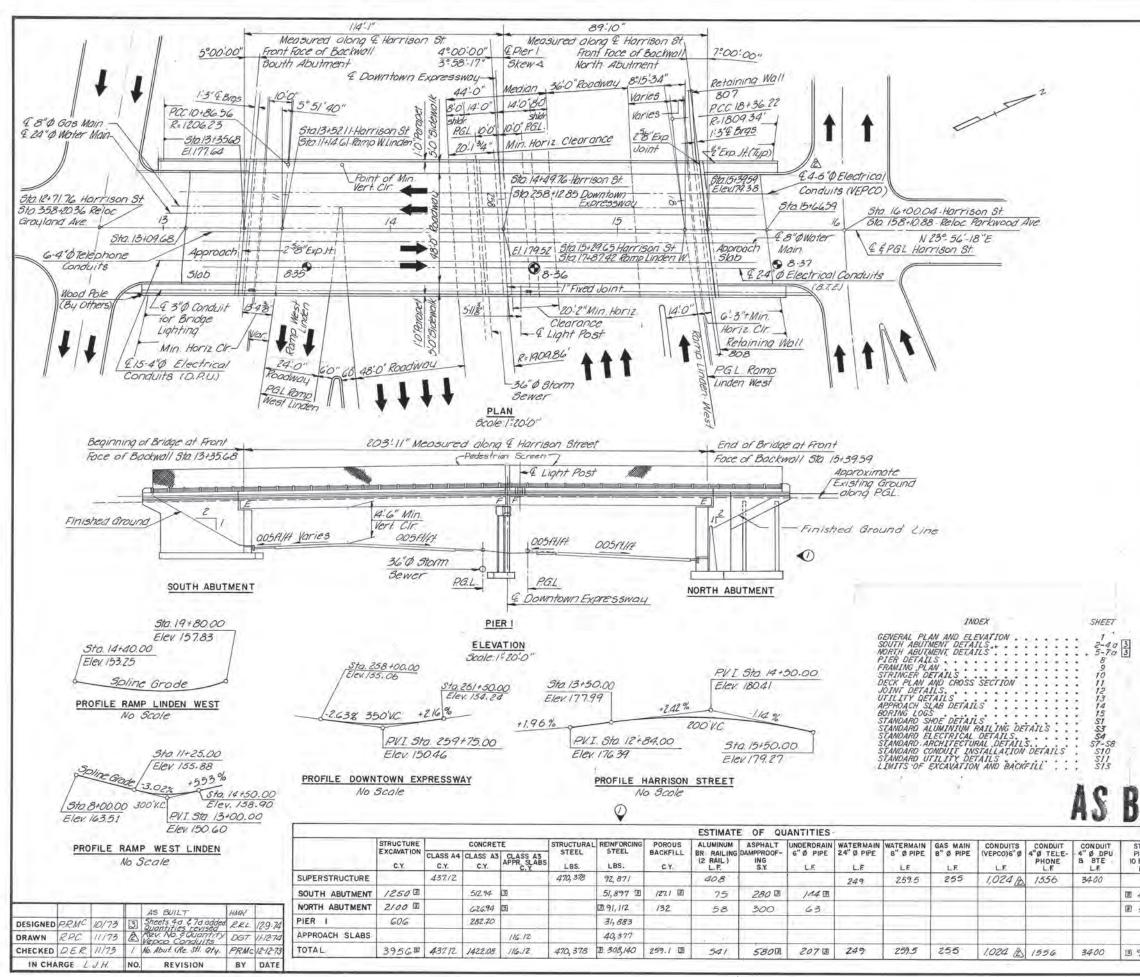




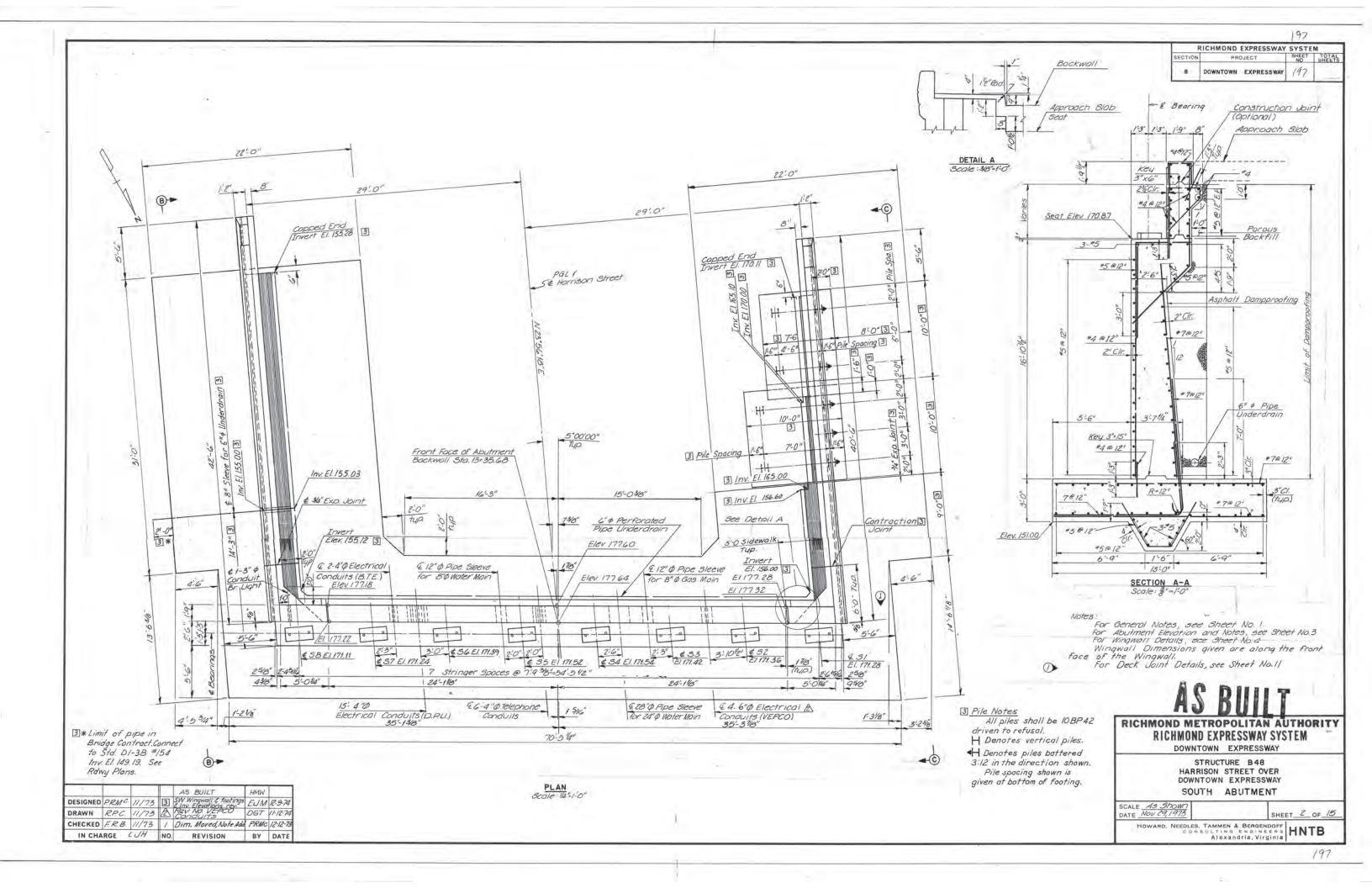
## Bridge 48

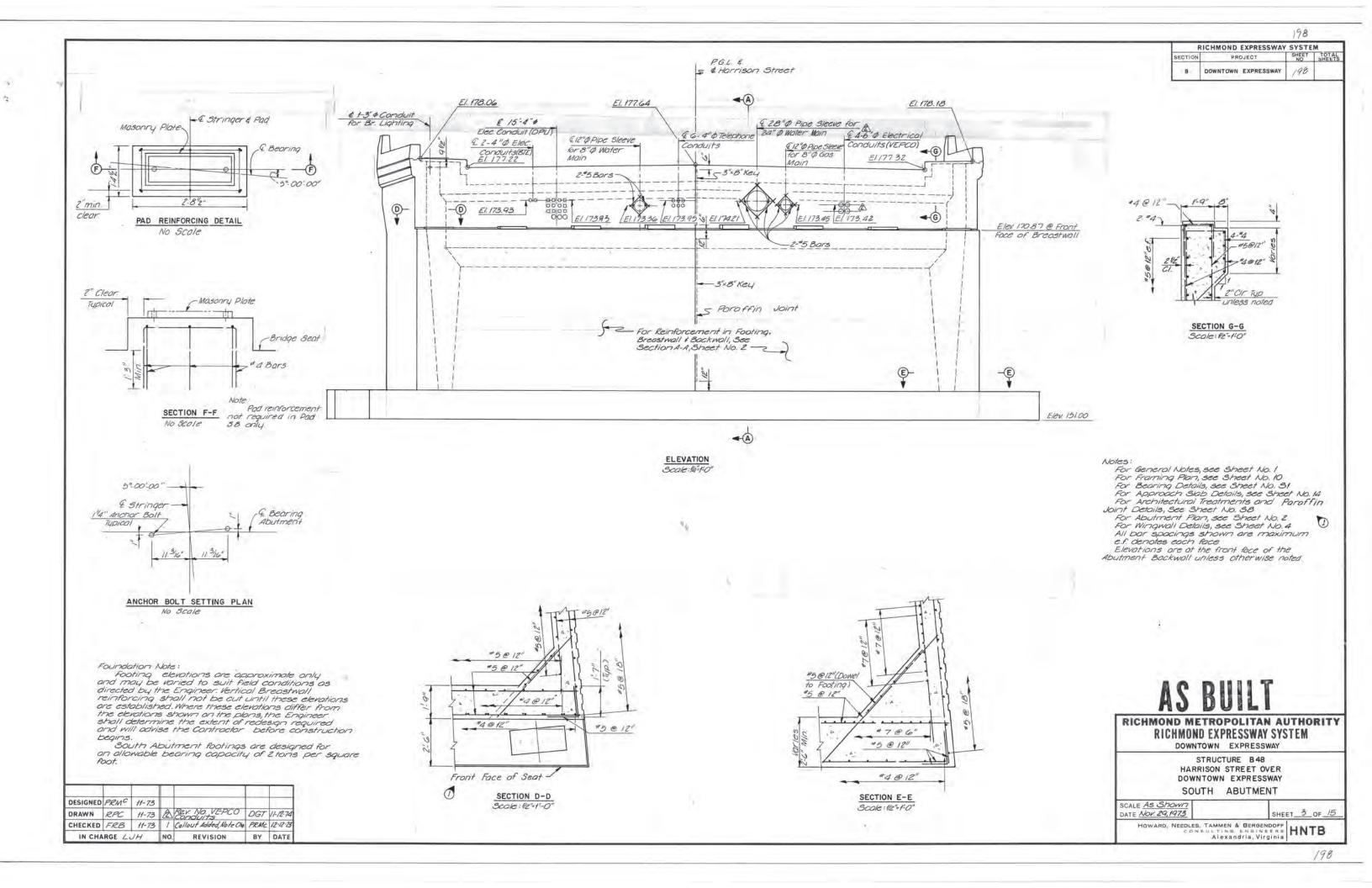
(Harrison Street over Downtown Expressway - Rte. 195)

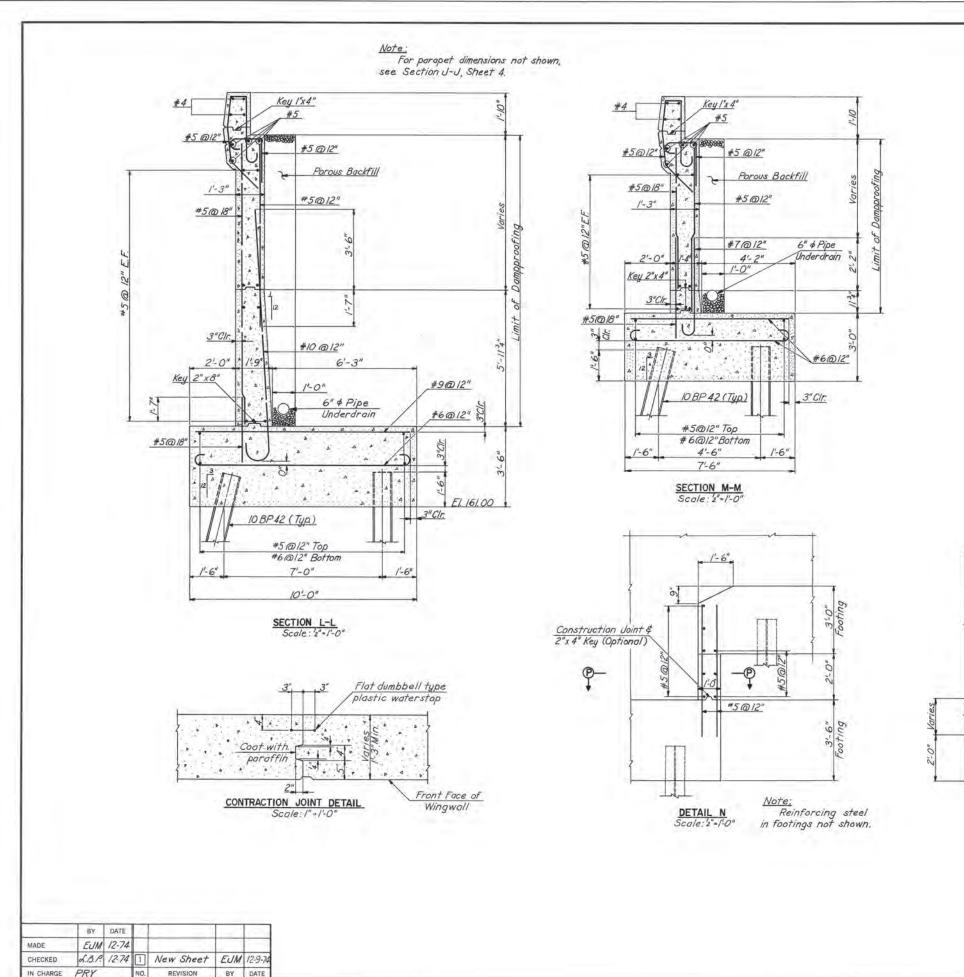
**Record Set Plans** 



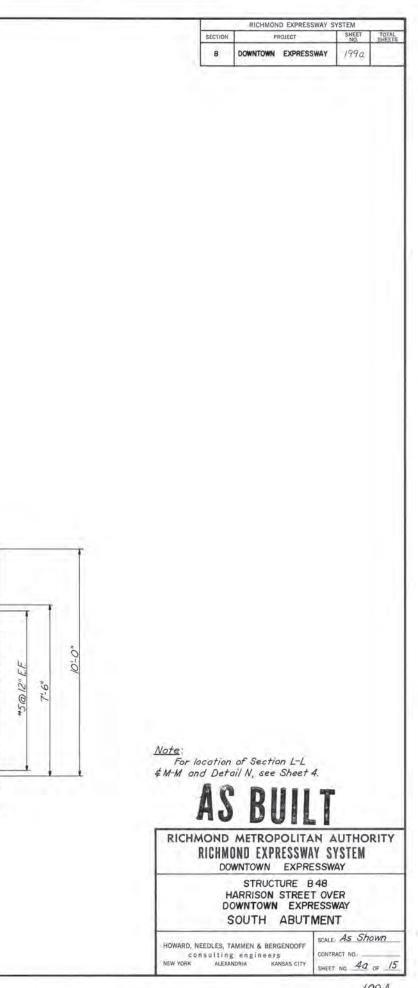
			1	RICHMOND EXPRESSWA	196 Y SYSTE	M			
			SECTION	PROJECT	SHEET	TOTA			
			8	DOWNTOWN EXPRESSWAY	196				
					1				
CENEDA	MOTES								
GENERAL ROADWAY		One 48'-0" -1	ear roa	dway. Two 5'-0" sid	lewalks				
CAPACIT		Dead Load - 1	neludes	15 lbs. ner so. for	t for				
- 10 C		future wearin Live Load H	g surfo \$20-44	ce. ond B.P.R. modified	for mil	1-			
SPECIFI	CATIONS: +	American wera	ginia D ication S.H.O. es, 197 2 Struc ling Soc		is Road lons for of the	an d			
		CONTRACT SPEC							
	-	Specification ferred to abo complete.	s and C ve are	ontract Special Prov necessary to make th	isions lese pla	re- ns			
DATUM :		U.S.C. and G. ment).	S. (Sea	level, 1929 General	Adjust	4			
TEMPERA	TURE :	The normal te 60° F. The t to 120°F.	mperatu emperat	re referred to on th ure range for moveme	ne plans ant is 0	Is F			
DIMENSI	ons:		s ore m	easured horizontally					
EXCAVAT	10N :	Excavation be shall be clas excavation ab as Regular Ex Structural Qu	low sub sified ove the cavatio antitie	grade and cut slope as Structure Excava se limits shall be a n and is not include s.	templot ion. A lassifi d in th	e 11 ed e			
FOUNDAT	IONS:	Factings shall rest on firm material. Foundation material shall be kept dry and special attention is called to section 401.05 of the General Speci- fications, and to the Contract Special Provisions, concerning preparation of foundations for footings.							
CONCRET	E NOTES:			and the second					
		Architectural Special Provi	Detail sions f	Sheets and the Cont or types and details	ract				
-		All reinforci cification Ad dimensions on of bars unles between reinf shall be as n shall be 30 a unless otherw	iameter	I shall conform to A g 40. All reinforci failed plans are to wise noted. Clear of steel and face of co the plans. All bar s of the smaller dio ed.	STM Spe ing bar centers listance nocrete laps ameter b	ar			
STEEL N									
		Structural St cation A36 ex shall be made strength bolt wise noted an A325.	eel sha cept as with h s shall d shall	II conform to ASTM S noted. All field of igh strength bolts. be d'' diameter unit conform to ASTM Spe	Specifi- connecti High ess othe ecificat	ons r- i on			
		All shop weld web to flange arc process.	ed web welds	splices, flange spli shall be made by the	ces and submer	ged			
BENCH M	ARK:	C-34 Monument in walk on the East side of Harr Street between Parkwood Avenue and Cary Street Elev. 179.381.							
NOTES:		Top of Pavement Elevations at ends of deck along P.G.L. are given on the plan, Remaining pavement elevations are given on Sheet (1							
ILT	•			hole boring.					
				TROPOLITAN A		RITI			
PIC CONDUIT 3" Ø L.F.	PEDESTRIAN	RIC	DOWNT	OWN EXPRESSIVAY	STEM				
L.F. 249	L.F. 408	1	S	TRUCTURE 848	-				
271	81		HARR	ISON STREET OVER					
	64	GEN		TOWN EXPRESSWAY	ATION				
		SCALE AS Sh		I CAN AND ELEV	ALION	-			
		14 20	1072			1. 1.1			
249	553	DATE NOV 29,		TAMMEN & BERGENDOFF		DF_/5			



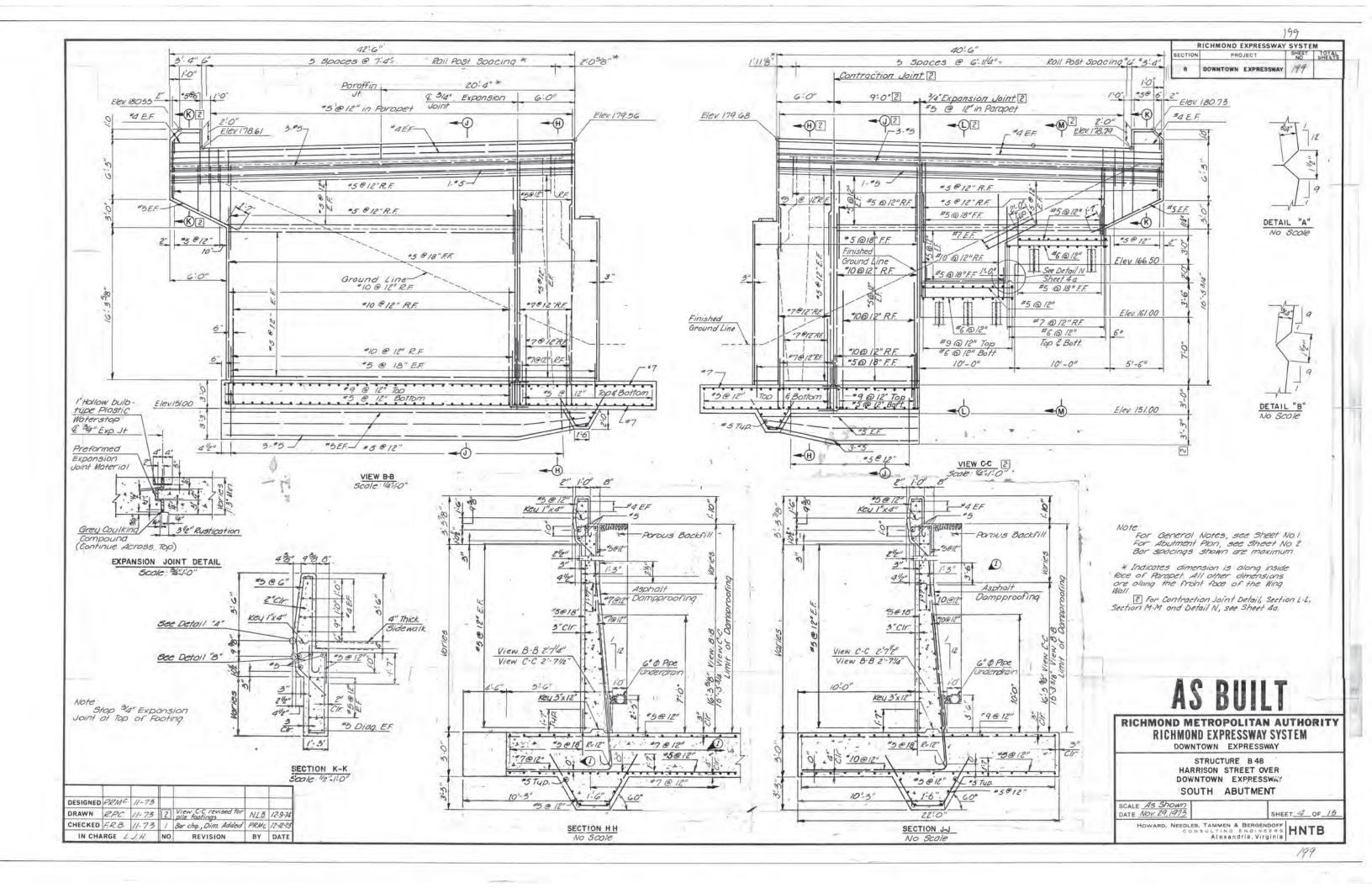


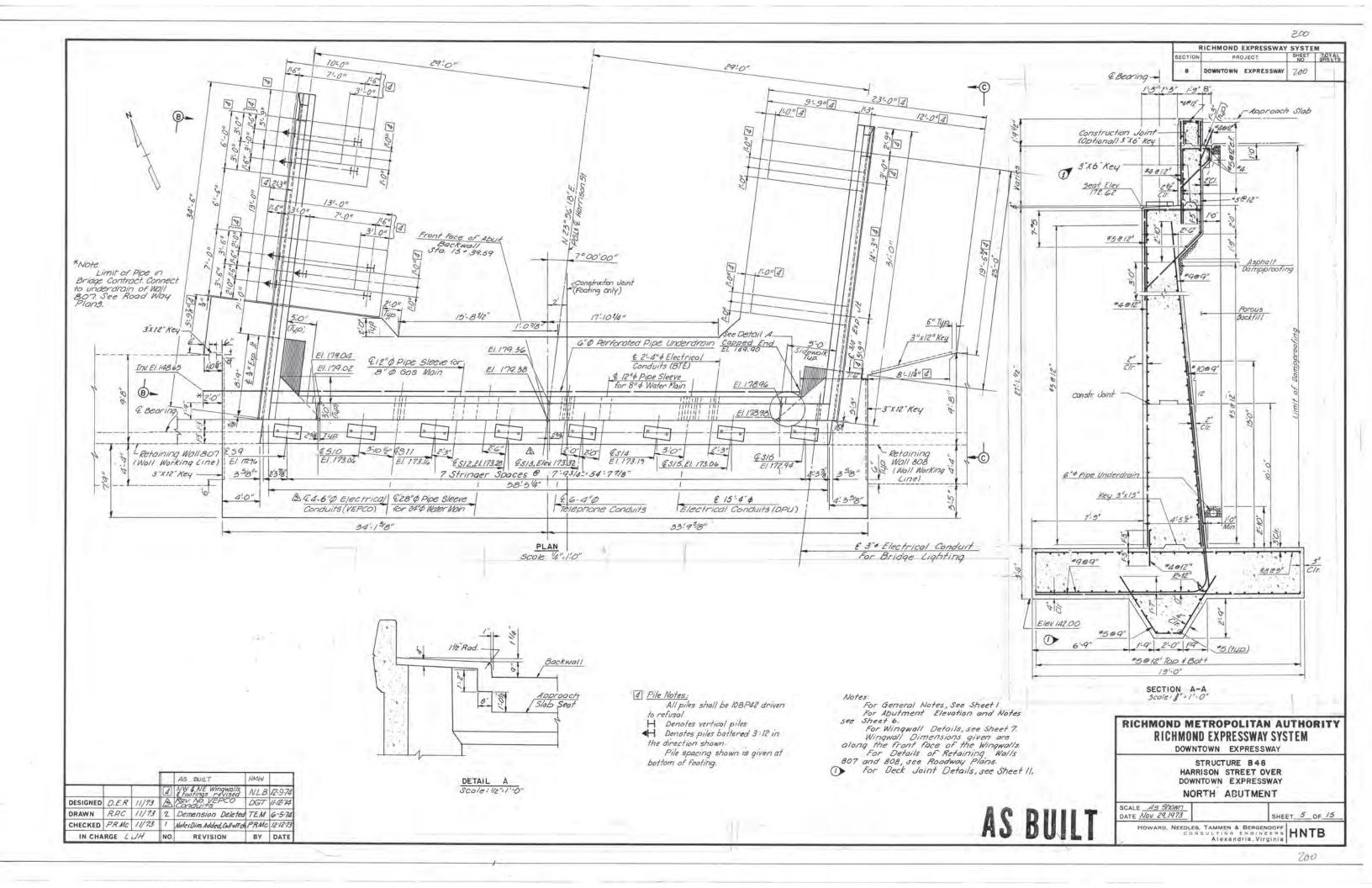


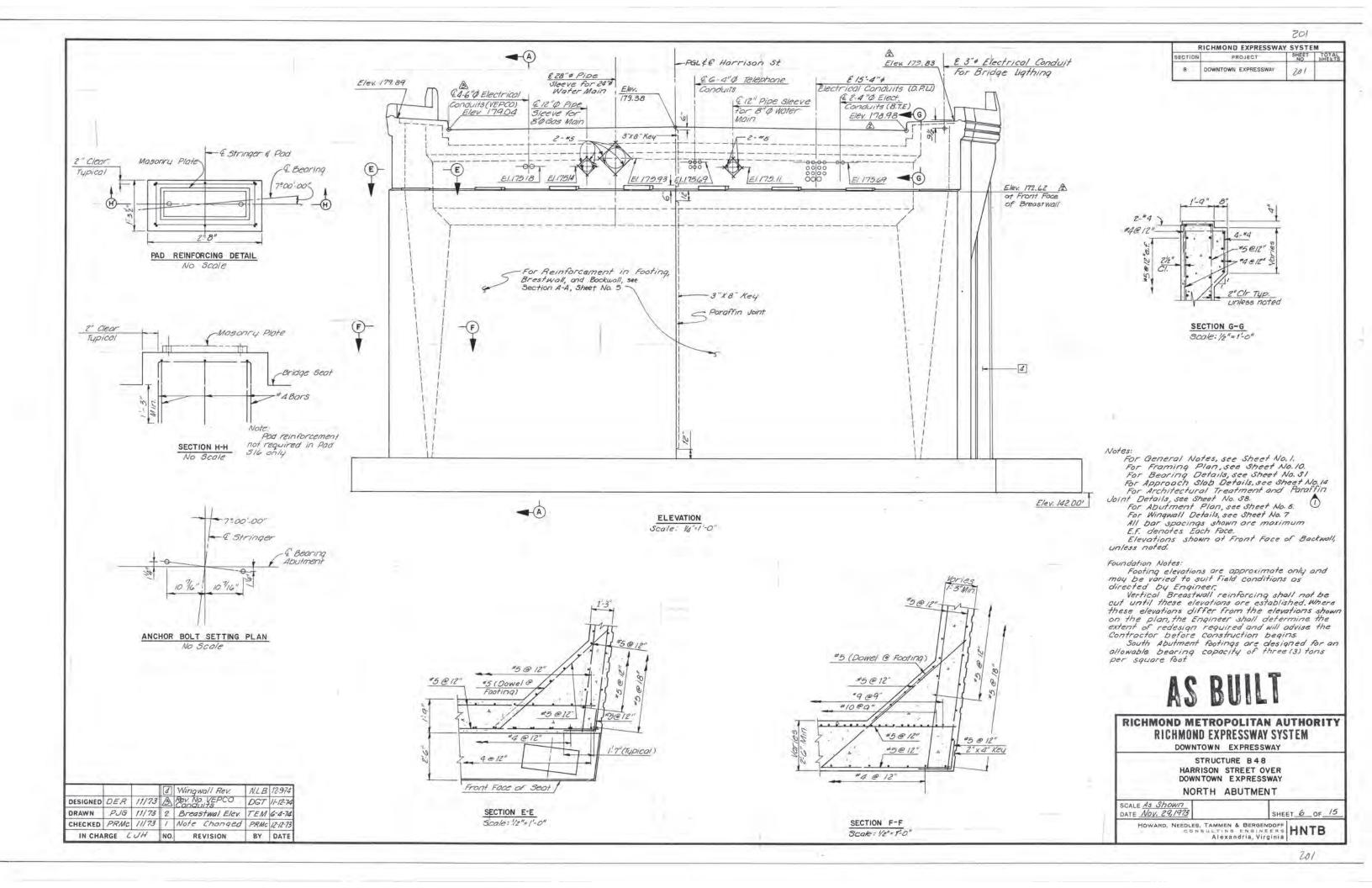
SECTION P-P Scale: 2"=/-0"

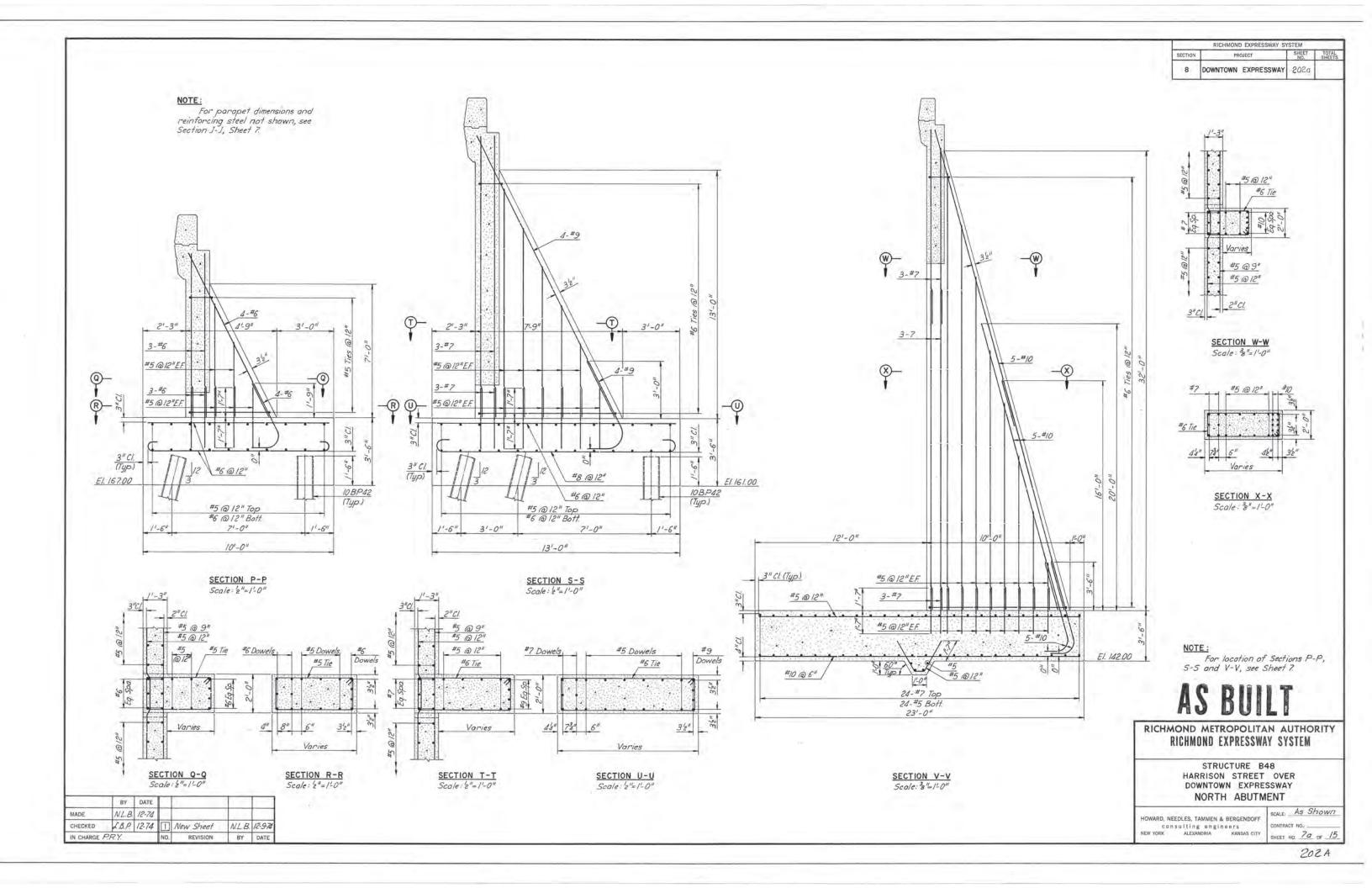


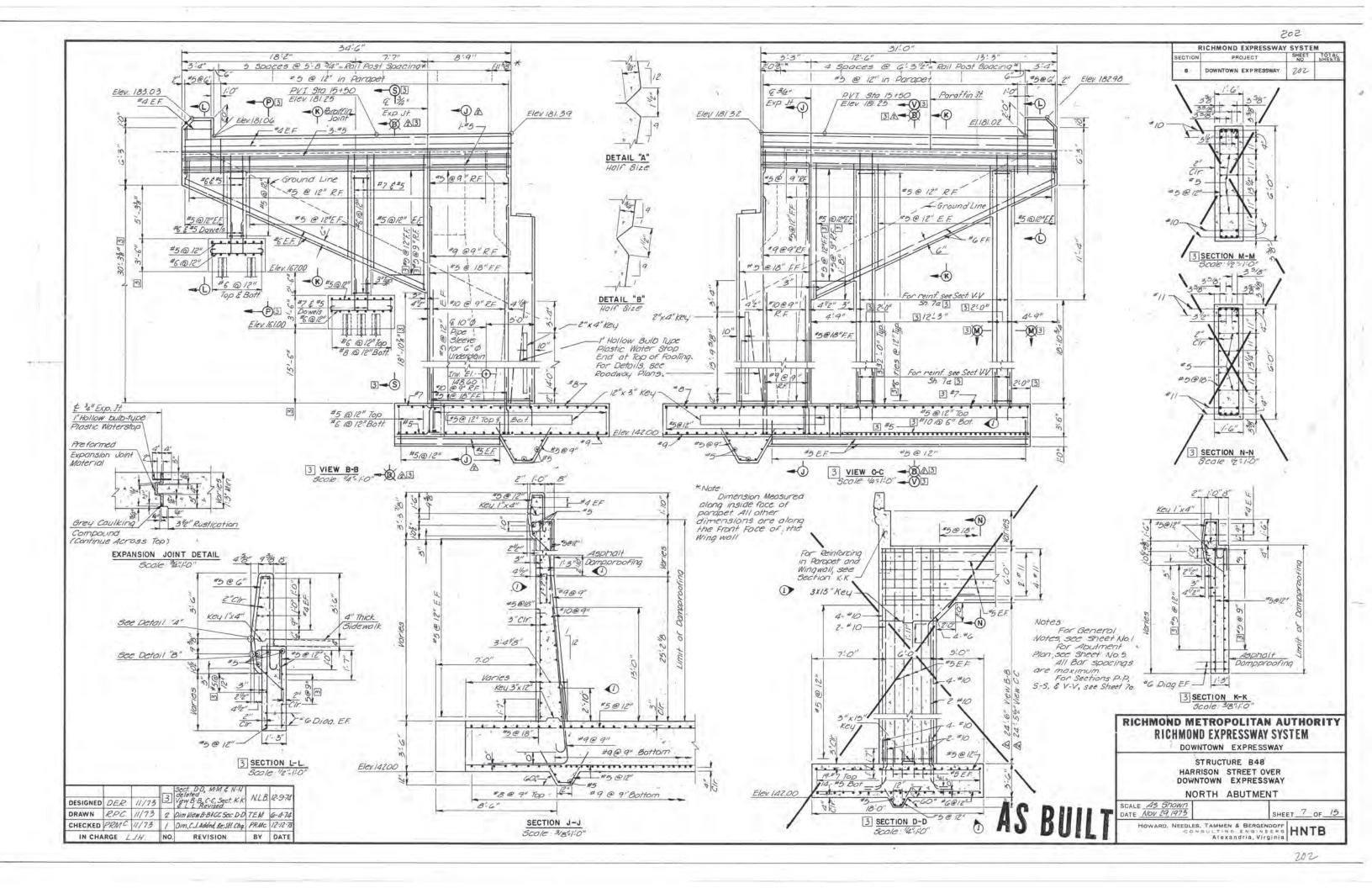
<sup>199</sup>A

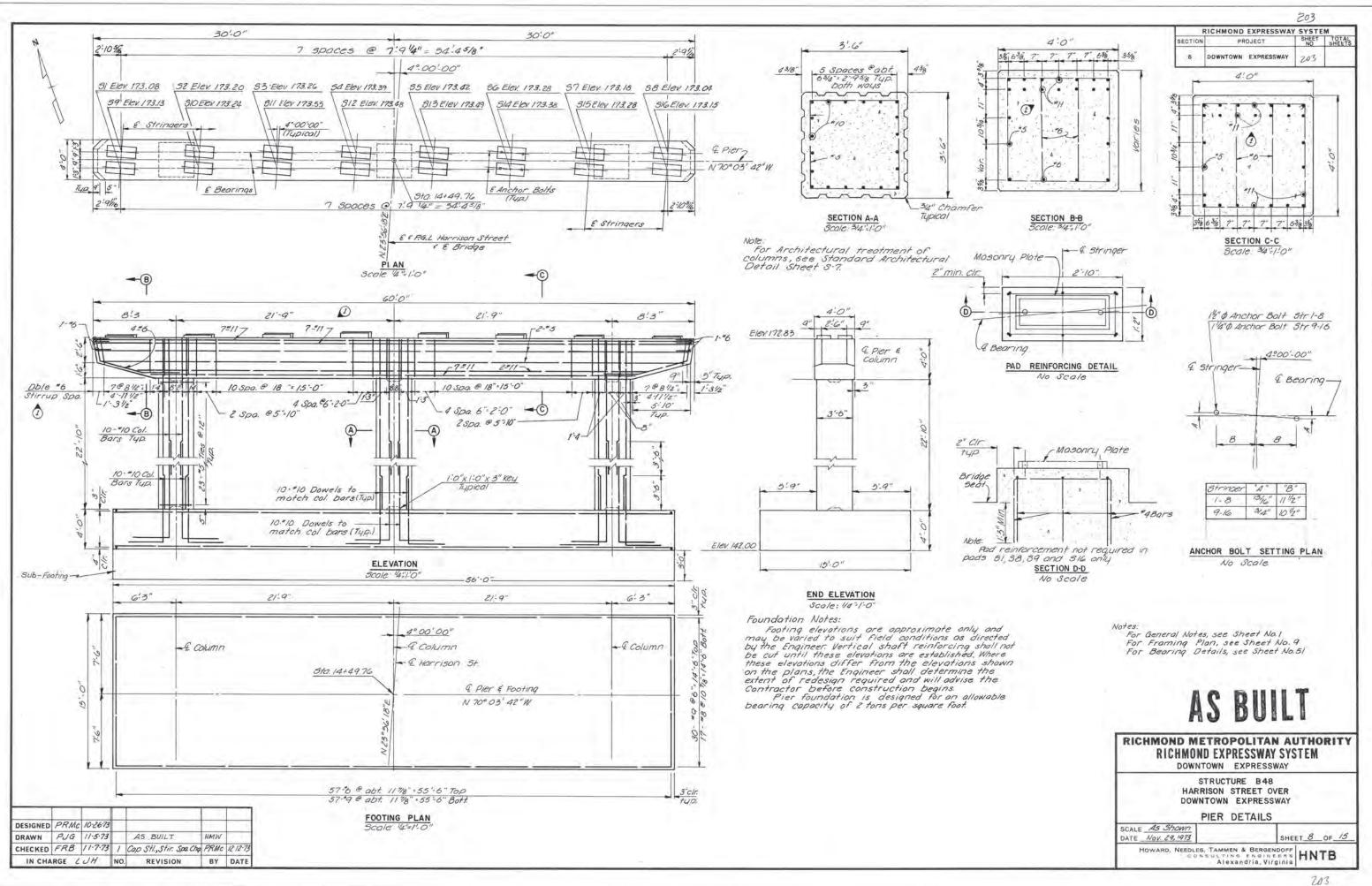


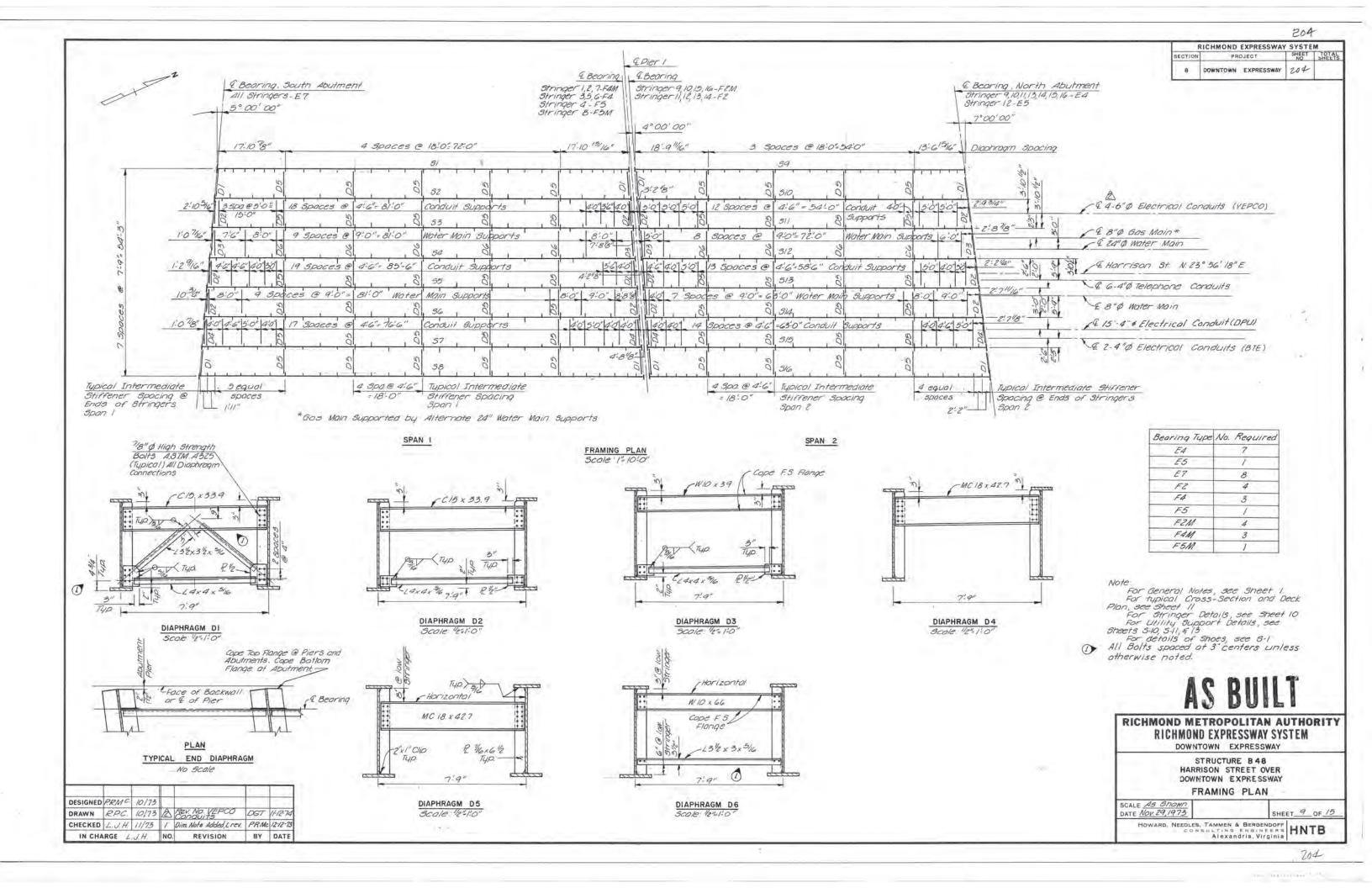




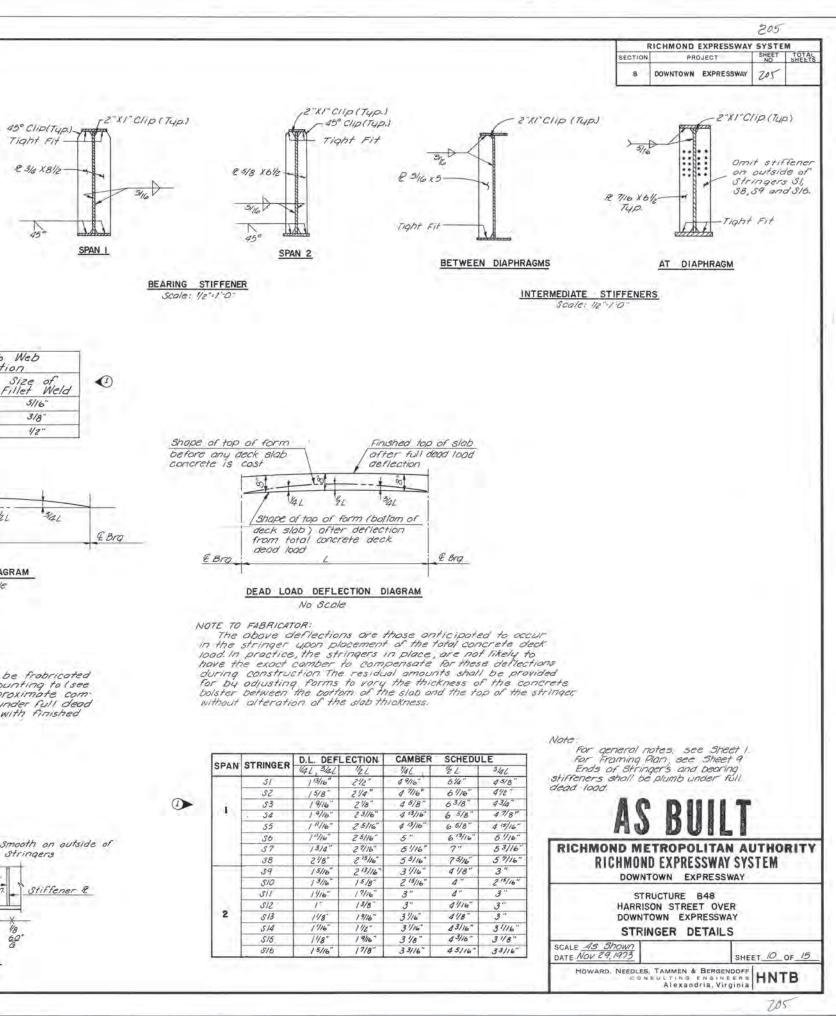


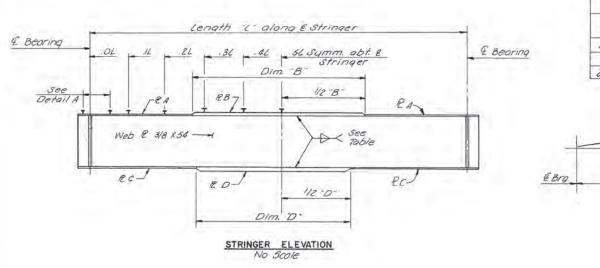


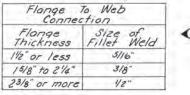


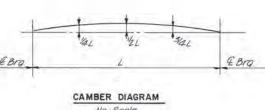


				\$	STRINGE	ER SC	HEDULI	E				
STRINGER	LENGTH	P.A.	R.B	DIM.	₽.c	DIM.	۴D	M	AX. SHEAF	STUD S	PACING	
STRINGER	"L"	EM	- 0	"B"	L 0	"D"	- 0	0.0L-0.IL	0.1L-0.2L	0.2L-0.3L	0.3L-0.4L	0.4L-0.5
51	107-946	718 X 14	15/8 X14	66'-0"	11/2 × 18	60'-0"	21/2×18	16"	18"	21"	24"	24"
52	109-05/16	1 × 14	15/8 X14	60-0-	13/8 X 18	60.0	21/8×18	18"	20"	23"	24"	24"
53	110'-215/16	13/8×14	17/8×14	58:0"	13/4×18	54'-0"	23/8X 18	18"	20"	23-	24"	24"
54	111- 5 5/8	13/4 × 14	21/8×14	46'-0"	17/8×18	54.0"	21/2× 18	18"	20"	24-	24"	24"
55	112-8116	11/4 × 14	17/8×14	58.0"	13/4.18	54:0"	23/8× 18	18"	20"	23"	20"	24"
56	113-10%	13/8×14	2×14	54.0"	17/8×18	54.0"	23/8×18	18"	20	23"	20"	24"
57	115-112	13/8×14	17/8×14	58'0"	17/8×18	54'-0"	23/8X18	18"	20"	23"	24"	24"
58	116-418	11/2×14	28.14	54:0"	2×18	54.0	21/2X18	15"	17"	21"	24"	24"
59	86.45/8"	3/4×12	1		7/8×14	56'0"	13/4×14	17"	18"	20"	22"	24"
510	86-9 9/16	314×12			7/8×14	52:0"	15/8×14	17"	20-	22"	24"	24"
3/1	87-21/2	3/4 ×12	11/8×12	38:0"	1 ×14	54.0"	17/8 X 14	19-	20-	22-	24"	24-
S12	87-73/8	3/4 112	11/0×12	56'0"	11/8×14	54:0"	2×14	19"	20"	22"	24"	24"
S13	88-05/16	3/4×12	1×12	38.0"	7/8×14	58-0-	13/4 × 14	19-	20"	22"	24"	24"
\$14	88-514	3/4×12	11/8×12	38:0"	7/8×14	58'0"	13/4×14	19"	20-	22"	24-	20-
\$15	88-10 18	3/4×12	1×12	40'0"	7/8×14	58:0"	13/4×14	17"	20	22"	24"	24"
\$16	89-3116	314×12	1×12	42:0"	1×14	56.0"	17/8 ×14	16"	18	20"	22-	20-





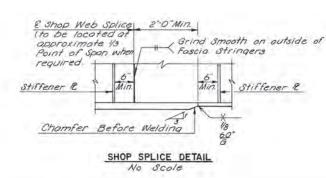


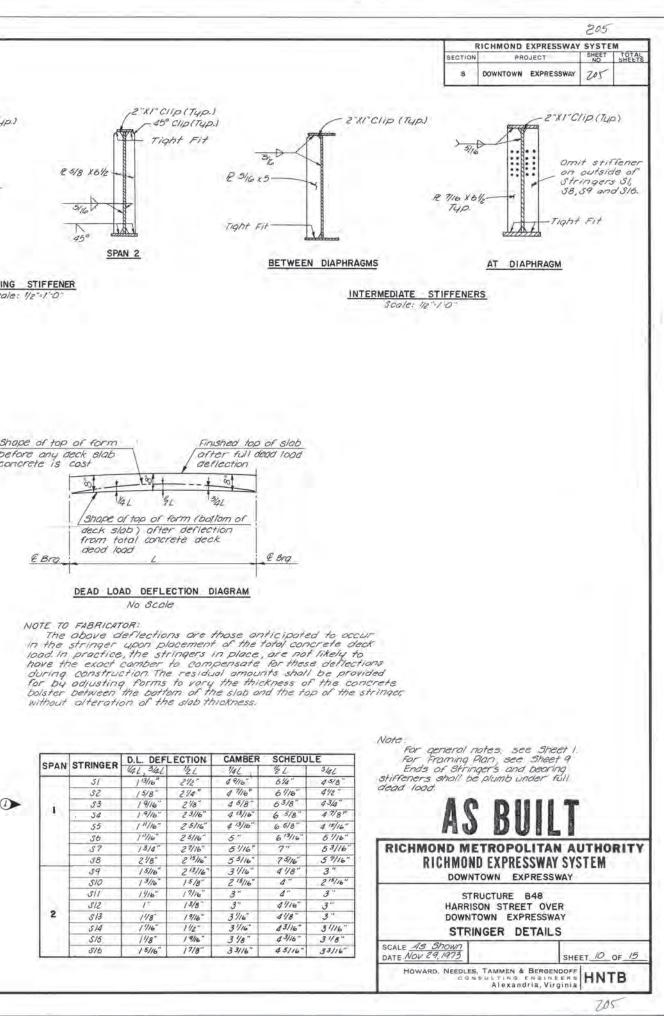


No Scale

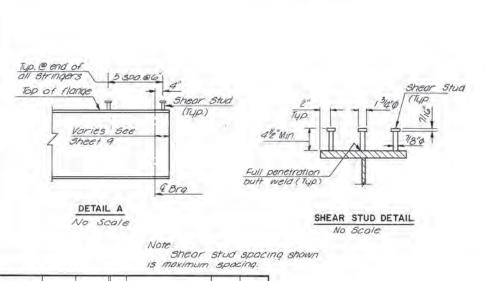
NOTE TO FABRICATOR :

The above stringers shall be frabricated with an upward comber amounting to (see table). This will provide approximate com-pensation for deflection under full dead load and for conformity with finished grade.





	OTOMOTO	D.L. DEFL	ECTION	CAMBE
SPAN	STRINGER	141,341	1/26	-1/4L ,
	51	1 13/16"	212"	4 9/16"
	52	15/8"	21/4"	4 7/16"
1.1	53	19/16"	21/8	4 5/8"
14.	. 54	1.9/16"	2 3/16"	4 13/16
	55	1"/16"	25/16	4 13/16"
	56	1"/16"	25/16	5"
	57	13/4"	27/16"	5 1/16"
	38	21/8"	2 15/16	55/16
	59	15/16"	213/16"	31/16"
	510	13/16	15/8"	2 15/16"
	SIT	1416"	17/16"	3"
	512	1*	13/8	3"
2	\$13	1118	19/16"	3716
2	514	1'116"	11/2-	31/16
	\$15	11/8"	19/16"	3 1/8"
	516	15/16"	17/8	3 3/16"



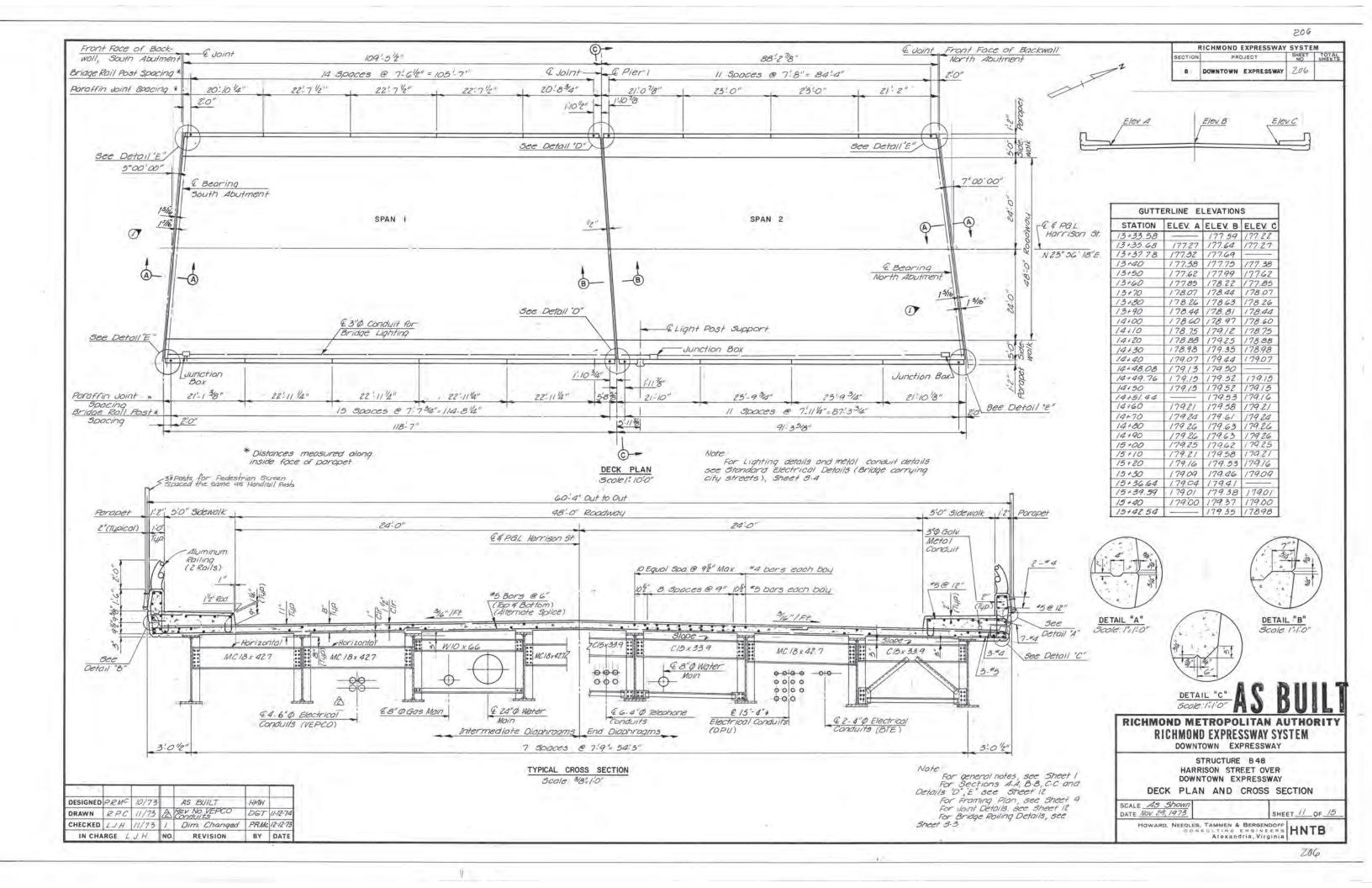
BY DATE

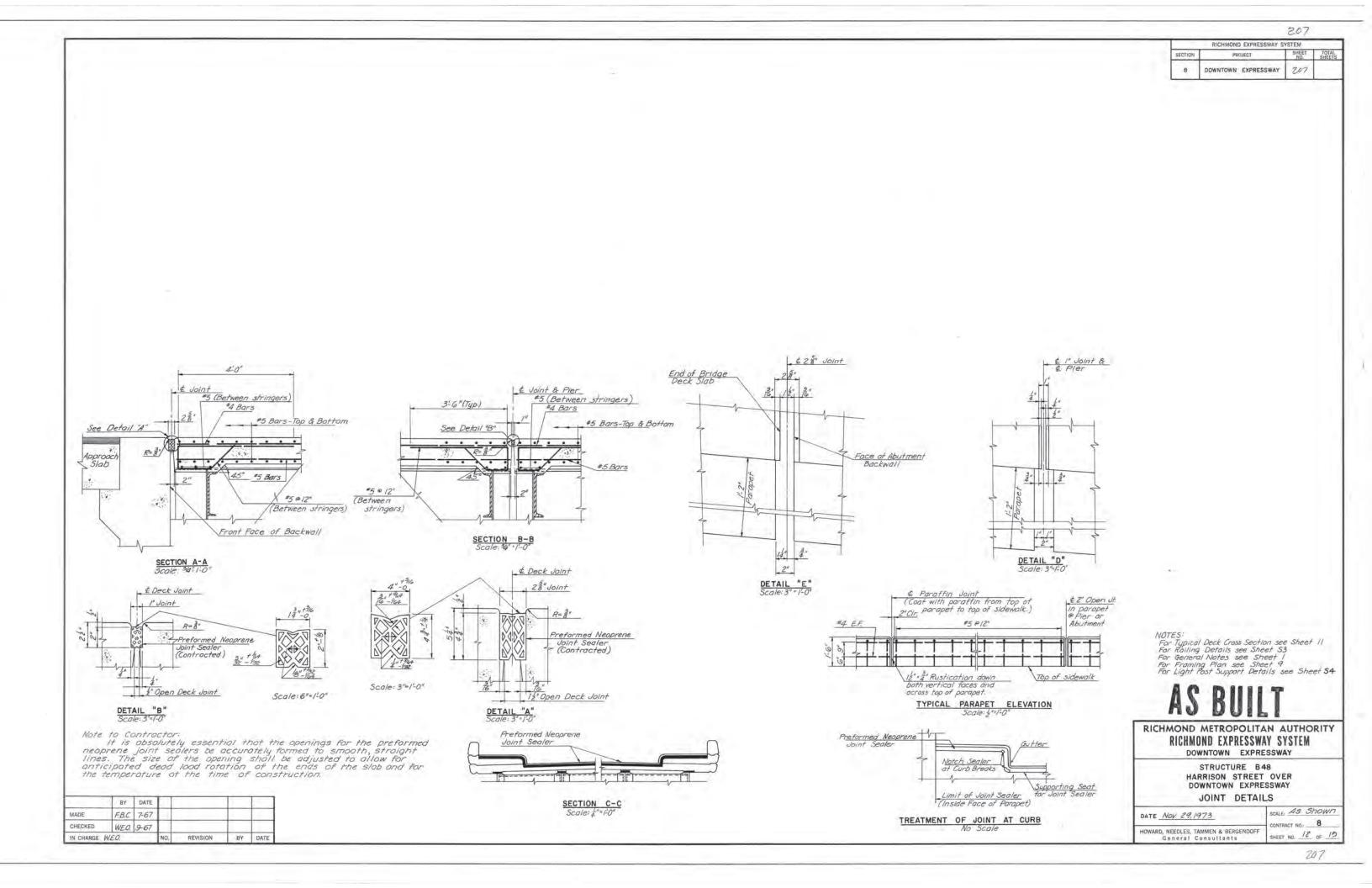
DESIGNED PRMC 10/73

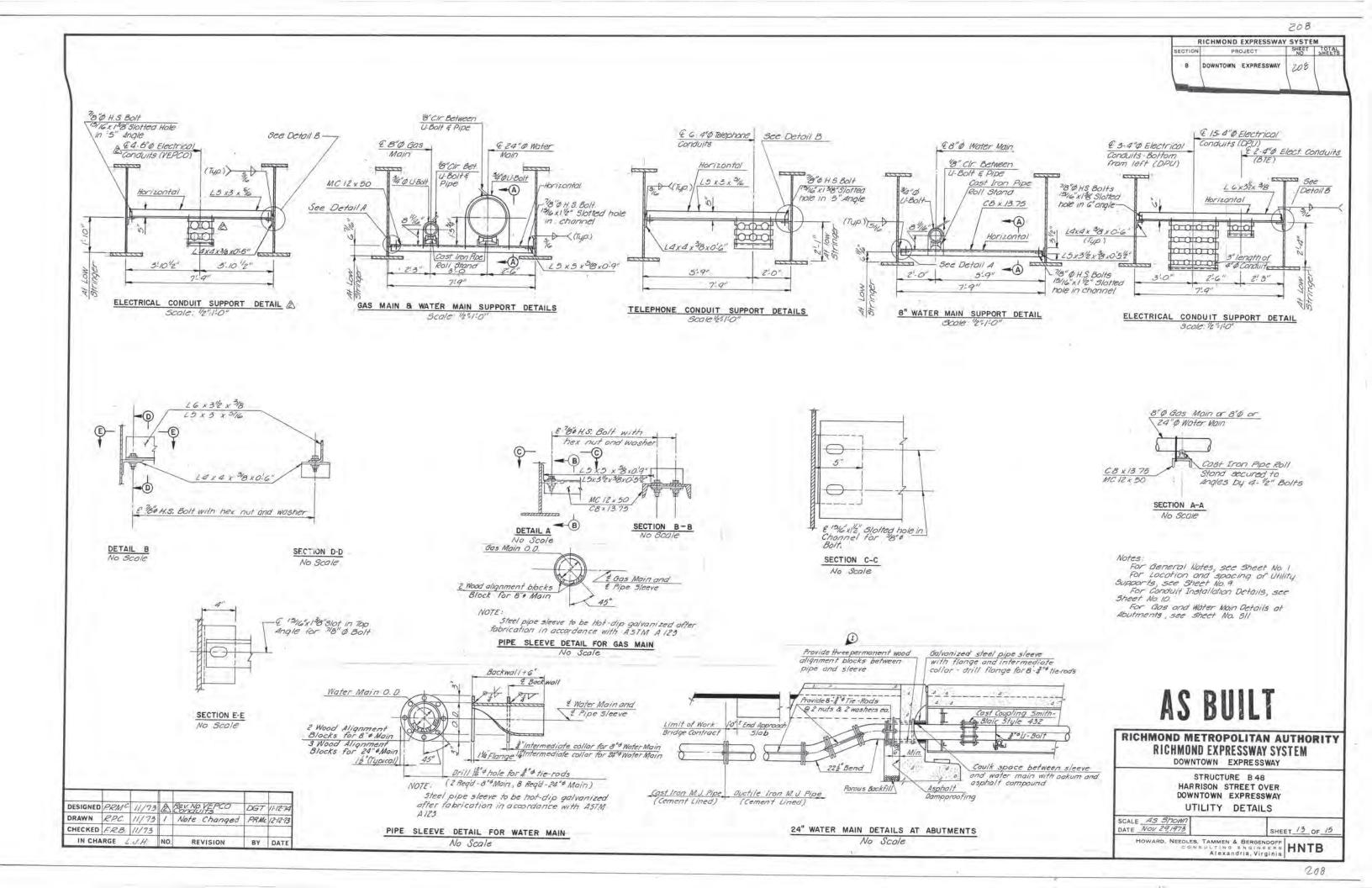
DRAWN P.J.G 11/73

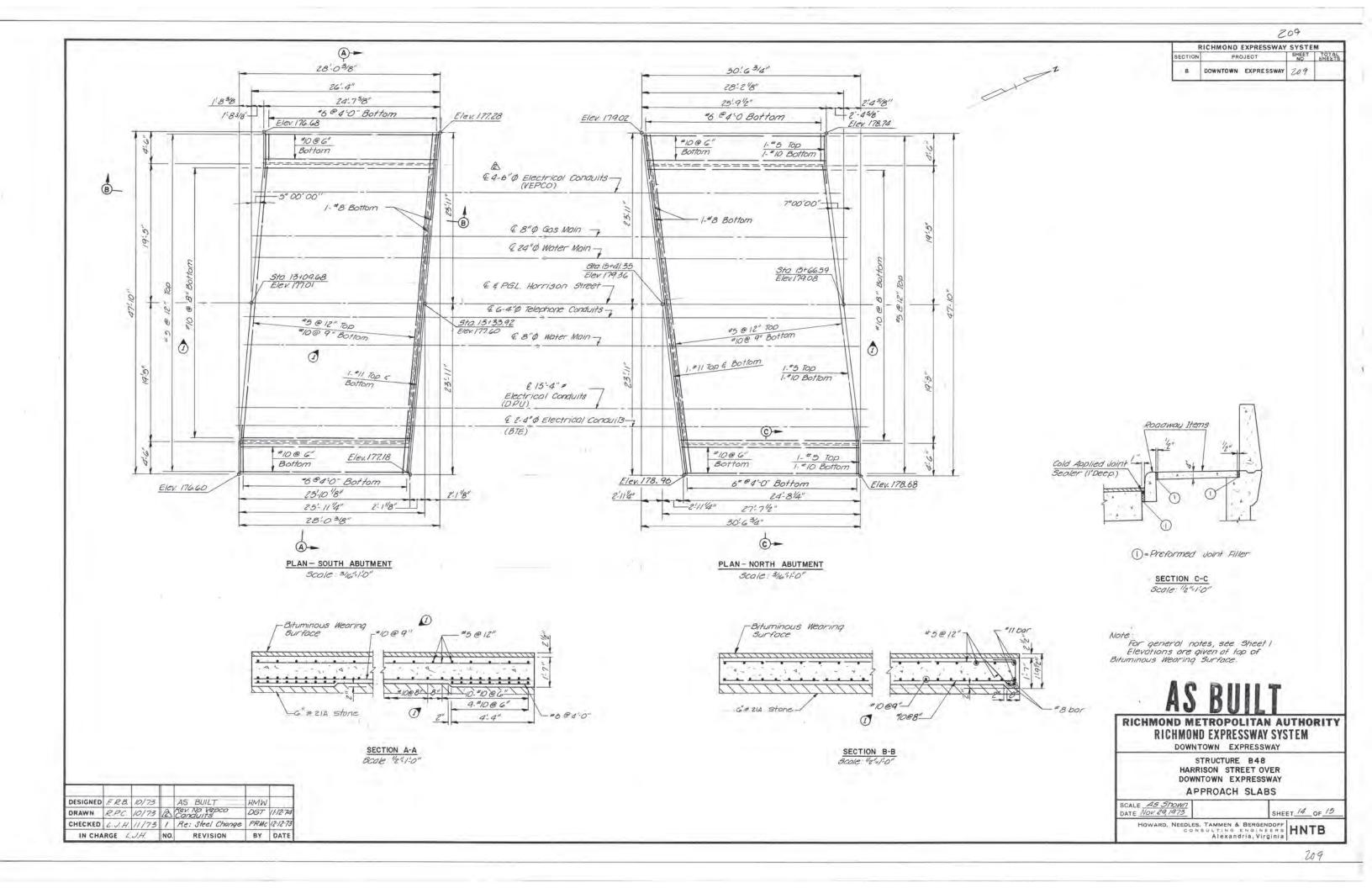
CHECKED LJ.H. 11/73 1 Dim Chq. Def. Table Added PRMC 12-12-73

IN CHARGE L.J.H. NO. REVISION





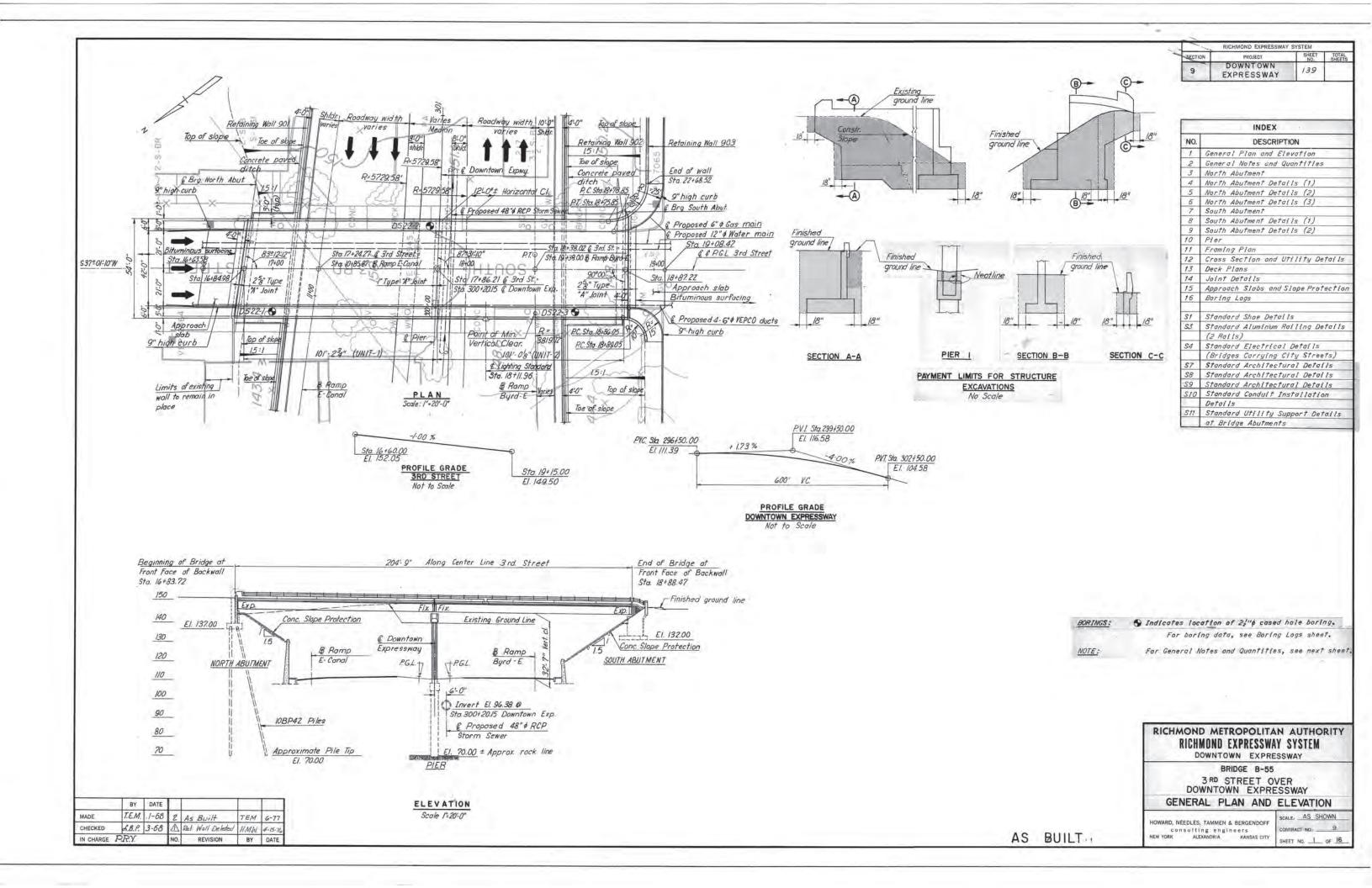




## Bridge 55

(3<sup>rd</sup> Street over Downtown Expressway - Rte. 195)

**Record Set Plans** 



## GENERAL NOTES

ROADWAY: CAPACITY:	One 42'-O" clear roadway. Two 5'O" sidewalks. Dead Load-Includes 15 lbs. per sq.ft. for future wearing surface. Live Loads-HS20-44 loading and B.P.R. modified for military vehicles.	CONGRETE NOTES	Concrete in superstructure shall be Class A4. other concrete shall be Class A5. All exposed edges and corners shall have a 3/4" chamfer or fillet unless otherwise noted. Care in the me of vibration, the use of low-slump concrete, dwng other means shall be employed to prevent, downg
SPECIFICATIONS	GENERAL-Virginia Department of Highway Road and Bridge Specifications, 1970.		movement of newly placed slab concrete (when g is over 2%).
	DESIGN-A.A.S.H.O. Standard Specifications for Highway Bridges, 1961, modified by Special Desian Provisions.	1	Finishing Concrete Surfaces: See the Standard Architectural Details sheets and the Contract Special Provisions for types and details.
	WELDING-1969 Standard Specifications for Welded Highway and Ratiway Bridges of the American Welding Society.		All reinforcing steel shall conform to ASIM A6/5 Gr All reinforcing bar dimensions on the detailed drawings are to centers of bars unless otherwi
	CONTRACT SPECIAL PROVISIONS		noted. Clear distance between reinforcing ste and face of concrete shall be as noted on the plans. All bar laps shall be 30 diameters of
	Specifications and Contract Special Provisions referred to above are necessary to make these plans complete.	and the second se	smaller diameter bar unless otherwise noted.
DATUM:	CITY OF RICHMOND	STEEL NOTES:	Structural steel shall conform to A.S.T.M. Spe
TEMPERATURE :	The normal temperature referred to on the plan Is 60°F. The temperature range for movement Is 0°F to 120°F.		fleation A36 except as noted. All field connections shall be made with high strength bolts. High strength bolts shall be 7/8" diameter unl
DIMENSIONS:	All dimensions are measured horizontally and vertically unless otherwise noted.	and the second second	otherwise noted and shall conform to A.S.T.M. Specifications A325.
EXCAVATION :	Excavation below subgrade and cut slope template shall be classified as Structure Excavation. All excavation above these limits shall be classified as Regular Excavation and is not included in the	BENCH MARKS:	See Reference Ties and Field Control Data shee In highway plans.
	Structural Quantities.	10	C-46 Plug, corner Byrd St. and S. 3rd St., Elev. 149.69
FOUNDATIONS:	Footings shall rest on firm material. Foundation material shall be kept dry and special attention is called to Sections	8	C-47 Plug, corner E. Canal St. and S. 3rd St Elev. 152.70
	401,05 and 401,06 of the General Specifica- tions and to the Contract Special Provisions concerning preparation of foundations for footings.		

								FIN	AL QU	ANTITIES						
	STRUCTURE EXCAVATION C.Y.	CONCRETE CLASS A3 C.Y.	CONCRETE CLASS A3 BR APPR SLABS C.Y.	CONCRETE CLASS A4 C.Y.	REINFORCING STEEL LBS.	STRUCTURAL STEEL LBS.	ALUMINUM BR. RAILING (2 RAILS) L.F.	DRILLED HOLES FOR CAISSONS 4' 0 L F.	POROUS BACKFILL C.Y	STEEL PILES IOBP42	CONC. SLAB SLOPE PROTECTION S.Y	DAMP- PROOFING S.Y.	UNDERDRAIN 6" Ø L.F	GAS MAIN 6°¢ L.F.	WATER MAIN 12"¢ L.F.	
SUPERSTRUCTURE				384.39	84,396	418,496	409.9							251	249	
NORTH ABUTMENT	299	175.67	<u>)</u>	100	12,201		22.7		29	2134	178.68		129			L
PIER I	38	140.94	1		39,985			95.3		1.						L
SOUTH ABUTMENT	623	209.43			16,751		28.9		41		246.75	118	90			
APPROACH SLABS		1	100.37		22,618					1						L
TOTAL	960	526.04	100.37	384.39	175,951	418,496	461.5	95.3	70	2134	425.43	229	219	251	249	
and the second s	1.2.2.2	122.13	· · · · · · ·				1			1	7	1000				

1.20	BY	DATE				
MADE	EVR	3-68				
CHECKED 6.B.P. 12-68		12-68	1	As Built	TEM	6-77
IN CHARGE	PRY.		NO.	REVISION	BY	DATE

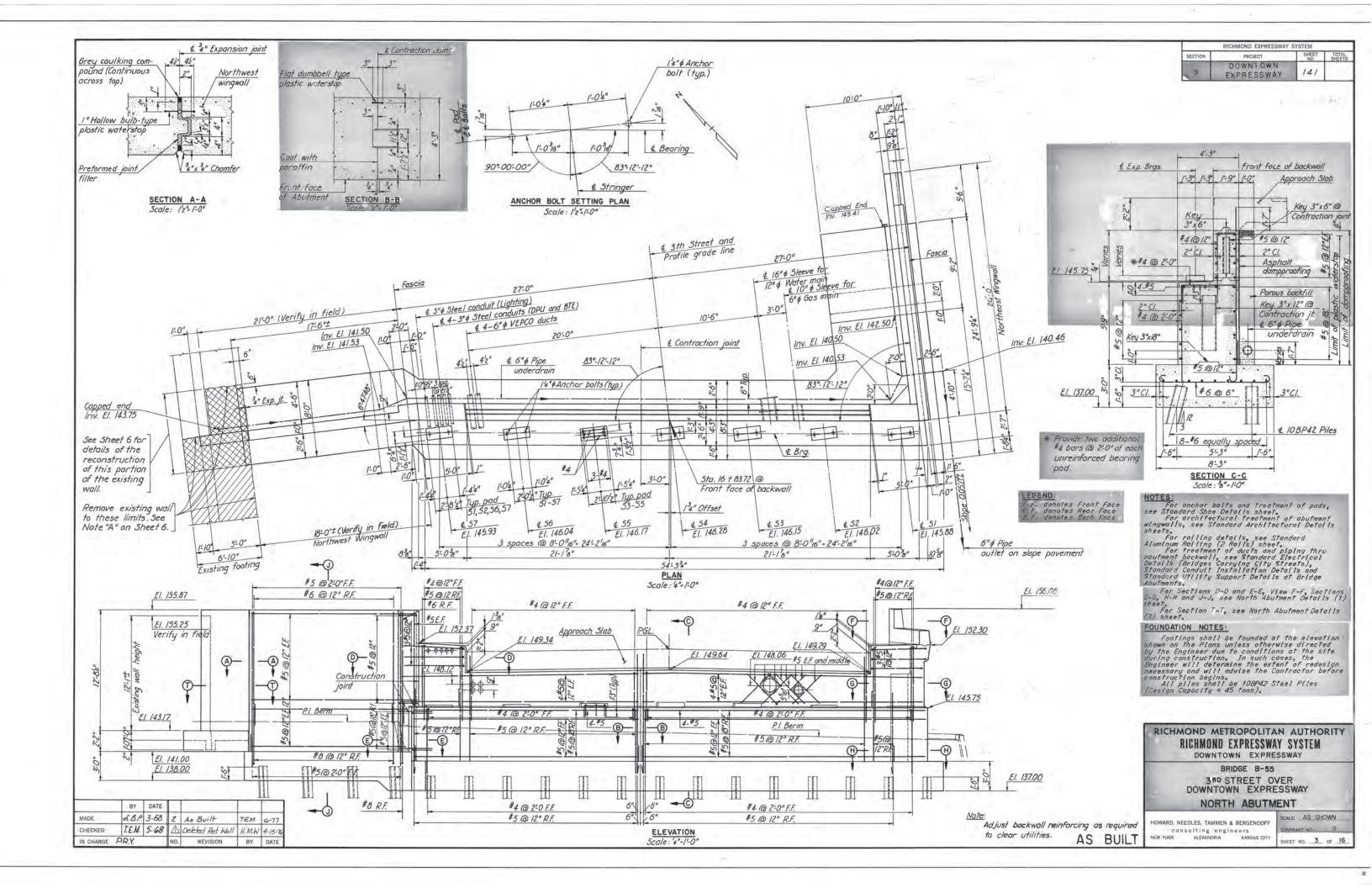
edd fl oft no Spi All All All Spi Still Alt Spi Spi Sein C									RICHMOND EXPRESSWAY		
edd fl oft no Spi All All All Spi Still Alt Spi Spi Sein C								SECTION	PROJECT	SHEET NO.	TOTAL
edd fl oft no Spi All All All Spi Still Alt Spi Sein C								9	DOWNTOWN	140	CONSULT.
Fill Arr Spi Al: dr no onin pli sm Sti Spi Spi Sein C	Ther con	in superstruct crefe shall i corners sha less otherwis ion, the use ms shall be o of newly place %).	11 6 8 4 8 3	IAN at a far					EXPRESSWAY		
Al no onin pim Still Spo See In G-	inishing	Concrete Sur ural Details Provisions for	rfaces: See sheets and	the Standar the Contrac							
File All still offi Spo See In C	11 notes	orcing steel orcing bar d are to center lear distanc of concrete il bar laps 'lameter bar d	imanalana a	a the detail	od						
In C-	ication Il field trength igh stre therwise	l steel shal A36 except as connections bolts, ngth bolts si noted and si tions A325.	s noted. shall be m hall be 7/8	nade with hig 3" diameter u	h nless						
	ee Refer n highwa	ence Ties and y plans.	d Field Con	ntrol Data sh	eet						
	EI	ug, corner By ev. 149.69 ug, corner E. ev. 152.70			st.,						
PR	DAMP- ROOFING	UNDERDRAIN 6* Ø	GAS MAIN 6'\$	WATER MAIN 12"¢	CONDUIT 6"¢	METAL	E				
N	S.Y.	LE	LF	L.E	VEPCO L F.	3"¢ L.F.					
-		1 1	251	249	997	1278					
12		1									

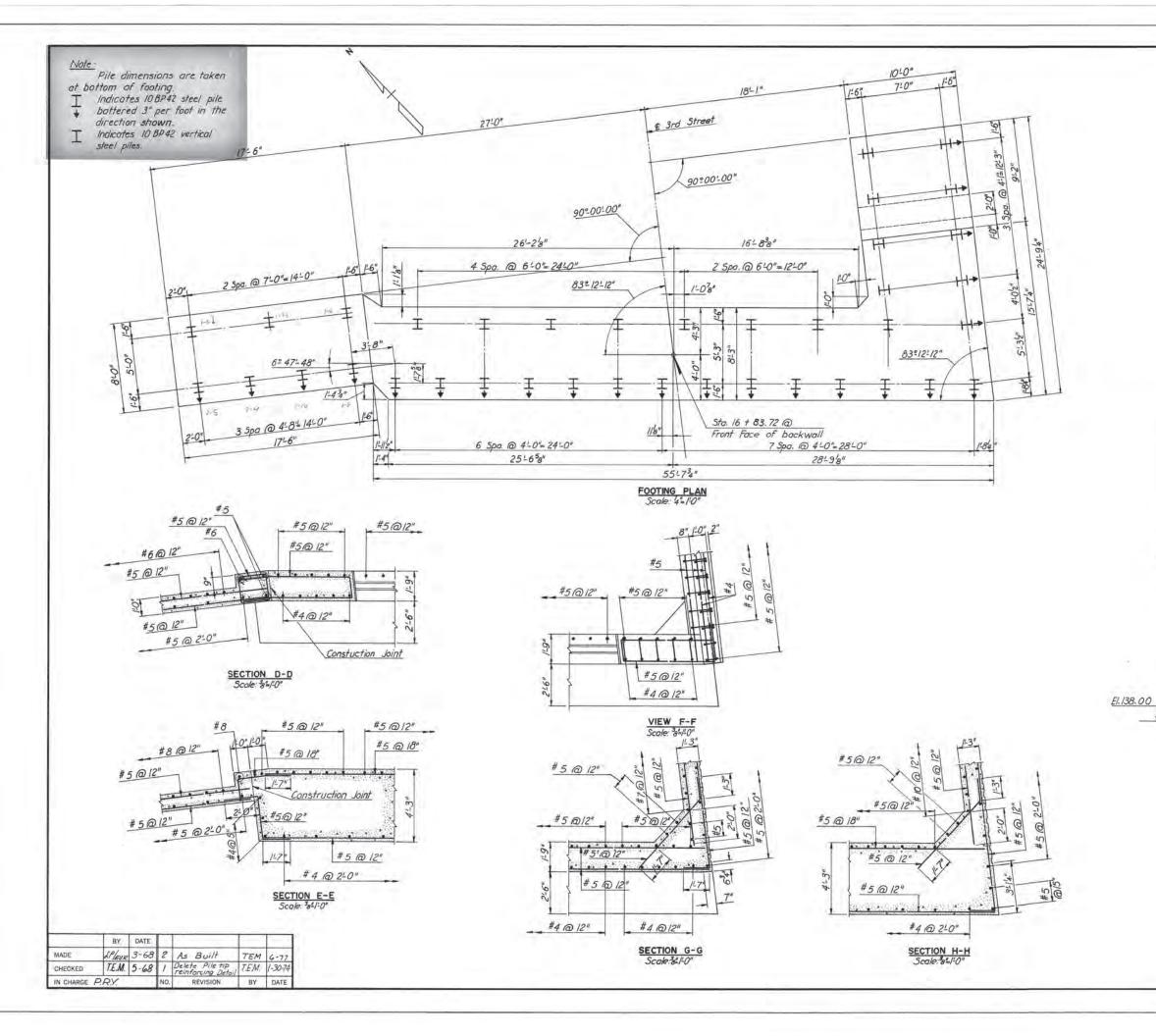


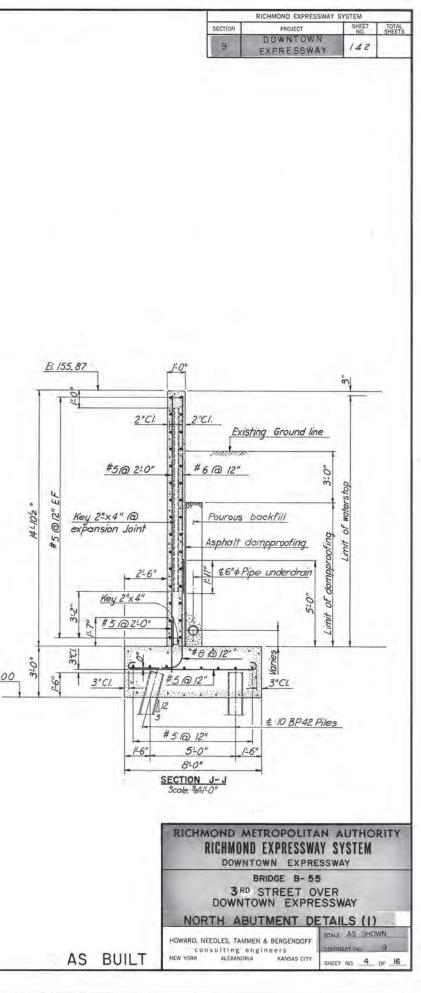
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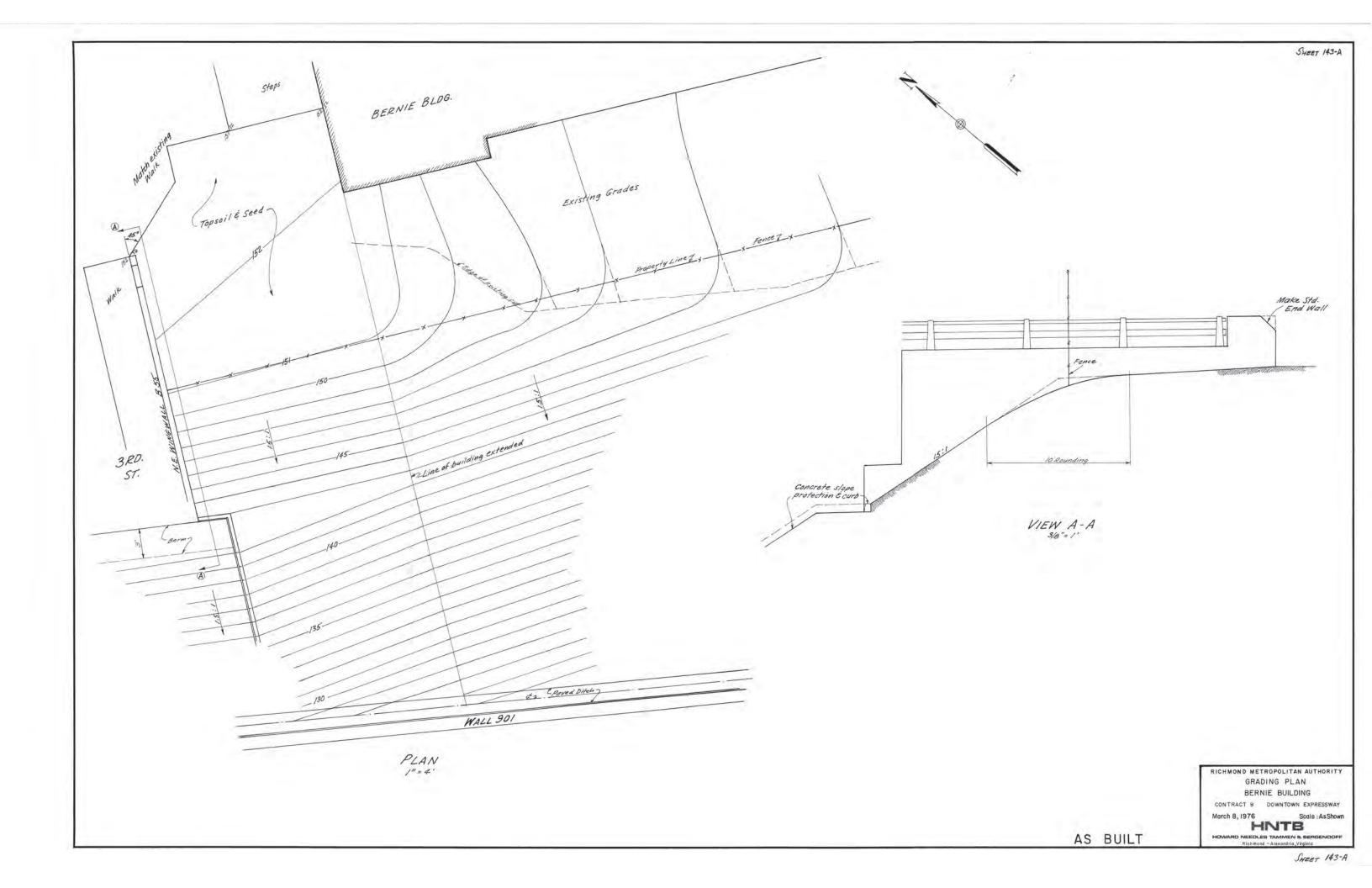
997

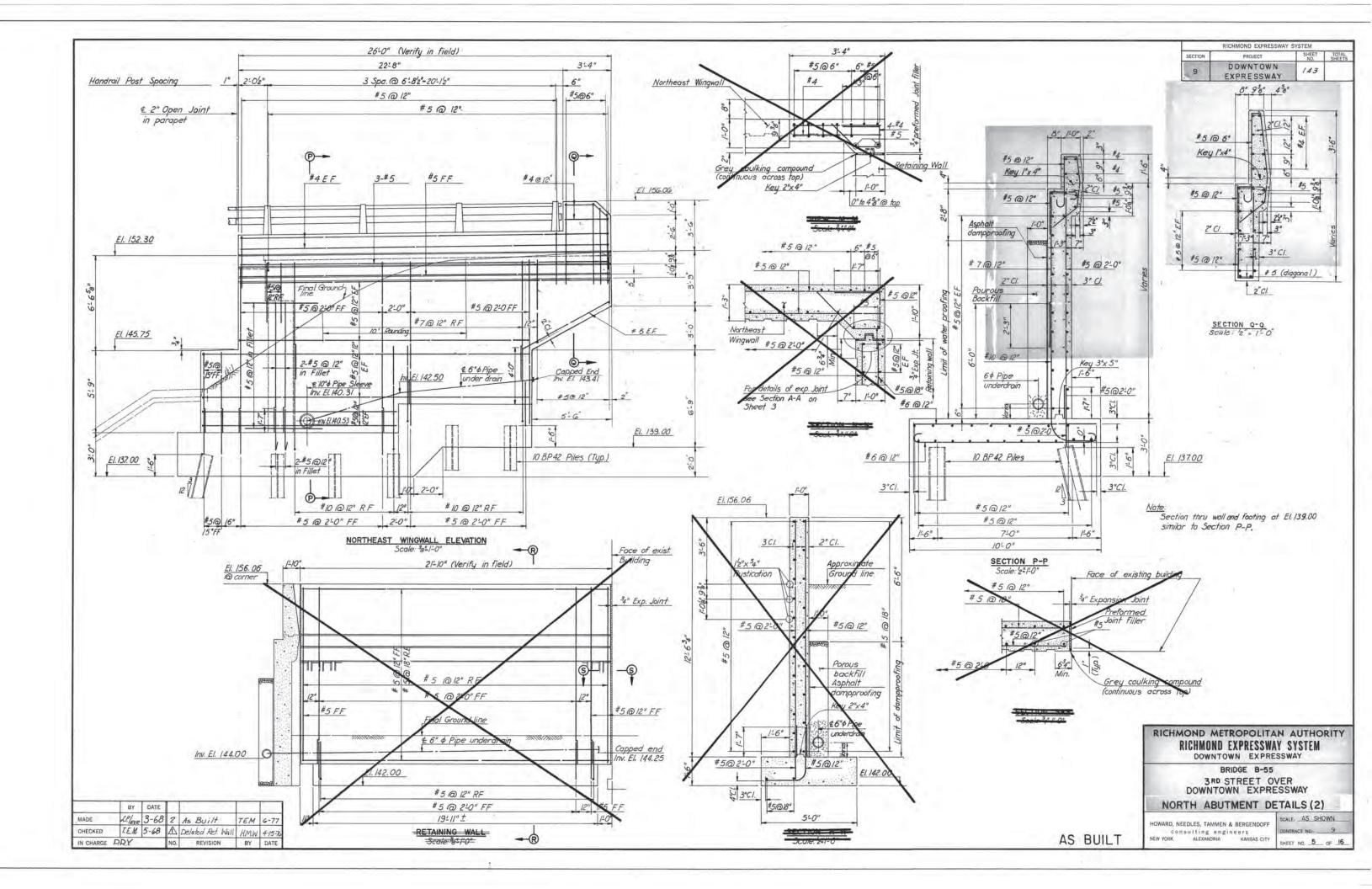
1278

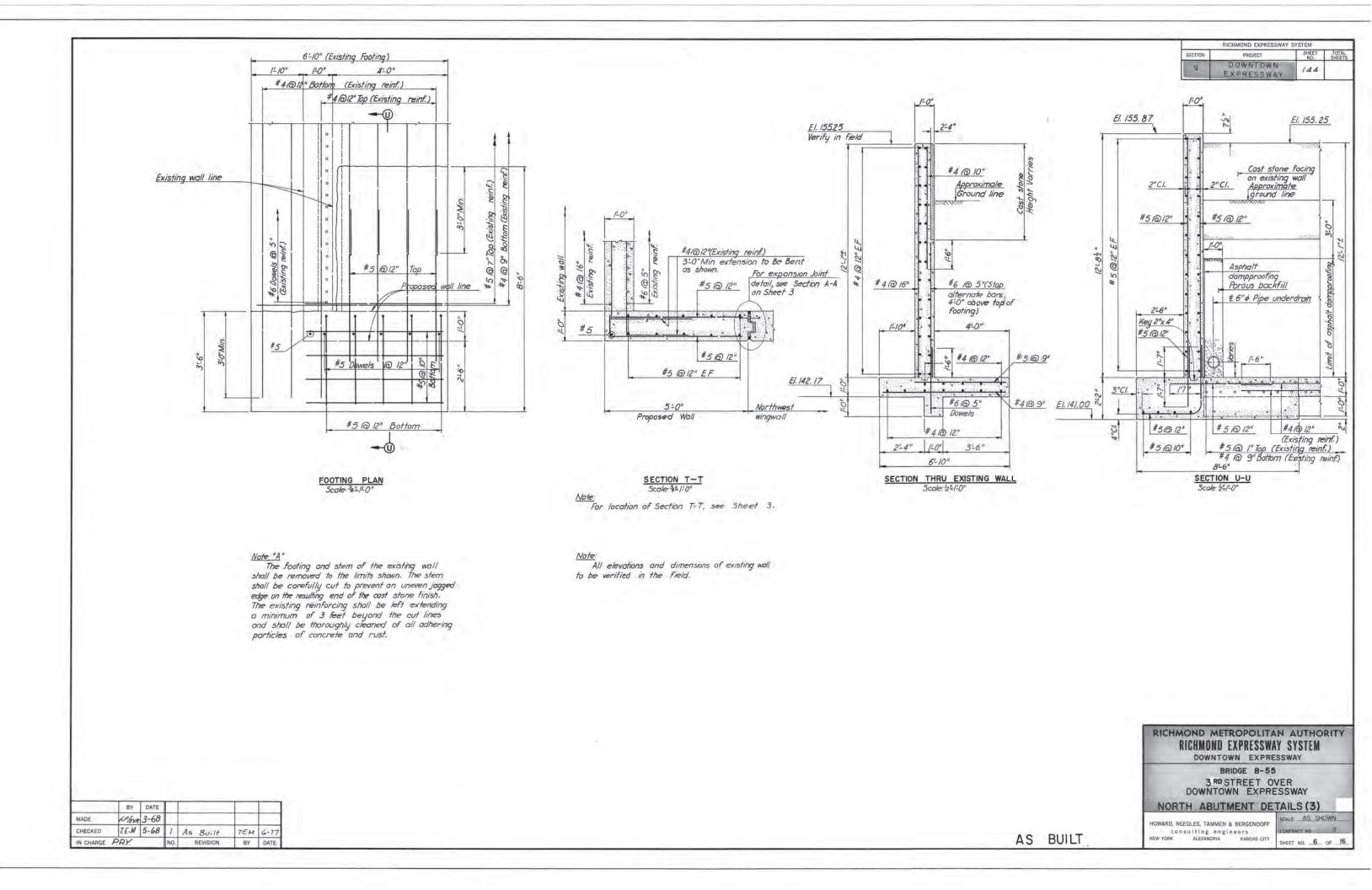


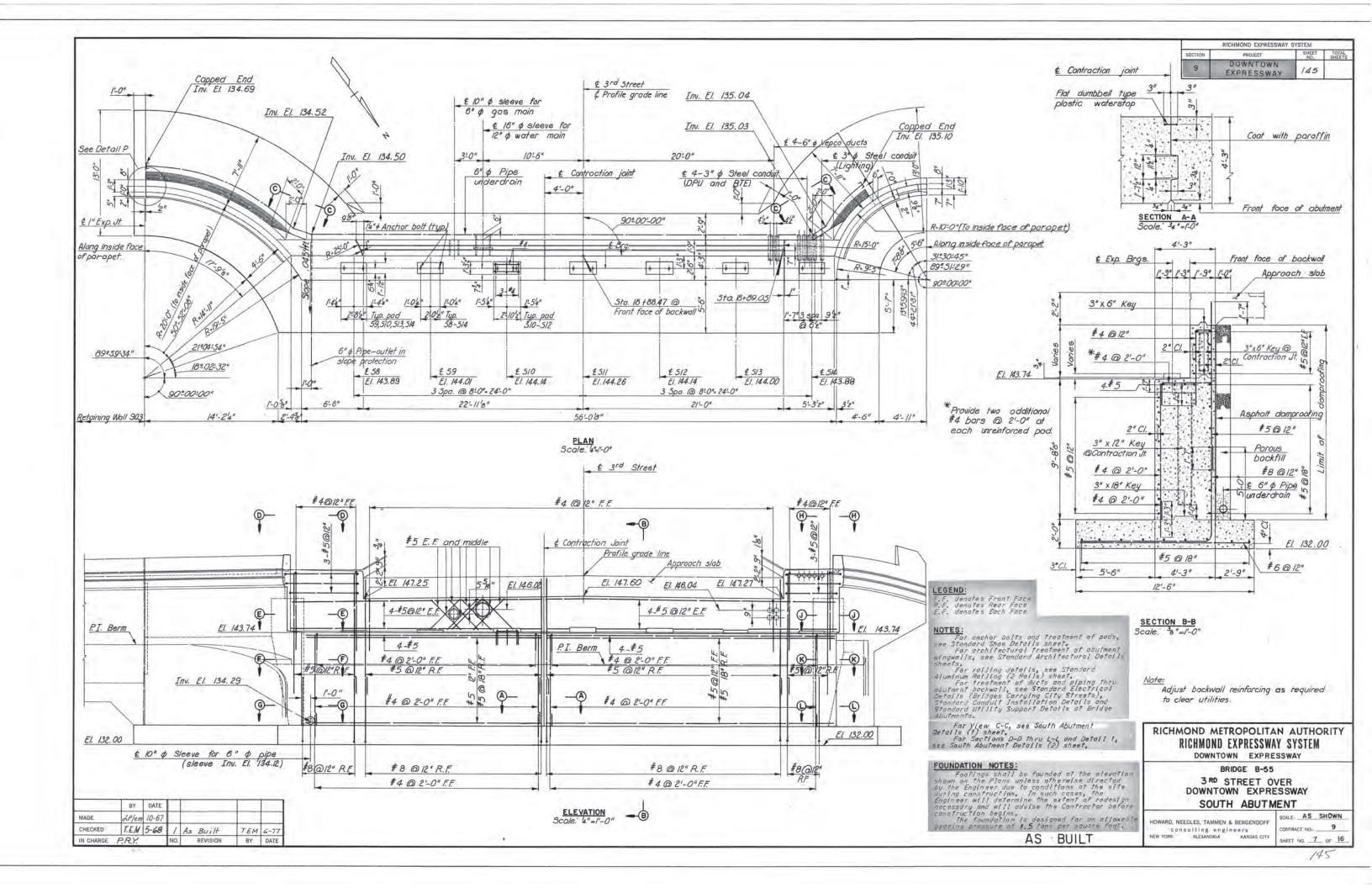


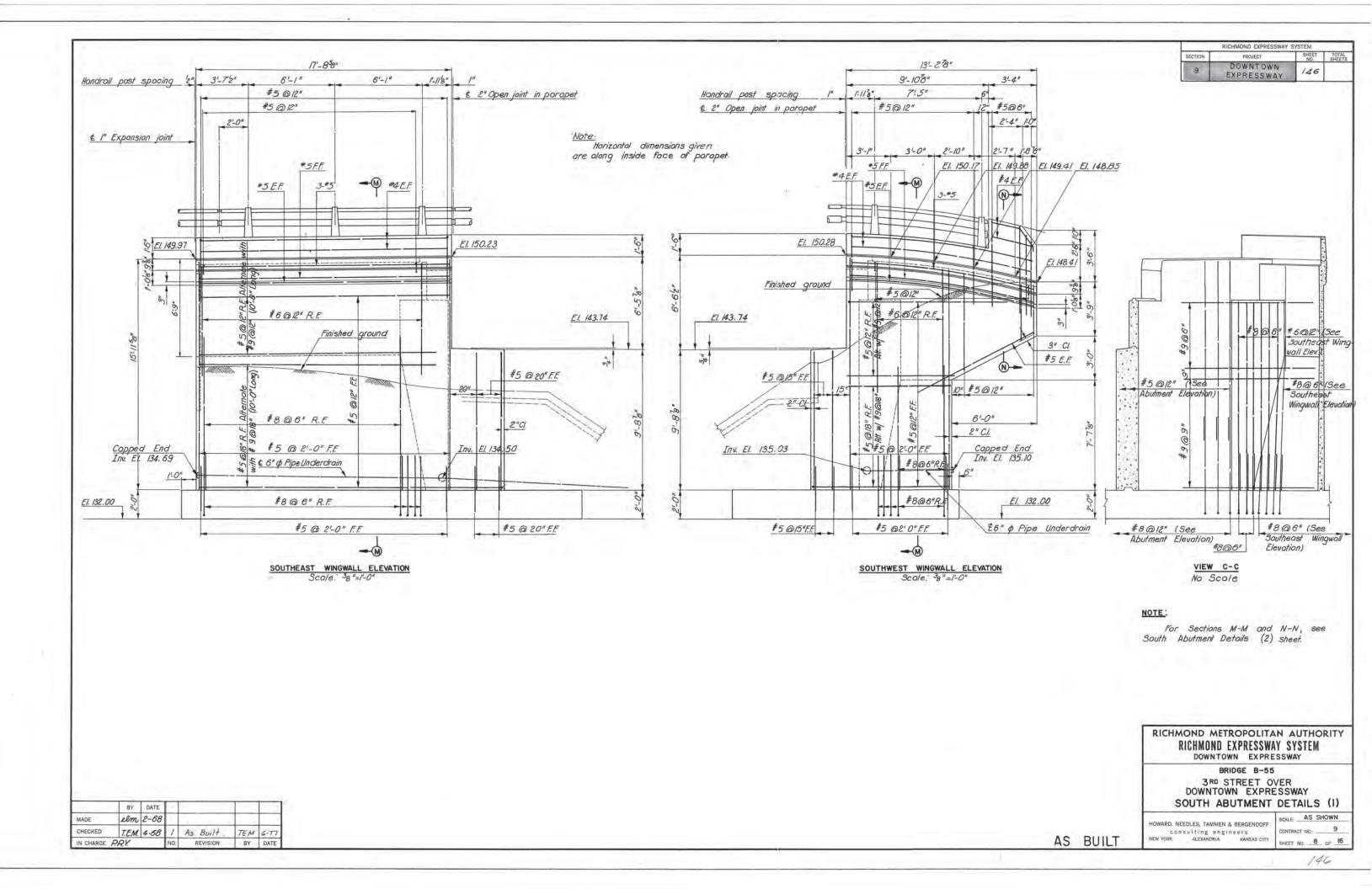


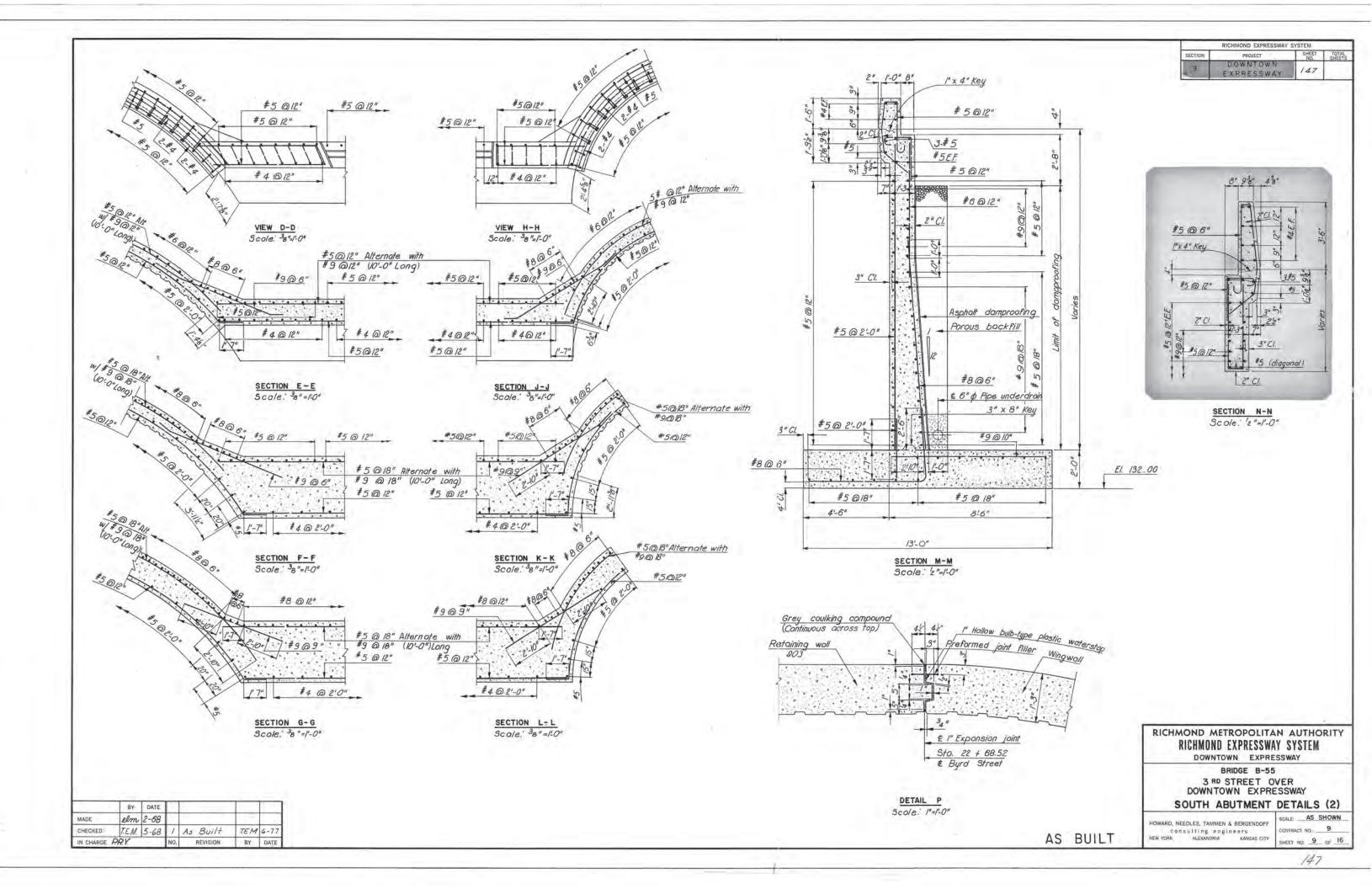


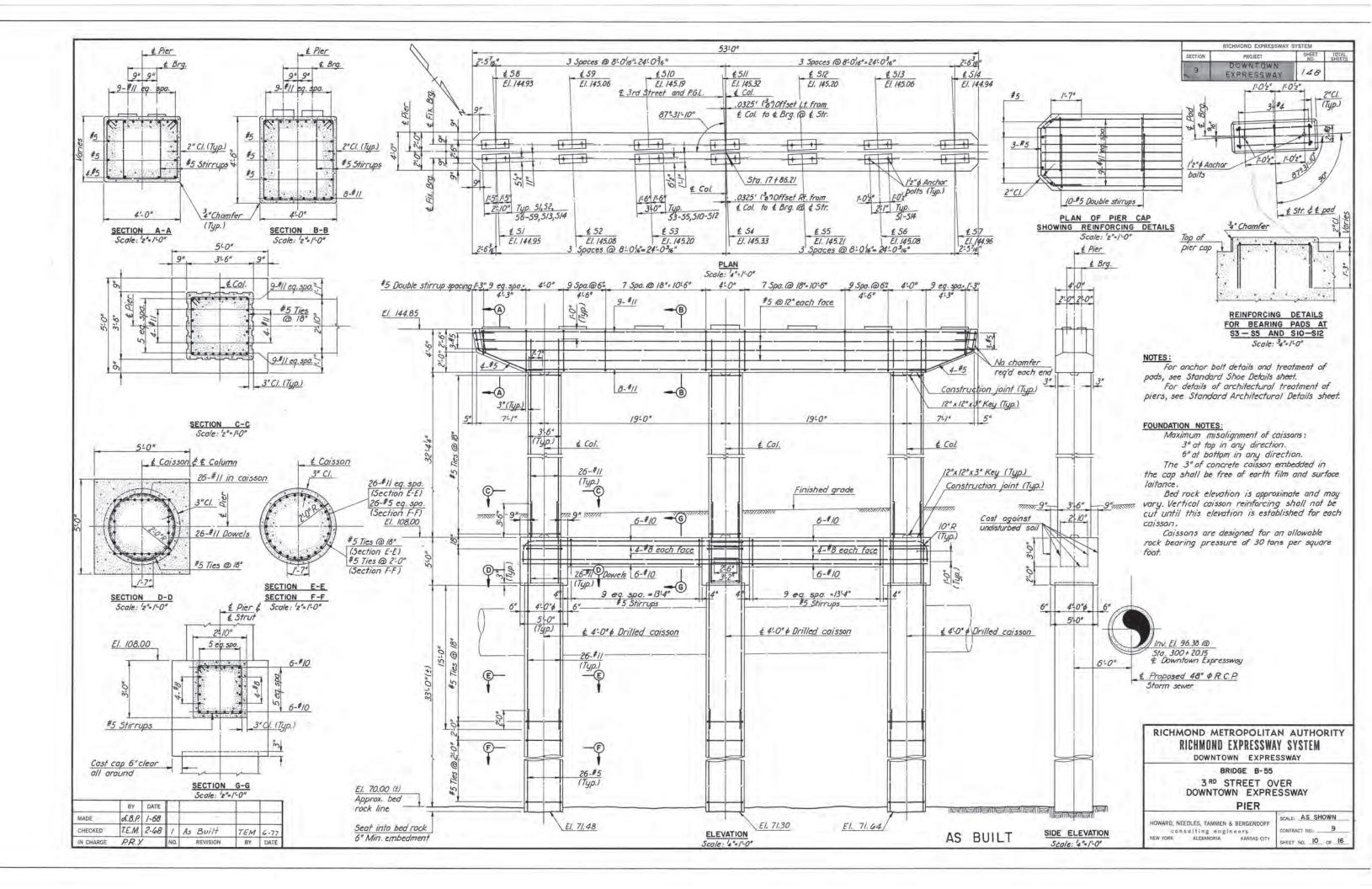


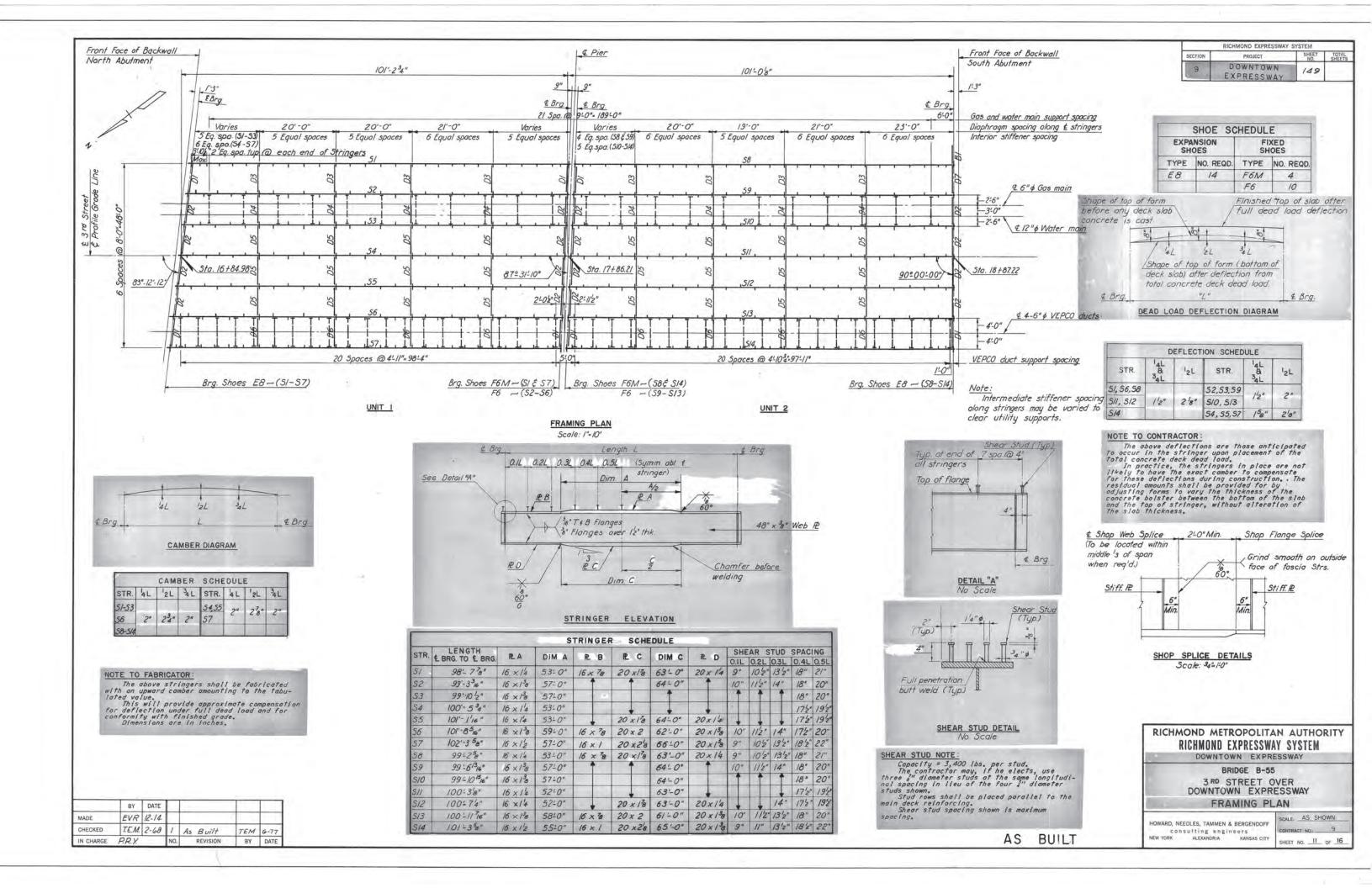


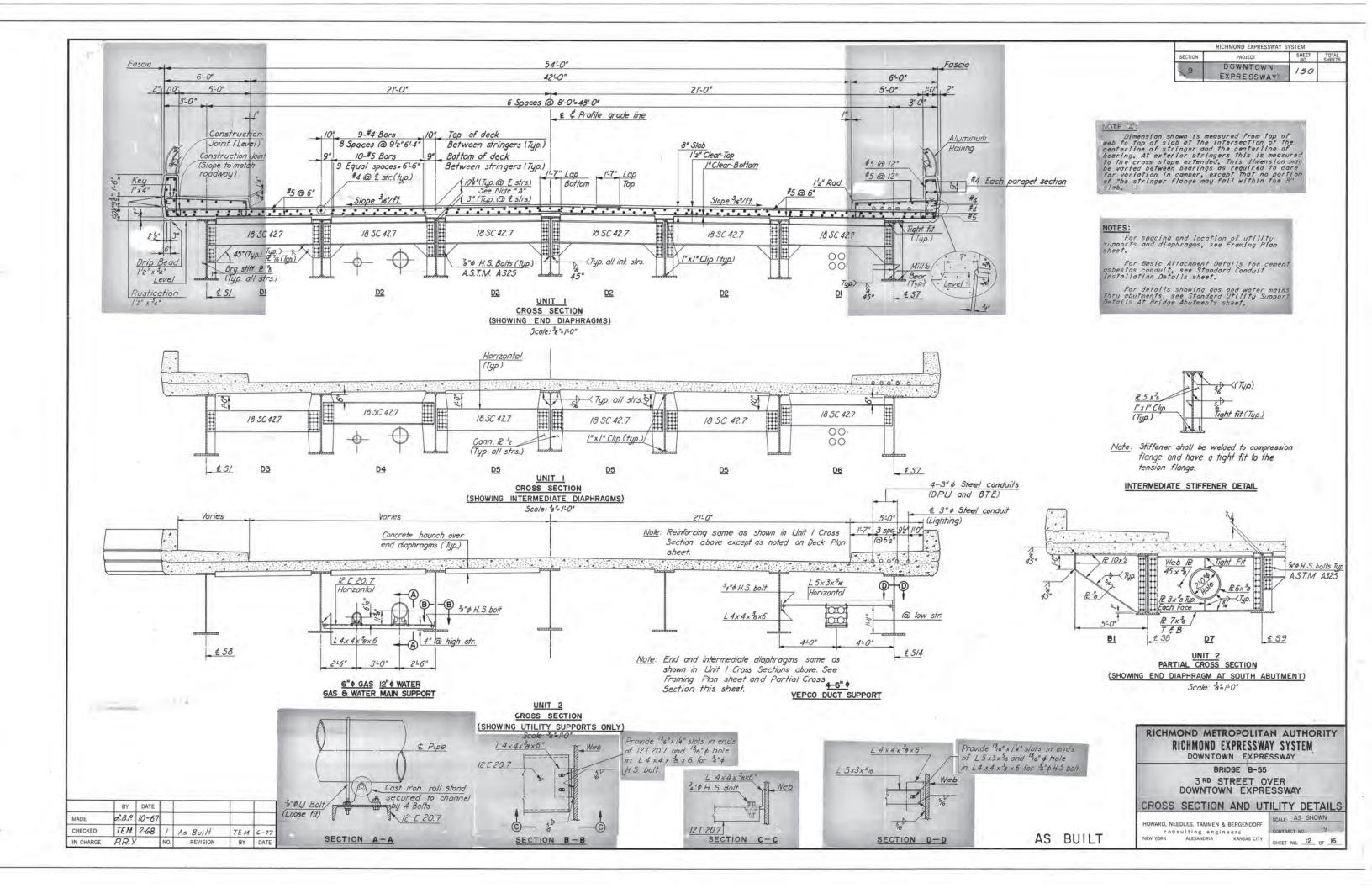


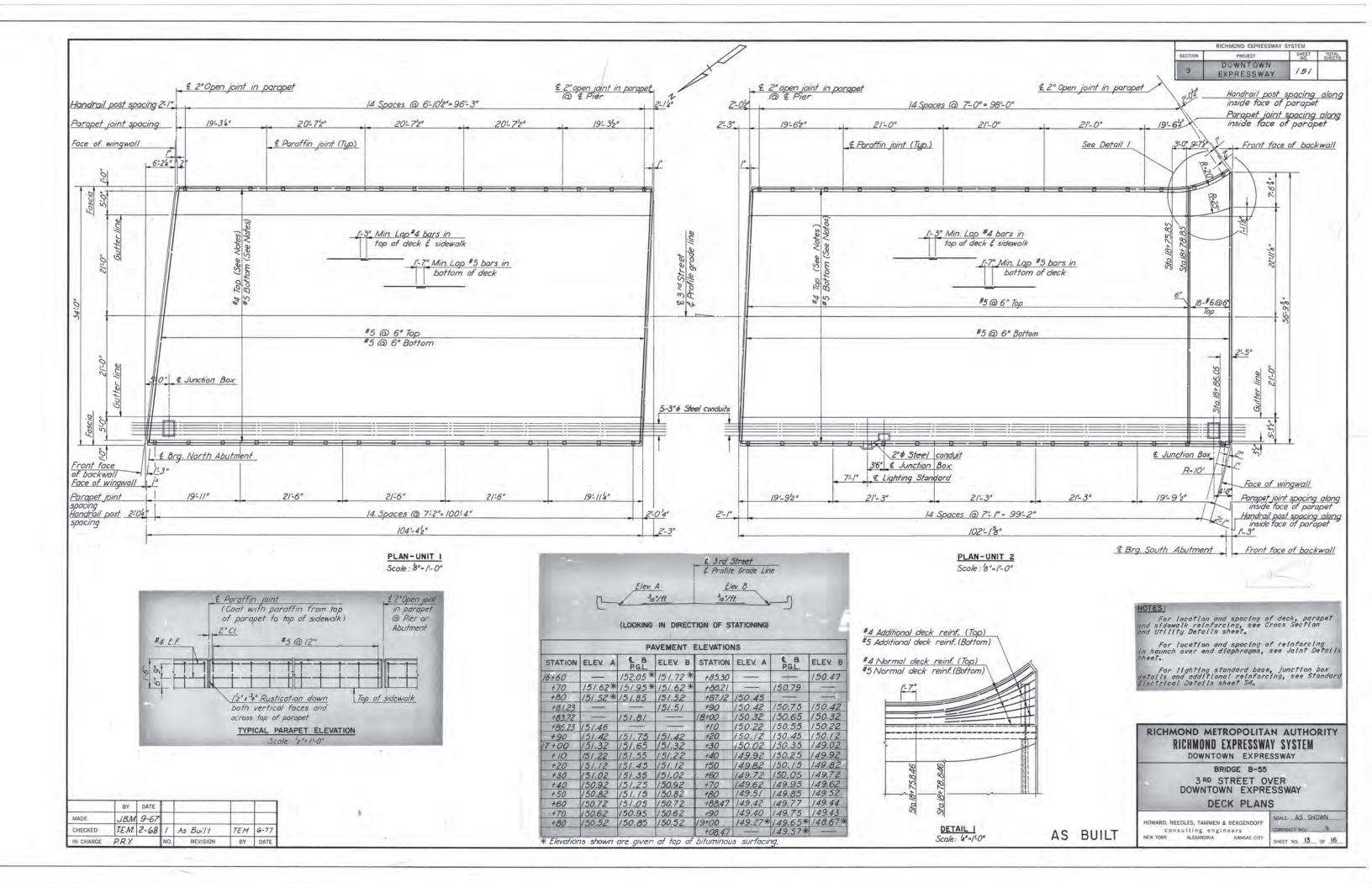


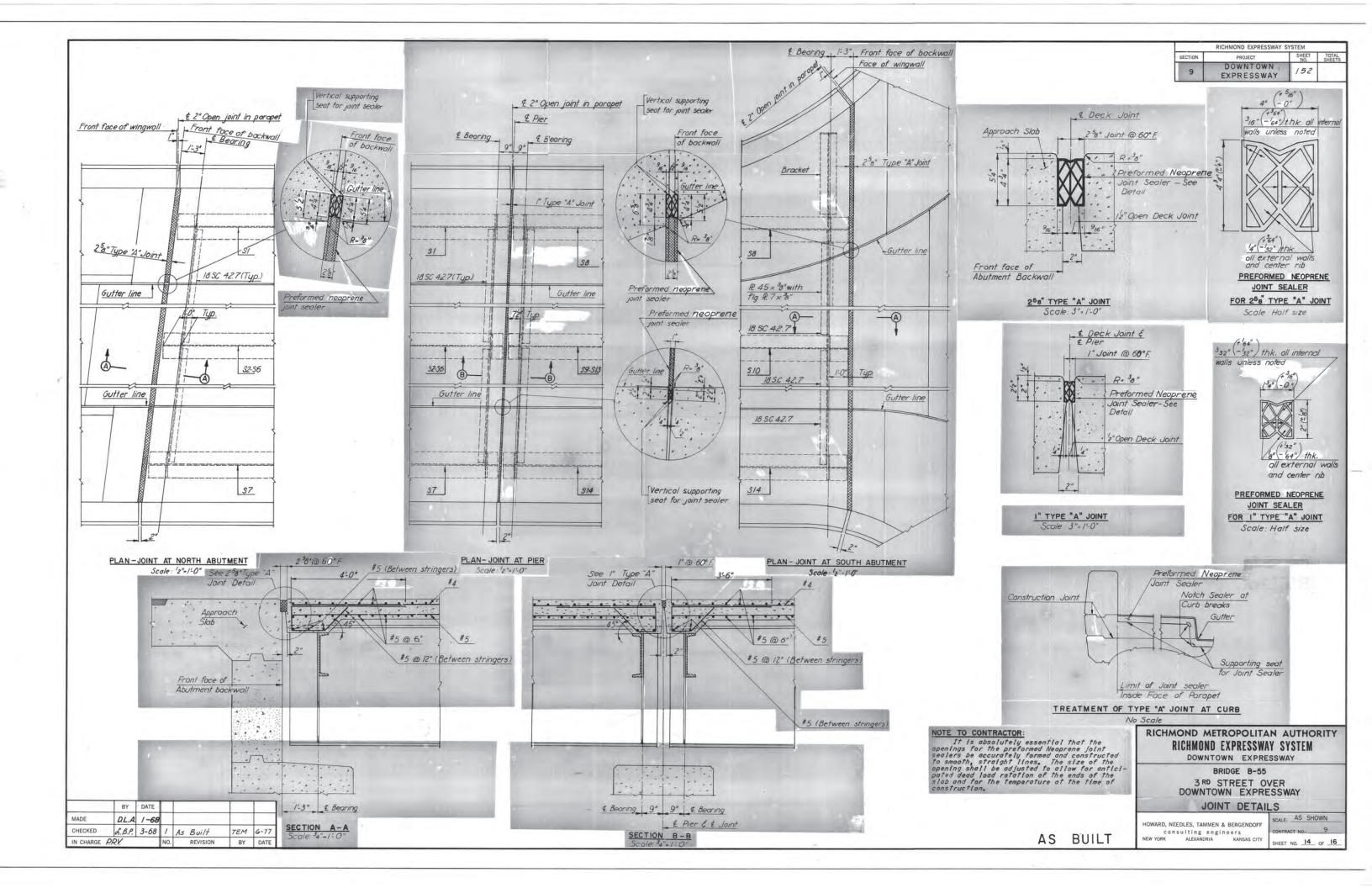


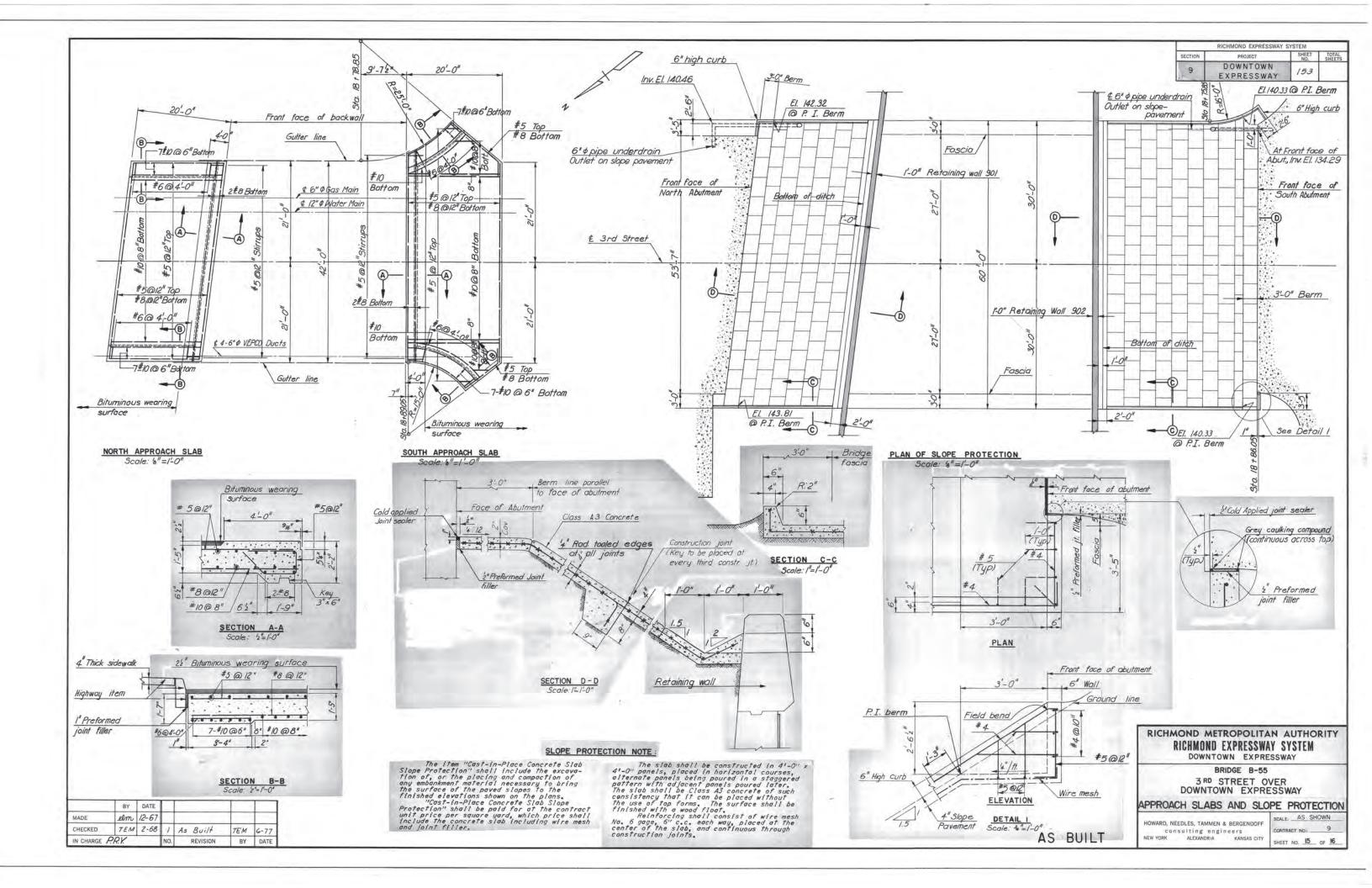












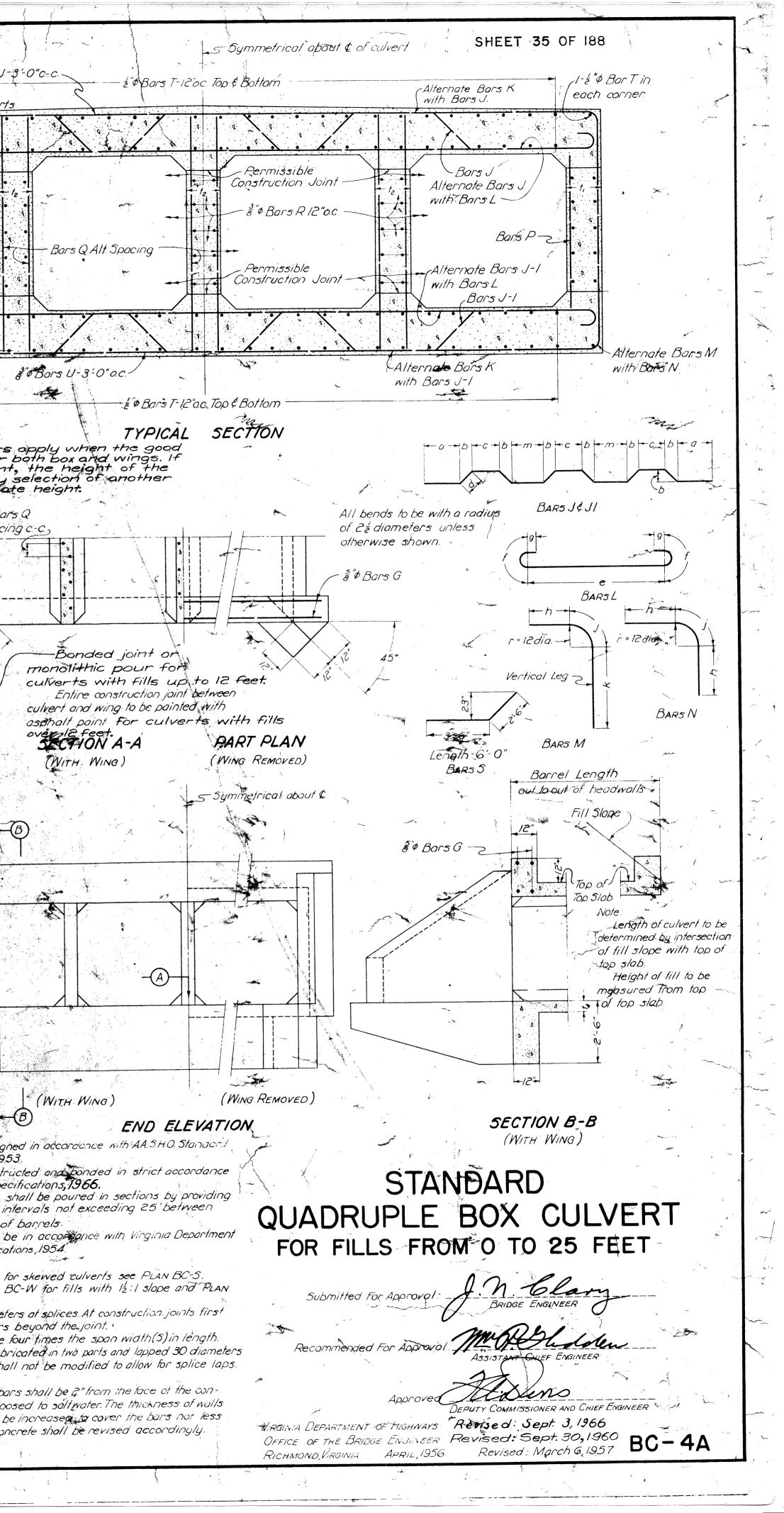
### **RCBC 1827 Plans**

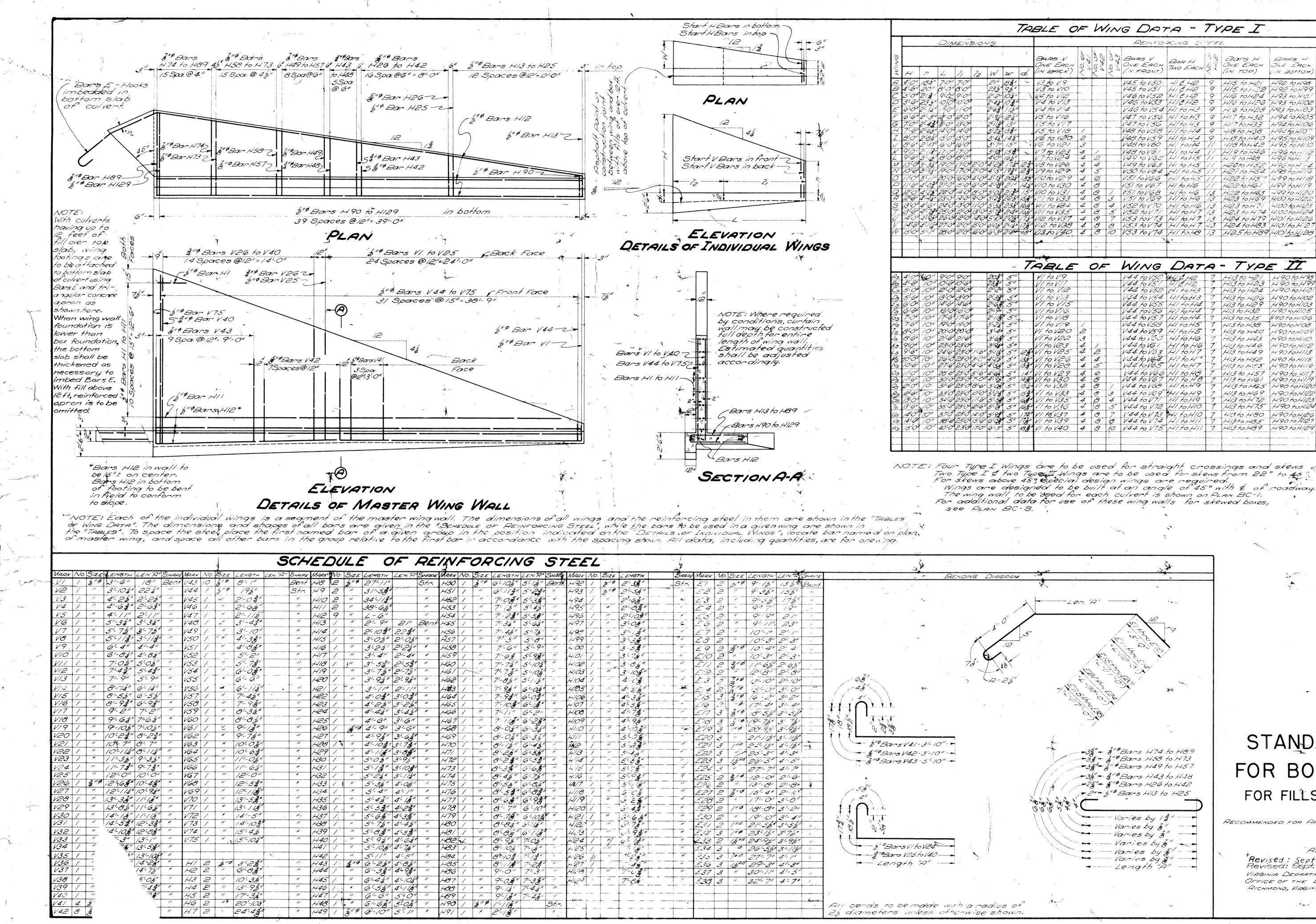
### 4 @ 14'X10'

(Under Forest Hill Ave Off Ramp)

		FILLS UP TO IS FEET			QUANTITIES	
Limensions HOILING	Bors J-1 Bors K Dimensions Dimensions	Bars L Bors M U Dimensions & Dimen. & Dimensions & Dimen. & Dimensions & Dimen. &	Bors N Bors P Bors C U Dimen 200 500 500 500 500 500 500 500 500 500	Bars G Borr Solow Harris Construction	Steel 5 sr Ft. Conc. 7 Steel 6 Ydwolls 9 Ydwolls 4 Ydwolls 4 Ydwolls 4 Ydwolls 4 Ydwolls 7 Ydwolls 4 Ydwolls 7 Ydwolls 7 Ydwoll 7	Slope & IE for all culverts
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4' 5' 6'' 6'' 6'' 6'' 6'' 6'' 18' 6'' 3' 4'' 2'' 2'' 2''' 2''' 2''' 2''' 2'	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	-4" 50 76 8 19-10" 52 1.308 -4" 20 92 8 23-10" 28 1.218	176.27 5.77 342 E 185.33 5.96 374 G 213,65 6.22 311 A 223.10 6.43 343 C	Hog Bars R
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$5_{2}^{3''} 22'7'' 21' 10'' 3'' 12'' 30'' 62'' 4'3'' 2'7'' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3'' 12'' 30'' 62'' 4'9'' 3'1''' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3'' 12'' 30'' 62'' 5'3'' 3'7'' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3''' 12'' 30'' 62'' 5'-9'' 4'1''' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3''' 12'' 30'' 62'' 5'-9'' 4'1''' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3''' 12'' 30'' 62'' 5'-9'' 4'1''' 30''  5_{2}^{3''} 22'7'' 21' 10'' 3''' 12'' 30'' 62'' 5'-9'' 4'1''' 30''  5_{2}^{3''} 22'' 21' 10'' 3''' 12'' 30'' 62'' 5'-9'' 4'1''' 30''  5_{2}^{3''} 22'' 30'' 30'' 30'' 12'' 30'' 30'' 30'' 30'' 30'' 30'' 30'' 3$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	-4" 40 92 8 23-10 44 1.402 -4" 50 92 8 23-10 52 1.494	223.10 6.43 343 C 231.26 6.63 375 E 239.42 6.84 407 G 249.54 7.05 439 I	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\frac{7}{8} \frac{7}{8} \frac{7}$	$b 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*''} \frac{3}{8} \phi 7_{2}^{*'} 4^{-6''} 2^{-8''} \frac{3}{8} \phi \\ b 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*''} \frac{3}{8} \phi 7'' 5^{-0''} 3^{-2''} \frac{3}{8} \phi \\ \phi 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*''} \frac{3}{8} \phi 7'' 5^{-6''} 3^{-8''} \frac{3}{8} \phi \\ \phi 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*''} \frac{3}{8} \phi 7'' 5^{-6''} 3^{-8''} \frac{3}{8} \phi \\ \phi 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*''} \frac{3}{8} \phi 7'' 5^{-6'''} 3^{-8'''} \frac{3}{8} \phi \\ \phi 5_{2}^{*''} 26^{-7''} 25^{-1}0'' 3'' 1_{2}^{*'''} \frac{3}{8} \phi 7''' 5^{-6'''} 3^{-8'''} \frac{3}{8} \phi \\ \phi 5_{2}^{*'''} 26^{-7'''} 25^{-1}0'' 3''' 1_{2}^{*''''} \frac{3}{8} \phi 7''' 5^{-6'''''} 3^{-8'''''} \frac{3}{8} \phi \\ \phi 5_{2}^{*''''} 26^{-7''''''''''''''''''''''''''''''''''''$	72     3-1"     15"     7"     34     12"     4-0"     3"     6"     4       7"     3-1"     15"     7"     34     12"     5-0"     3"     6"     4       7"     3-1"     15"     7"     34     12"     5-0"     3"     6"     4       7"     3-1"     15"     7"     34     12"     5-0"     3"     6"     6"       7"     3-1"     15"     7"     34     12"     6-0"     3"     6"     6"	*6"     30     108     8     27'10"     36     1.711       *6"     40     108     8     27'10"     44     1.802       *6"     50     108     8     27'10"     52     1.895	260.52 7.13 377 C 269.88 7.33 409 E 271.86 7.54 441 G	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	1" 20 6" 30 11" 5-48" 4" 3-7" 58" 3-4" 20 6" 208 2	$ 5\frac{3^{*}}{4^{*}} 2G^{+}7^{*} 25^{+}10^{*} 3^{*} 1^{*}_{2} \frac{3^{*}}{8} \phi G^{+}_{2} G^{-} G^{-} 4^{+}2^{*} \frac{3^{*}}{8} \phi G^{+}_{2} G^{+}_{2} G^{+}_{2} G^{+}_{2} G^{+}_{3} G^{+}_{4} G^{+}_{2} G^{+}_{3} G^{+}_{4} G^{+$	and the second	26 60 108 8 2710° 60 1.986 36 70 108 8 2710° 68 2.078 147 30 124 8 31-10° 36 2.054	27125 821 171 01.	*Wing walls catted for by letters foundation level is the same for
$7' \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c} 5 \\ 6'' \\ 31' \\ 0'' \\ 0$	G" 3:5" 17" 7" 2018 8:0" 20 G" 2 53" 3:5" 1.7" 7" 2018 10:0" 20 G" 2	27" 70 124 8 31-10" 68 2496	357.78 8.54 338 K 368.51 8.95 607 0	foundation levels are different wing wall shall be adjusted by lettered wing wall of appropriat
8 82 82 6 82 9" 34 52 355 648 42" 41 63 34	$\frac{3''}{8''} \frac{3''}{6} \frac{5_{2'}}{2} \frac{35' \cdot 3''}{35' \cdot 4''} \frac{6' \cdot l_{4'}}{5''} \frac{5''}{4' \cdot l'''} \frac{4' \cdot l''}{7''} \frac{3' \cdot 8''}{3' \cdot 8''} \frac{2''}{2''} \frac{5_{2'}}{22' \cdot l''} \frac{22' \cdot l''}{2} \frac{2}{2} \frac{35' \cdot 4''}{5''} \frac{6' \cdot l_{4'}}{5''} \frac{5'''}{4' \cdot l'''} \frac{7''}{7''} \frac{3' \cdot 8''}{3' \cdot 8''} \frac{2''}{2''} \frac{5_{2'}}{22' \cdot l'''} \frac{2}{2} \frac{22' \cdot l''}{2} \frac{2}{2} \frac{35'' \cdot 6''}{5'''} \frac{6' \cdot 2_{4''}}{5'''} \frac{5'''}{4' \cdot l'''} \frac{7'''}{7'''} \frac{3' \cdot 8''}{3' \cdot 0''} \frac{2}{2} \frac{5}{2} \frac{22' \cdot l'''}{22' \cdot l'''} \frac{2}{2} \frac{35'' \cdot 6'''}{5'''} \frac{22' \cdot l'''}{5'''} \frac{2}{5'''} \frac{22' \cdot l'''}{5'''} \frac{2}{5'''} \frac{22' \cdot l'''}{5'''} \frac{2}{5'''} \frac{22' \cdot l'''}{5''''} \frac{2}{5''''} \frac{3' \cdot 6''''}{5''''} \frac{3' \cdot 6''''}{5'''''} \frac{3' \cdot 6'''''}{5''''''} \frac{3' \cdot 6''''''}{5''''''''''''''''''''''''''''''$	$ \begin{array}{c} \phi & f_{2}^{z} & f_{2}^{-0} & f_{3}^{-0} & f_{4}^{-0} & f_{4}^{$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	59" 40 140 8 3510° 52 2.763 39" 60 140 8 3510° 68 3.025 09 80 140 8 364" 84 3.489	391.21 8.85 507 H 408.57 9.11 572 L 437.40 9.49 640 P	Bock face of Curtain Wall
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\frac{3''}{2} \frac{2}{0} \frac{5}{2} \frac{39}{39} \frac{6''}{6} \frac{6'}{1!\frac{1}{2}} \frac{5}{2} \frac{1''}{2} \frac{4'5''}{4'5''} \frac{7}{4''} \frac{4'3''}{4'3''} \frac{2}{2} \frac{5}{2} \frac{5}{2} \frac{25'4'}{23} \frac{1}{23} \frac{1}{2} \frac{1}{2} \frac{5}{2} \frac{5}{2} \frac{1}{2} 1$	"\$ 52" 39:4" 38-4" 4" 2" 30 G" 7-2" 4-10" 30	$ \begin{array}{c} 6 & G_2'' & 4'-1'' & 21'' & 7'' & \frac{1}{2} \phi & 18'' & G^{-} O'' & \frac{3''}{8} \phi & G^{*} & 0 \\ 6 & G^{*} & 4^{-}1'' & 21''' & 7'' & \frac{1}{2} \phi & 15'' & 8^{-} O'' & \frac{3''}{8} \phi & G^{-} & 0 \\ \end{array} $	140 20 156 8 3940 36 2.988 642 40 154 8 3940 52 3.222 340 60 156 8 3940 68 3.466 046 80 156 8 404 84 3.904	465.05 9.44 541. H 486.16 9.74 605 L	2" Bars 5-6"c-c. For fills up to the former fills of the former fills of the former fills of the former for
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$5'' \frac{1}{2} \phi G'' 40' \frac{2}{2} (7') \frac{1}{2} (5''_{2} + 4'5') \frac{7}{4} (4'5''_{2} + 4'5''_{$	$ \begin{array}{c} & \phi & G^{\prime\prime} & 40^{\prime} G^{\prime\prime} & 39^{\prime} G^{\prime\prime} & 4^{\prime\prime} & 2^{\prime\prime} & 8 \phi & 5 \frac{3}{4}^{\prime\prime} & 9^{\prime} \cdot 3^{\prime\prime} & G^{\prime} 10^{\prime\prime} & 8 \phi \\ & \phi & 8 \frac{1}{4}^{\prime\prime} & 43^{\prime} \cdot 3^{\prime\prime} & 42^{\prime} \cdot 0^{\prime\prime} & 4 \frac{7}{8}^{\prime\prime} & 2 \frac{5}{8}^{\prime\prime} & \frac{1}{2}^{\prime} \phi & 9 \frac{1}{2}^{\prime} & 5^{\prime} \cdot 9^{\prime\prime} & 2^{\prime} \cdot 11 \frac{3}{4}^{\prime\prime} & \frac{1}{2}^{\prime} \phi \\ \end{array} $	0 5 4 4 3" 22" 7" 20 15" 120" 10 12" 8 9 2 4 9" 23 4 9 2" 50 18 4 0 30 6	240 100 156 8 40-4 04 3.304 240 100 156 8 40-10 100 4.500 5-0 20 172 8 43-10 36 3.479 7-0 40 172 8 43-10 52 3.730	531.95 10.46 742 F 531.24 9.85 510 D	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$5'' \frac{8}{8} \phi \ B_4^{I''} 43^{-} 1/1'' 7^{+} 7\frac{8}{8} G_2^{I''} 5^{+} 1'' 9\frac{1}{4} 4^{+} 5'' \frac{8}{8} \phi \ B_4^{I''} 28^{+} 1'' \frac{8}{8} 6\frac{1}{4} 28^{+} 1'' \frac{8}{8} 6\frac{1}{4} 28^{+} 1'' \frac{8}{8} 6\frac{1}{4} 28^{+} 1'' \frac{8}{8} \frac{8}{4} 6\frac{1}{4} 28^{+} 1'' \frac{8}{8} \frac{8}{4} 6\frac{1}{4} 28^{+} 1'' \frac{8}{8} \frac{8}{4} \frac{8}{4} \frac{8}{4} 28^{+} 1'' \frac{8}{8} \frac{8}{4} \frac{8}{4}$	"\$ 84" 43'6" 42'-3" 48" 28" 3" 6" 7'-5" 4'-11" 8"	6" 4-5" 23" 7" 1" 15" 8-0" 3" 6"	9:0° 60 172 8 43:10° 68 4042 12'0° 80 172 8 44:4° 84 4.481 3:0° 100 112 8 44:4° 100 5083	562.26 10.31 G38 L 587.08 10.74 707 P	of Wing't
DIMENSIONS Bors J		OM I2 FEET       TO       25       FEET         G STEEL (See Note for Bars U)       Bars L       Bars M	Bars N Bors P Bors		QUANTITUES rel 2 H'd walls	Lengin
pon de WI Thick de WI Thick t W'i Thick de WI Thick t W'i Thick de WI Thick de MI Thick de	of Bars Dimensions Sof Bars Softh Construction Softh Construction	Dimensions Dimensions Coing c-c Cring c-c Cring c-c	prime con day prime con day prime con Bars con Bars con Bars con day c	ength	eint. Steel 35 per Fil 35 A <sub>3</sub> Conc in Häwolls einf. Steel 5 in Häwal 7 Nolls N	
$\begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \end{array} \\ \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \end{array} \\ \begin{array}{c} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \\ \end{array} \\ \end{array} \\ \end{array} \\ \end{array} \\ \begin{array}{c} \end{array} \\ \begin{array}{c} \end{array} \\ \end{array} $	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c} & & \\ & & \\ & & \\ \phi \end{array} = \begin{array}{c} & & \\ & & \\ & & \\ \phi \end{array} = \begin{array}{c} & & \\ & & \\ & & \\ \phi \end{array} = \begin{array}{c} & & \\ & & \\ & & \\ & $	20 60 8 15-10° 0.966	Q     Image: Signature     Q     Image: Signature     Image: Signature <td></td>	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c} & \phi & \phi \\ $	\$	4'6 30 76 8 19'10 1.263 5-6' 40 76 8 19'10 1.356	203.05 5.23 166 A 199.17 5.44 166 C 207.44 5.65 166 E 216.77 5.85 166 G	e Bors
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c} & \phi & 5_{2}^{-1} &  8^{-7}{}^{*} &  7^{*0}0'' & 3^{*} &  _{2}^{-1} & _{8}^{-6} & 5_{4}^{-3} & 5^{-4}{}^{*} & 3^{-8}{}^{*} \\ & & 3^{*}\phi & 5_{2}^{+''} & 22^{*}7^{*'} & 24^{*}10^{*'} & 3^{*'} &  _{2}^{+''} & _{8}^{-6} & 7^{*'} & 4^{-3}{}^{*'} & 2^{-4}{}^{*'} & _{8}^{-6} \\ & & & 3^{*}\phi & 5_{2}^{+''} & 22^{*}7^{*''} & 21^{-1}0^{*'} & 3^{*'} &  _{2}^{+''} & _{8}^{-6} & 7^{*''} & 4^{-9}{}^{*'} & 2^{-1}0^{*'} & _{8}^{-6} \\ & & & & 3^{*}\phi & 5_{2}^{+'''} & 22^{-7}{}^{*''} & 21^{-1}0^{*''} & 3^{*'} & 1_{2}^{+'''} & _{8}^{-6} & 6_{1}^{-(''''''''''''''''''''''''''''''''''''$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	4-9 30 92 8 23-10 1.690	216.77       5.85       166       G         232.62       5.96       199       B         240.68       6.17       199       D         250.42       6.37       199       F	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3 \$\$ 52 22-7" 21-10" 3" 12" 3 \$ 6 5-9" 3-10" 3 5 \$ 52 22-7" 21-10" 3" 12" 30 6" 5-9" 3-10" 3 5 \$ 52 22-7" 21-10" 3" 12" 50 6" 6-3" 4-4" 8	\$	7.9°60 92 8 2340° 1.965 4-11° 50° 108 8 27-10° 2.117	260.66         6.58         199         H           268.93         6.79         199         J           305.00         686         232         D	
$5' G'' G'' 9_{2}'' 10'' \frac{1''}{2} G'' 27'3'' 4'4_{8}'' G_{8}'' 3'' 2'' 8_{8}'' 2'' G' G'' G'' 9_{2}''' 10'' \frac{1''}{2} G'' 27'3'' 4'4_{8}'' G_{8}'' 3'' 2'' 8_{8}'' 2'' G' G'' G'' G'' 9_{2}''' 10'' \frac{1''}{2} G'' G'' 27'' 4'' G_{8}'' 3'' 2'' 8_{8}'' 2'' G' G'' G'' G'' 9_{2}''' 10'' 10'' 10'' 10'' 10'' 10'' 10'' $	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\frac{1}{2} \phi 7\frac{1}{2} 26^{2} 9^{*} 25^{2} 9^{*} 4^{*} 2^{*} \frac{3^{*}}{8} \phi 6\frac{1}{2} 5\frac{1}{2} 6^{*} 3\frac{1}{2} 5^{*} \frac{3}{8} \frac{3}{8} \frac{1}{2} \frac{1}{2} 6^{*} \frac{3^{*}}{2} 5^{*} \frac{3}{8} \frac{3}{8} \frac{1}{8} \frac{1}{8$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	7-11 60 108 8 27-10 2.418	32482 7.27 232 H 347.80 7.42 232 J	Note: Copacity ## 20-516 Loading ; designed
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	5'2" 30 124 8 31-10" 2.696 7:2" 50 124 8 31-10" 2.880	357.77       7.57       232       L         362.58       7.41       .266       D         378.90       7.83       266       H         405.88       8.18       266       L	Specifications for Highway Bridges,195 Construction joints shall be constru- with the current Road and Bridge Speci Barrels more than 35' in length si construction joints in vertical planes at in
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\frac{1}{2} \phi  7_{2}^{t''}  3l^{-} 7^{*}  30^{-} 7^{''}  4^{''}  2^{''}  \frac{1}{2} \phi  7_{2}^{t''}  8^{+} 8^{''}  6^{-} l_{4}^{t''}  \frac{1}{2} \\ \frac{1}{2}^{''} \phi  5_{2}^{t''}  34^{-} 9^{*}  33^{-} 9^{*}  4^{''}  2^{''}  \frac{3}{8} \phi  6_{2}^{t''}  5^{-} 8^{''}  3^{-} 2^{''}  \frac{3}{8} \\ \frac{1}{2}^{''} \phi  5_{3}^{t''}  34^{-} 9^{*}  33^{-} 9^{*}  4^{''}  2^{''}  \frac{3}{8} \phi  6_{2}^{t''}  6^{-} 8^{''}  4^{-} 2^{''}  \frac{3}{8} \\ \frac{1}{2} \phi  5_{3}^{t''}  34^{-} 9^{*}  33^{-} 9^{*}  4^{''}  2^{''}  \frac{3}{8} \phi  6_{2}^{t''}  6^{-} 8^{*'}  4^{-} 2^{''}  \frac{3}{8} \\ \frac{1}{2} \phi  5_{3}^{t''}  54^{-} 9^{*'}  33^{-} 9^{*'}  4^{''}  2^{''}  \frac{3}{8} \phi  6_{2}^{t''}  6^{-} 8^{*'}  4^{-} 2^{''}  \frac{3}{8} \\ \frac{1}{2} \phi  5_{3}^{t'''}  54^{-} 9^{*''}  33^{-} 9^{*''}  4^{'''}  2^{''}  \frac{3}{8} \phi  6_{2}^{t'''}  6^{-} 8^{*'''}  4^{-} 2^{''''''''''''''''''''''''''''''''''''$	$ \begin{array}{c} \phi & 7_{\overline{z}}^{I''} & 4^{I}4^{I''} & 2l_{4}^{I'''} & 9l_{\overline{z}}^{I''} & l_{\overline{z}}^{I''} & 7^{I''} & 10^{I}0^{\bullet} & l_{\overline{z}}^{I''} & 9^{I''} \\ \phi & G_{\overline{z}}^{I''} & 4^{I}5^{I''} & 23^{I''} & 7^{I''} & \theta & 12^{I''} & 4^{I}0^{I''} & \theta & G^{I''} \\ \phi & G_{\overline{z}}^{I'''} & 4^{I}5^{I''} & 23^{I''} & 7^{I''} & \theta & 12^{I''} & 6^{I}0^{I''} & \theta & 6^{I'''} \\ \phi & G_{\overline{z}}^{I'''} & 4^{I}5^{I''} & 23^{I''} & 7^{I'''} & \theta & 12^{I''} & 6^{I}0^{I''} & \theta & 6^{I'''} \\ \end{array} $	7'5" 40 140 8 3540 3.541	44089 845 270 P 425.49 887 299 D 44180 829 299 H	joints nor more than "30' from ends of Materials and construction shall be of Highways Road and Bridge Specificati All concrete shall be Class <sup>t</sup> A <sub>3</sub> .
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$\frac{1}{2} \begin{bmatrix} \frac{1}{2} \phi & G^{*} & 3G^{*} \phi & 5^{-} 1 \end{bmatrix}^{*} \begin{array}{c} \phi & 4^{*} \phi & 1 \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 5^{-} 1 \end{bmatrix}^{*} \begin{array}{c} \phi & 4^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & G^{*} & 23^{*} \phi \\ \phi & 5^{*} \phi & 7^{*} \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \\ \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c} \phi & 0 \end{array} \xrightarrow{2} \begin{array}{c}$	$\frac{1}{2} \phi  G^*  35^{-} 7^*  34^{-} 7^*  4^*  2^*  \frac{1}{2} \phi  8_{4}^{-}  9^{-} O^*  G^{-} 2_{4}^{-}  \frac{1}{2} \\ \frac{5}{2} \phi  7^*  38^{-} 11^{**}  37^{-} 8^{**}  4_{4}^{-*}  2_{4}^{-*}  \frac{1}{2} \phi  7^*  5^{-} 11^{**}  3^{-} 3^{**} \\ \frac{5}{2} \phi  7^*  5^{-} 11^{**}  3^{-} 3^{**}  \frac{1}{2} \phi  7^{**} $	$ \begin{array}{c} \phi & 8_{4}^{*} & 4^{2} g^{*} & 23_{4}^{*} & 9_{2}^{*} & \overline{z} \phi & 9 & 10^{-0} & z\psi & 9 \\ \phi & 7^{''} & 4^{1} g^{*} & 2^{1} 1^{''} & 7^{''} & \overline{g} \phi & 12^{''} & 4^{1} 0^{''} & \overline{g} \phi & G^{''} \end{array} $	11'5" 80 140 8 36'4" 4.289 5'7" 20 156 8 39'10 3.981	440.43       8.64       299       L         488.23       8.92       303       P         539.59       8.38       332       D         558.02       8.79       332       H	For modification of above details for For details of wing walls see PLAN BU BC-WI for fills with 2:1 slope Bars R and T shall lop 30 diameter
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{c} 0^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7^{\prime\prime} & 40^{2} 0^{\prime\prime} & G^{2} 3 \underbrace{!}^{\prime\prime} & 10^{\prime\prime} & 4^{2} 9^{\prime\prime} &  4 \underbrace{!}^{\prime\prime} & 3^{\prime} 1^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7^{\prime\prime} & 2G^{\prime} 2^{\prime\prime} \\ \hline 0^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime} & 40^{2} 1^{\prime\prime} & G^{4} 4^{\prime\prime} & 10^{\prime\prime} & 4^{2} 9^{\prime\prime} &  4 \underbrace{!}^{\prime\prime} & 3^{\prime} 1^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime\prime} & 2G^{\prime} 2^{\prime\prime} \\ \hline 0^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime\prime} & 40^{2} 10^{\prime\prime} & G^{\prime} G \underbrace{!}^{\prime\prime} & 10^{\prime\prime} & 4^{\prime} 9^{\prime\prime} &  4 \underbrace{!}^{\prime\prime} & 3^{\prime} \cdot 3^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime\prime} & 2G^{\prime} 2^{\prime\prime} \\ \hline 22^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime\prime} & 40^{2} 10^{\prime\prime} & G^{\prime} G \underbrace{!}^{\prime\prime} & 10^{\prime\prime} & 4^{\prime} 9^{\prime\prime} &  4 \underbrace{!}^{\prime\prime} & 3^{\prime} \cdot 3^{\prime\prime} & \underbrace{\$}^{\prime} \phi & 7 \underbrace{!}^{\prime\prime} & 2G^{\prime} 2^{\prime\prime} \\ \hline 26^{\prime} 2^{\prime\prime} & \underbrace{!}^{\prime\prime} & 10^{\prime\prime} & 4^{\prime} 9^{\prime\prime} &  4 \underbrace{!}^{\prime\prime} & 3^{\prime} \cdot 3^{\prime\prime} & \underbrace{!}^{\prime\prime} \phi & 10^{\prime\prime} & 1$	$\frac{3}{8}\phi 7'' 38''1'' 37'8'' 4'' 2^{*}_{8} 3^{*}_{8}\phi G_{2}'' G''1''' 4''3''' 3^{*}_{8}$ $\frac{3}{8}\phi 7_{2}'' 39'0' 37'9' 4^{*}_{8}'' 2^{*}_{8} 3''_{9}\phi G_{2}'' 7'11''' 5'3'' 3^{*}_{8}$ $\frac{3}{8}\phi 7_{2}'' 39'9' 38'G'' 4^{*}_{8}'' 2^{*}_{8} 2^{*}_{8} 3''_{9} 8'' 9'4'' G'4_{4}'' 2^{*}_{8}$	WOJE E 4 02 27 12 100 24 0.	9:7" 60 156 8 39:10" 4.381 11:7" 80 156 8 40:4" 4.917	558.02       0313       532       11         554.55       9.14       332       1         602.02       9.42       337       P         642.75       9.50       341       T	placed bars shall project 30 diameters Bars U are straight and shall be to Bars over 40' in length may be fabri at solices; quantities shown on plans shall Bars G,K,P and Q are straight.
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$			5:10" 20 172 8 13:10" 4828	GO2.46       8.79       366       E         GI9.21       9.14       366       I         GI2.44       9.53       367       M	The centers of main reinforcing bar crete except when the concrete is exposed and slabs exposed to salt water shall be than 3" and estimated quantity of conc
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c} -G'' \stackrel{\bullet}{\$} \stackrel{\bullet}{$} \phi 7'' 45' 1'' 7' 1 \stackrel{\bullet}{\$} \stackrel{\bullet}{$} h \stackrel{h}{$} \stackrel{h}{$} \stackrel{t}{$} 5' 3'' 1 \stackrel{\bullet}{$} \stackrel{h}{$} \frac{1}{$} \stackrel{\bullet}{$} 3' \stackrel{\bullet}{$} \frac{1}{$2} \phi 5 \stackrel{t}{$2'' 27' 2'' } \\ -28'' \stackrel{\bullet}{\$} \stackrel{\bullet}{$} \phi 7'' 46' 1''' 7 \stackrel{\cdot}{$} \stackrel{\bullet}{$} \stackrel{\bullet}{$} 11 \stackrel{t}{$2'' 5'' 3'' 16 \stackrel{\bullet}{$} \frac{1}{$} 3' 8'' \stackrel{t}{$2' \phi 5 \stackrel{t}{$} \frac{1}{$} 27' 2'' } \end{array} $	$\frac{5}{3} \frac{\phi}{\phi} \begin{array}{c} 7.^{"} \\ 43^{'} 9^{"} \\ 43^{'} 9^{"} \\ 43^{'} 43^{'} 43^{'} 43^{'} \\ 58 \frac{\phi}{\phi} \end{array} \begin{array}{c} 7^{"} \\ 28 \frac{\phi}{\delta} \end{array} \begin{array}{c} 25^{"} \\ \frac{1}{2} \frac{\phi}{\phi} \\ 82 \frac{\phi}{\delta} \end{array} \begin{array}{c} 9^{'} 8^{"} \\ 9^{'} 8^{'} \\ 6^{'} 54 \frac{\phi}{\delta} \end{array}$	$\frac{1}{2} \frac{1}{2} \frac{1}$	11-10" 80 172 8 44-4" 5.743 13-10 100 172 8 44-10" 6.439	652.43 9.76 370 Q 688.43 9.87 374 U	

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LEN."A"	SHAP	MARK	No.	ŜIZE	LENGTH	SAAPE	MARK	16.	S/z·E	LENGTH	LEN. 2	SHASE	BENDING DIAGRAM
5' ! 3"	Bent	H92	/	210	2.33	s.Str.	E1	2	210	9'-12"	13 2	Bent	
5'-23"	11	493	/	210	2'-55"		E2	2	11	9'35"	155"	1)	
5'-35	11	494	/	11	2-63"	16-	E3	2	"	9-55	175"		· · · · · · · · · · · · · · · · · · ·
5-42	11	495	/	-	2'-03"	1.	E4	2	"	91: 71	19"		
5'-5	"	496	1	11	21/03"	*	E5	2	//	91-91	21"		
5-64	".	497	/	"	3'-05"	14	26	2	11	91-11	23"	1,	
51-75	//	490	1	"	31-13"		E7	2	//	10'-1"	21-1"		
5'-8"	11	1-199		"	31-2511		E8	2	//	10'-3"		"	
51-91	11	4:00	1	11	3-53"		E9		510		CONTRACTOR AND A CONTRACTOR AND AND A CONTRACTOR AND A CONT		. 12
5-95	- 11	4.01		1	3-75		ETO	22	9	10-3"			
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	-,	H103		11	3-10-5-11		E:2	2	310	12:81	21.81		
5'-105 5'-112"	11	4104					E.3		710	12-0			-63" R=22"
				,	0			2	710	1		1	-4 <del>3</del> " **
6-03		H105			4' 28		E14	2	ā 3110	151-01		· · · · · · · · · · · · · · · · · · ·	
6-03"		1-106			4-371.		=15	3	310	16-2"	CONTRACTOR CONTRACTOR INCOME.	/	
6-13"		4107			4-55"		216	2	í	171-41			
61-2"		H108			41-73		E17	.3	8"\$	18'-52			
6-25		H109	/		4'-95"		E10	3	Z'ø	19-75		11	
6-34	//	HIIO	/	//	4'-103"	11 -	E19	3	1110	20-95		11	and the second s
6'-3 <u>3</u> "	"	H///	/	11	5-05	"	E20	3	"	21-112			
6'-4 2"	"	HN2	/	"	5-23"	//	.521	3	1110	24-12"	4'-12"	-*.	2"Bars V41-31-10"-
6-55	"	H113	/		5'-4 8"	. "	E22	3	1/100	25-3"			\$"\$ Bars V42-3'-10"-
6-54		H'14	/		5-53"		E23	3	12110				-Z" Bars V43-5-10"-
6-68	11	4,15	/	1	5-75	1	£24	3	11	27-7	41-71	<i>i</i> .	
6-75	"	4/16	/	//	5-931	1	E25	2	3/0	12.01		11	]
6-85	"	HX17	/	"	5-15		£26	2	11	131-8"			57"
6'-83	"	1.118	/	"	6.03	11	E27	2	3"9	15'- 4"		11	
6-93	"	H119	1	"	625	11	£28	2	11	171-01	3'-0"	11	
6-10	1 //	H120	/	"	6-42"	"	E29	2	110	181-81	31-21	<i>(.</i>	
6-10	2 11	H121	1	"	6-63	11	530	2	11	191-10			
5-114"	1	4122		//	61-75"	"	E.31	2	1110	211-5	31-5	4	De the the
5-117		HIL23	/	11	61-63 61-75" 61-75"	"	E.32	3	1110	23-15	3-76	1:	$1  X \times M $
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V26	4	4		V49 to V63	HI to H5	11		H96toH114			3.79 7	80	9.50	71
V27	4	5		V50 to V&4		11	H21 to H54				9.30 8			
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Len. "A"

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-35" - " Bars H74 to H89

+35 - 1" Bars H58 to H73

-35" - " Bars H49 to H57

-35" - 3" Bars H43 to H48

-25" \$"Bars H26 to H42

\* 2" = = = Bars H13 to H25

Varies by 13"

Varies by Z"

Varies by &"

--- Length "A"

Varies by 5" -

Varies by Z"

Varies by Z"

te-

STANDARD WINGS FOR BOX CULVERTS FOR FILLS WITH 2:1 SLOPE

RECOMMENDED FOR APPROVAL 1- Udder

APPROVED \_\_

VIRGINIA DEPARTMENT OF HIGHWAYS

OFFICE OF THE BRIDGE ENGINEER

\*Revised: Sept 3, 1966. Revised: Sept. 30, 1960.

Car

RICHMOND, VIRGINIA

42

BRIDGE ENGINEER

CHIEF ENGINEER

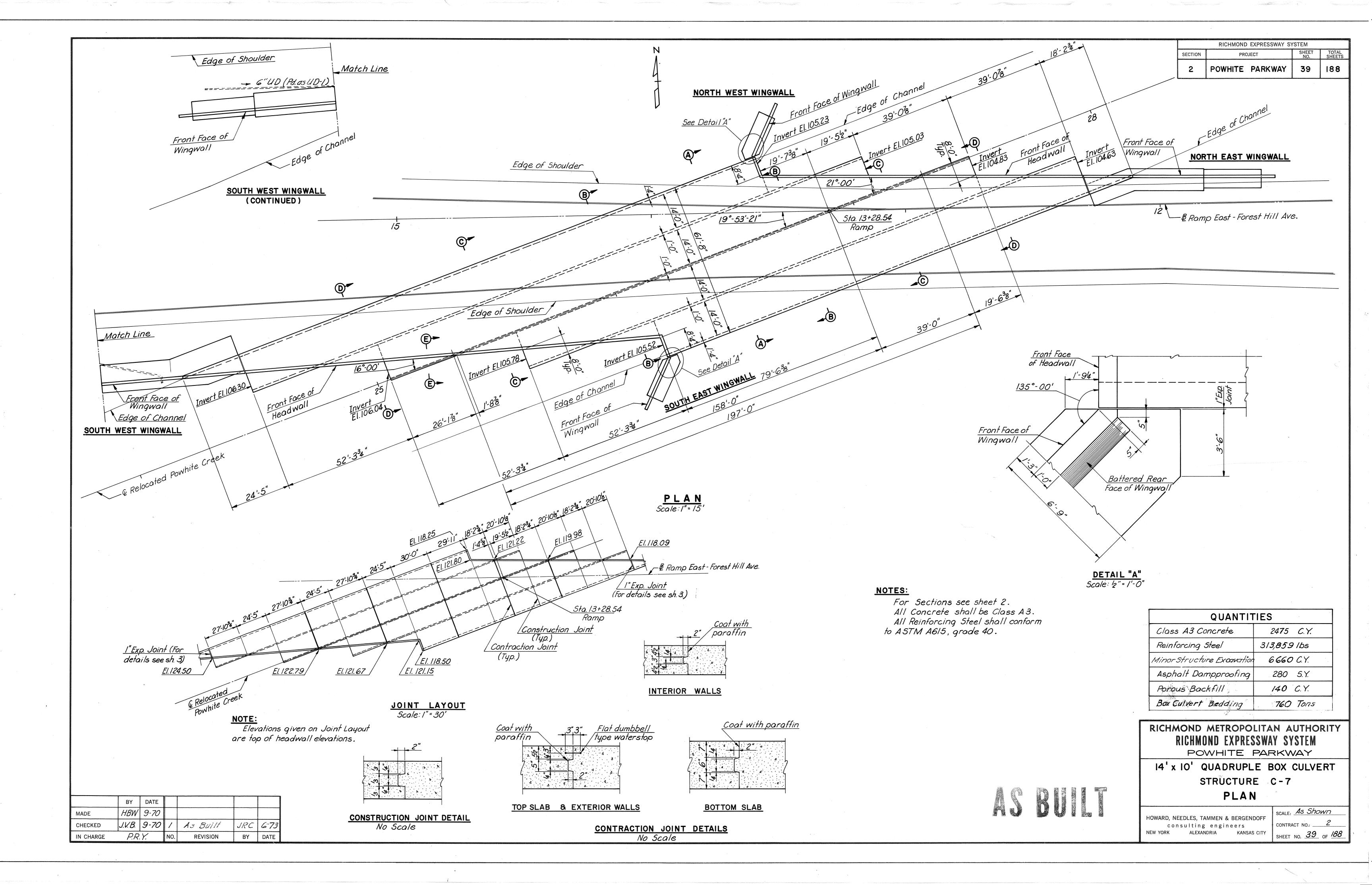
REVISED & RETRACED JAN., 1947

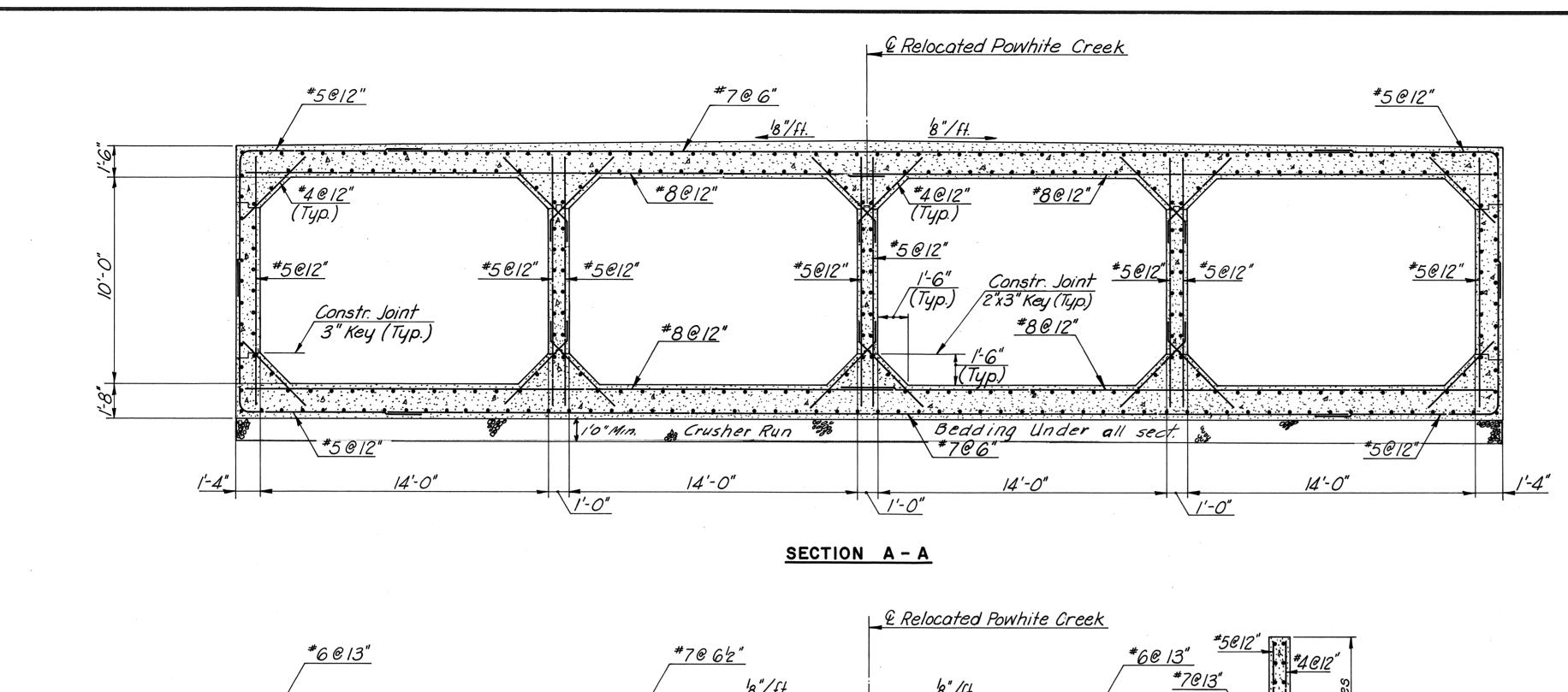
(MAY, 1946 (ORIGINAL)

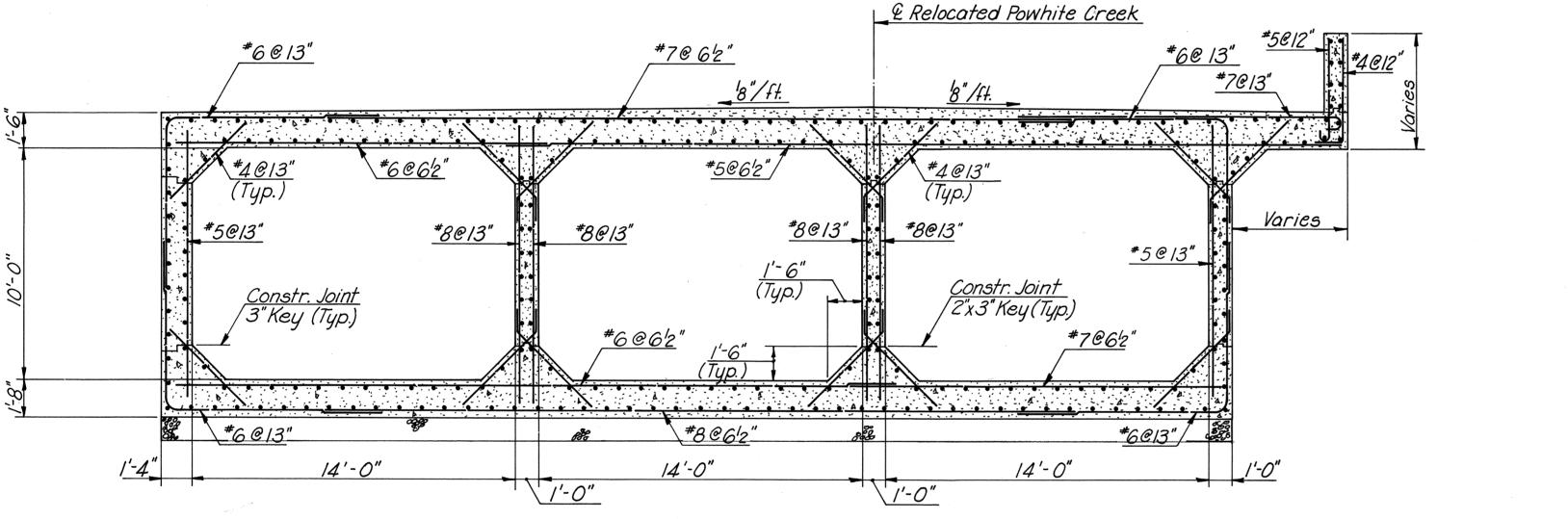
Revised May, 1958

6. P. Mulley

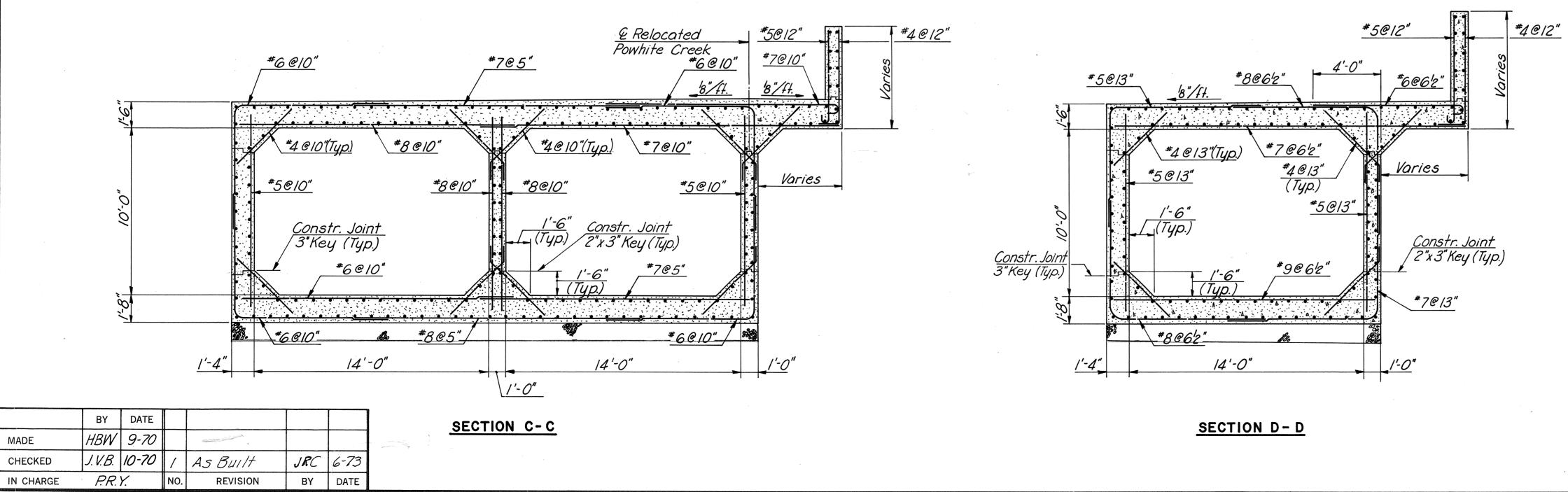
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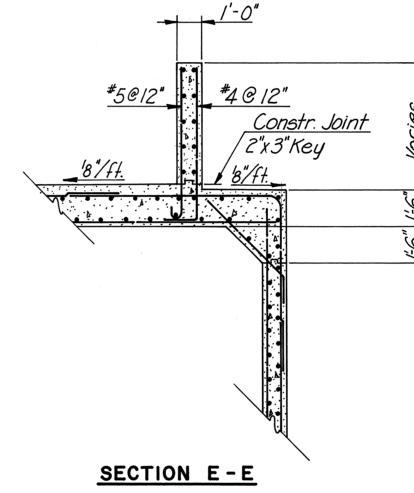




SECTION B-B



ف ب



	RICHMOND	EXPRESSWAY SY	/STEM	
SECTION	PRO	JECT	SHEET NO.	TOTAL SHEETS
2	POWHITE	PARKWAY	40	188

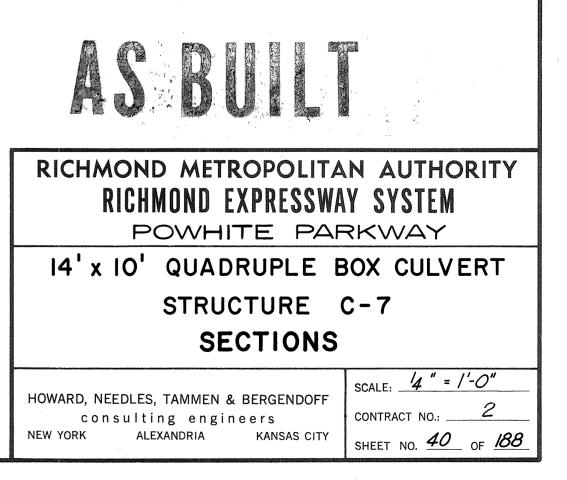
#### NOTES:

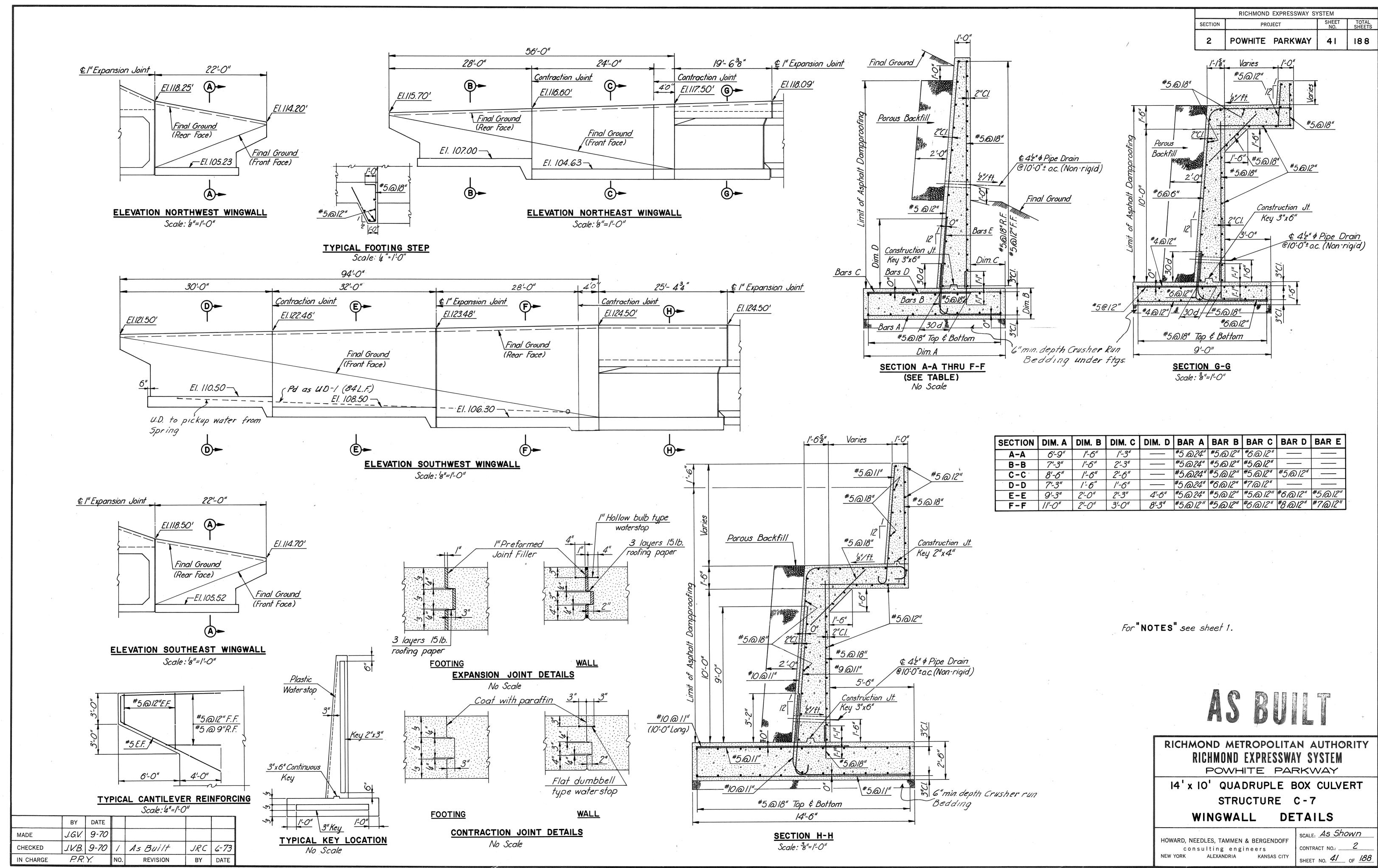
Laps shall be 30d.

Longitudinal bars shall be #4 spaced at approximately 12".

Corner bars shall have 12 d radius bends. Hooks shall have standard dimensions. All bars shall have a min. of 2" from center of bar to face of concrete except that bars in the bottom of bottom slab to be 3" from center of bar to face of concrete.

For location of Sections see sheet 1.





SECTION	DIM. A	DIM. B	DIM. C	DIM. D	BAR A	BAR B	BAR C	BAR D	BAR E
A-A	6'-9"	<i> '-6"</i>	/'-3"		#5 @24"	#5@12"	<i>#6@12"</i>		
B-B	7'-3"	1'-6"	2'-3"		#5@24"		#5@12"		
C – C <sup>•</sup>	8'-6"	1'-6"	2'-6"		#5/@24"	#5@12"	#5@12"	#5@I2"	
D-D	7'-3"	1'-6"	/'-6"		#5@24"	#6@I2"	#7@12"		
E-E	9'-3"	2'-0"	2'-3"	4'-6"	#5@24"	#5@12"	#5/@12"		#5@12"
F-F	11'-0"	2'-0"	3'-0"	8'-3"	#5@12"	#5@12"	#6@I2"	#8 @12"	#7@12"

### **RCBC 1831 Plans**

### 4 @ 10'X12'

(Under Powhite Parkway between POW-N and POW-S Toll Plazas)

	INDEX OF SHEETS
SHEET NO.	TITLE
1	COVER SHEET
2	GENERAL NOTES AND LEGEND
3 - 4	GEOMETRIC LAYOUT AND REFERENCES
5	SUMMARY OF EARTHWORK
6	TYPICAL ROADWAY SECTIONS
7 - 9	EXISTING CONDITIONS PLANS
10	SITE PLAN - POWHITE CREEK RELOCATION
11	GRADING PLAN - POWHITE CREEK RELOCATION
12 - 14	PAVING, DRAINAGE & GRADING PLANS
15	PROFILE & TYPICAL SECTION, STREAM RELOCATION
16	ROADWAY PROFILE
17	STORM DRAIN PROFILES & DETAILS
18 - 22	CHANNEL RELOCATION SEQUENCE AND PROTECTION OF STREAM FLOW PLANS
23 - 25	EROSION & SEDIMENT CONTROL PLANS
26 - 27	NOTES & DETAILS, SEDIMENT CONTROL AND PROTECTION OF STREAM FLOW
28	SUGGESTED CULVERT INSTALLATION SEQUENCE
29 - 30	PLAN AND PROFILE, GABION RETAINING WALL
31 - 32	GABION RETAINING WALL DETAILS & SECTIONS
33 - 34	GENERAL PLAN, ELEVATION & DETAILS, QUAD 10' X 12' BOX CULVERT EXTENSION
35	GENERAL PLAN, ELEVATION & DETAILS, TRIPLE 6'X 5' BOX CULVERT EXTENSION
36 - 37	PLANTING PLAN, NOTES & DETAILS
38 - 39	SANITARY SEWER AND WATER RELOCATIONS
40 - 42	PAVEMENT MARKING AND SIGNING PLANS

LIMIT OF CONSTRUCTION STA. 600+00, B/L CONSTR. POWHITE PARKWAY NB

**CONTRACTOR SHALL NOTIFY** "MISS UTILITY" @ 800-552-7001 **48 HOURS IN ADVANCE OF** ANY EXCAVATION ACTIVITY

#### CONVENTIONAL SIGN

STATE LINE COUNTY LINE CITY,TOWN OR VILLAGE RIGHT OF WAY LINE WATER LINE \_\_\_\_\_6V\_\_\_\_\_ SANITARY SEWER LINE \_\_\_\_\_8S\_\_\_\_\_ TRAVELED WAY GUARD RAIL RETAINING WALL RAILROADS ..... BASE OR SURVEY LINE

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POWER PO	LES		
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HEDGE		 000	
HEAVY WO	ODS	 · • •	
GROUND E	방법에 지난 것을 만들었다. 이 가지 않는 것이 같아. 것이 같아.		

FOR ADDITIONAL SYMBOLS AND SIGNS, SEE LEGEND ON SHEET 2

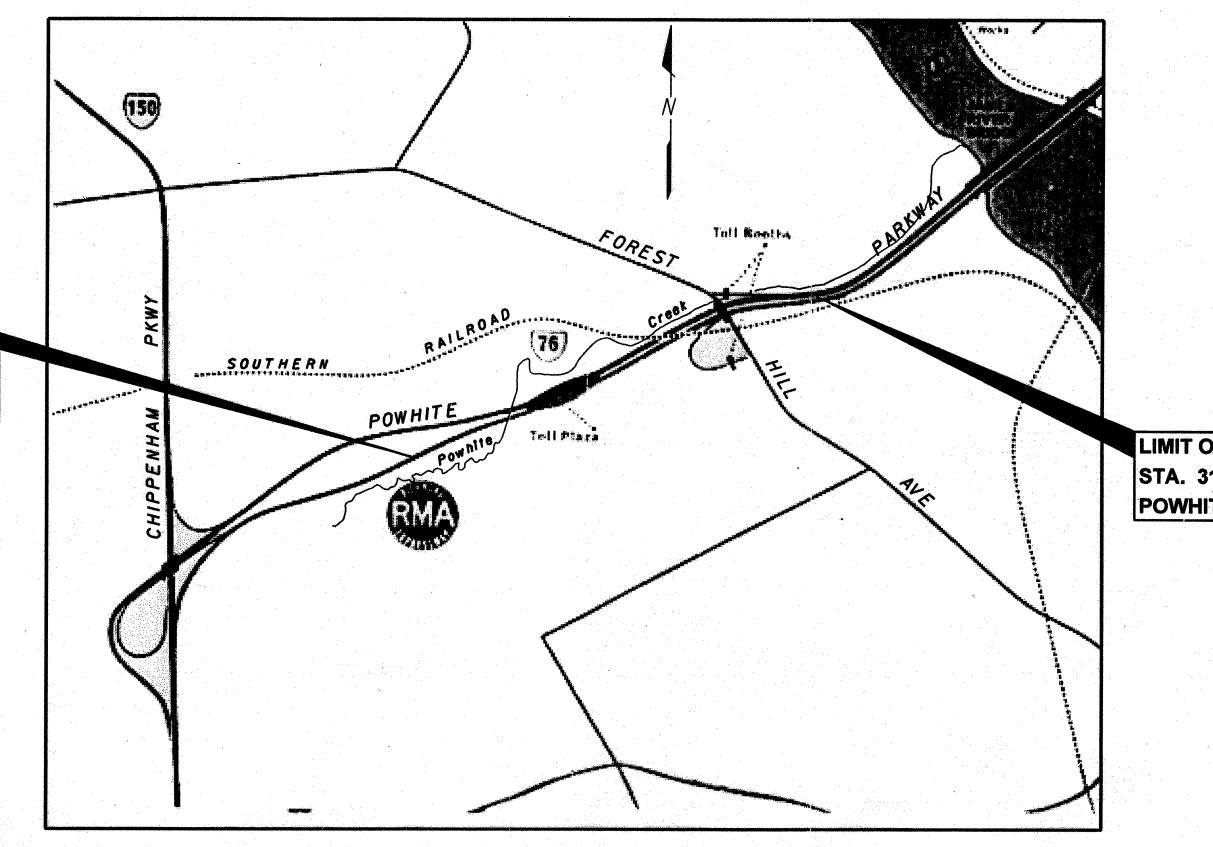
## RICHMOND

METROPOLITAN AUTHORITY

## RICHMOND EXPRESSWAY SYSTEM

## EXPRESS TOLL PROGRAM

# EAST OF CHIPPENHAM PARKWAY TO FOREST HILL AVE. POWHITE CREEK AND PAVING PROJECT

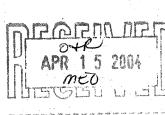


LOCATION MAP SCALE

0 500' 1000'

**CONTRACT PCPP - 2004** 

2000'

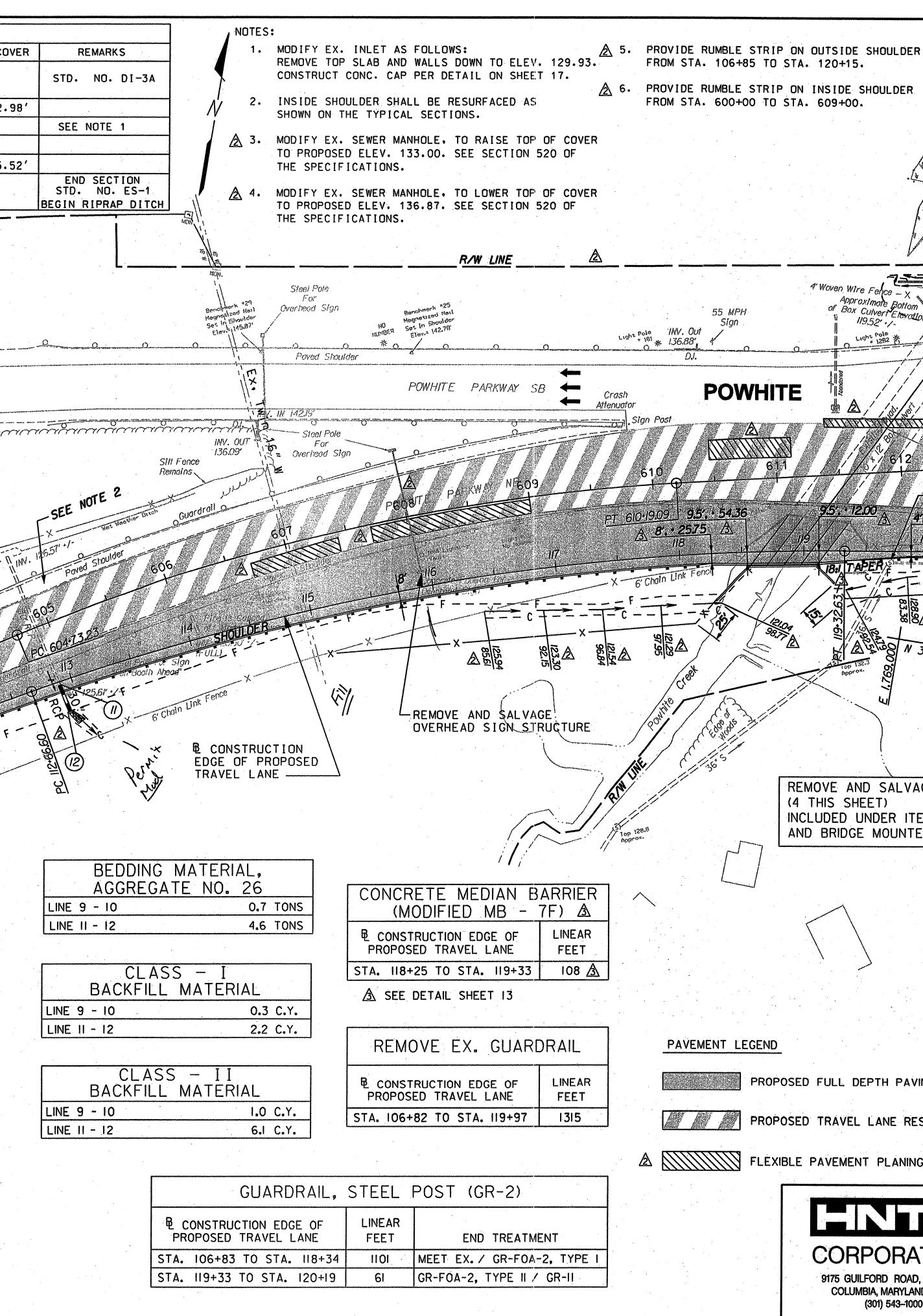


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	SHEET	NDUM NO. 3: APRIL I IS - 1, 6, 11, 12, 13, 23,27,28,33,38,39,	14, 16, 18,		
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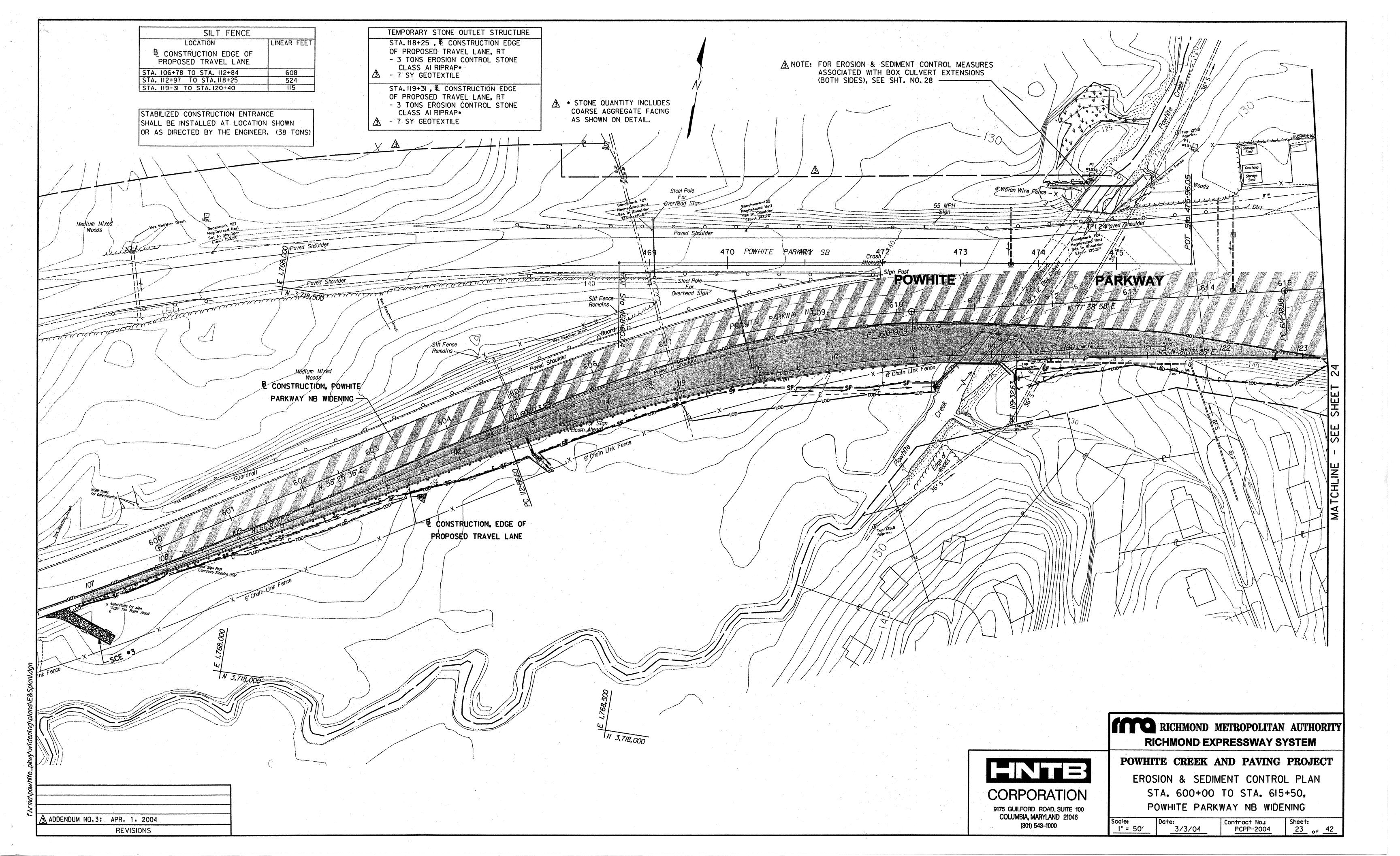
**HNTB** CORPORATION 9175 GUILFORD ROAD, SUITE 100 COLUMBIA, MARYLAND 21046 (301) 543-1000

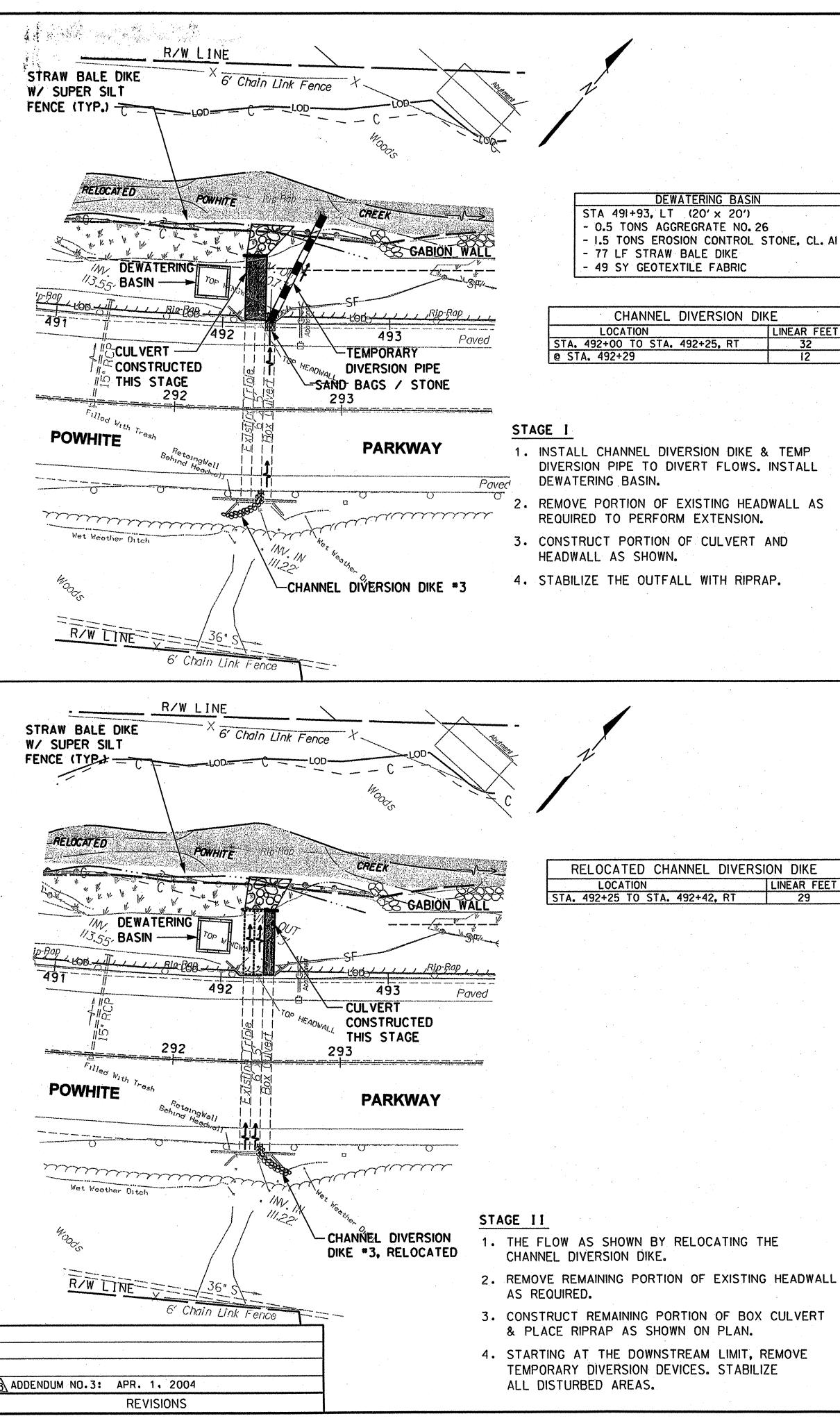
3-4-04 Date	HARDE M. Molu HNTB CORPORATION general consultant	:l
	RECOMMENDED BY	
<b>3 - 5 - 01</b> Date	GENERAL MANAGER, RICHMOND METROPOLITAN AUTHORIT	
	Plans Revised	
Sheet N	0.	Date
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CTD M	1 T K 1 P**			TABULATIONS	T	101
STR. No.	LINE	LOCATION STA 122+65, & CONST.	DESCRIPTION	INV. TO INV.	TOP	'D' OR 'H'
9		EDGE OF PROPOSED TRAVEL LANE	STANDARD DROP INLET		132.07'	4.23'
	9 - 10		6' - 15" RCP	127.90' - 127.84'		
10		122+65, 3.75' LT	EX. INLET		129.93'	
11		112+88, 50.68' RT	EX. END OF PIPE		133.82'	8.21'
	11 - 12		23' - 30" RCP	125.61' - 125.41'		
12		112+89, 73.32' RT	STANDARD CONC. END SECTION			
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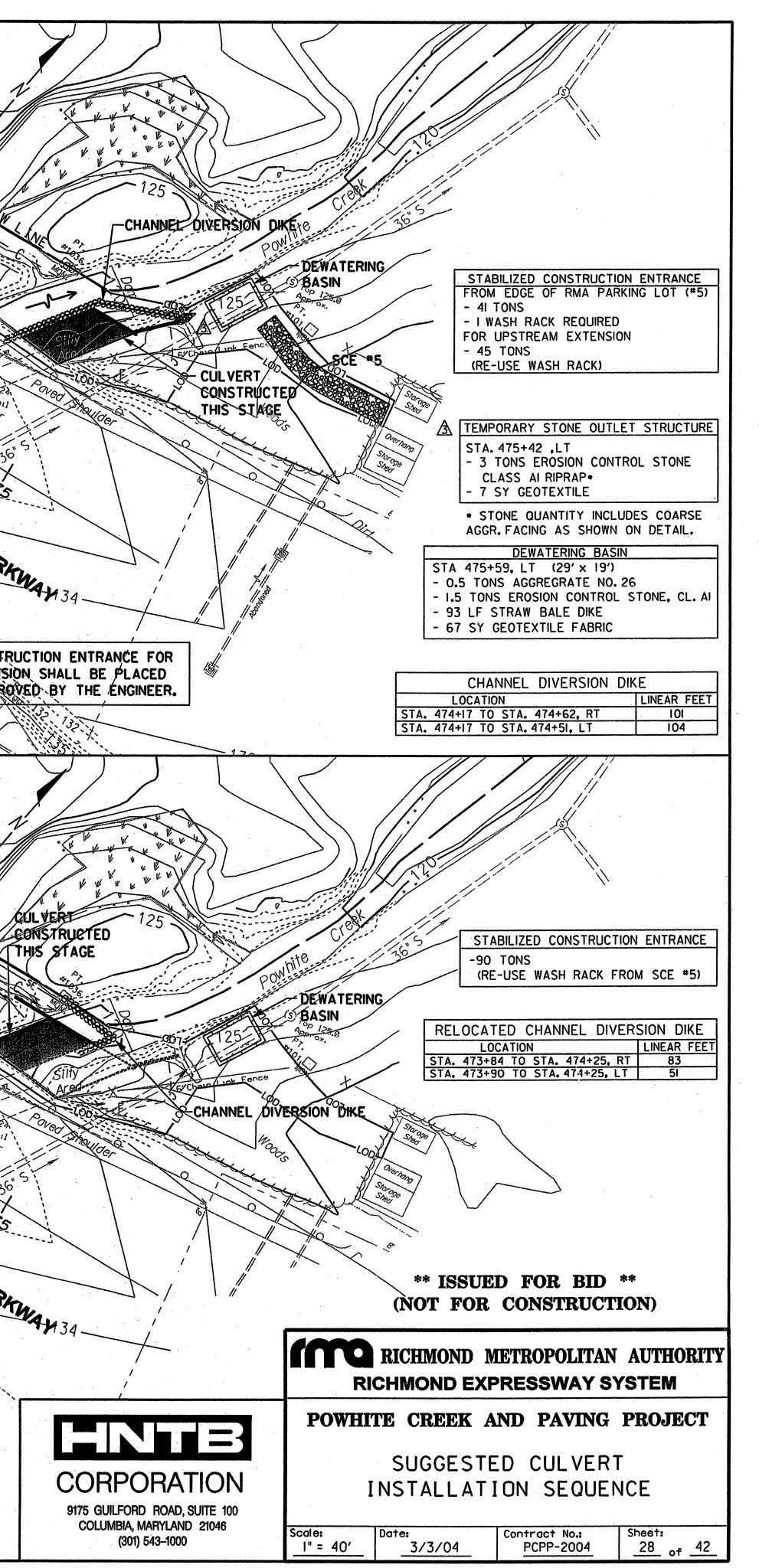


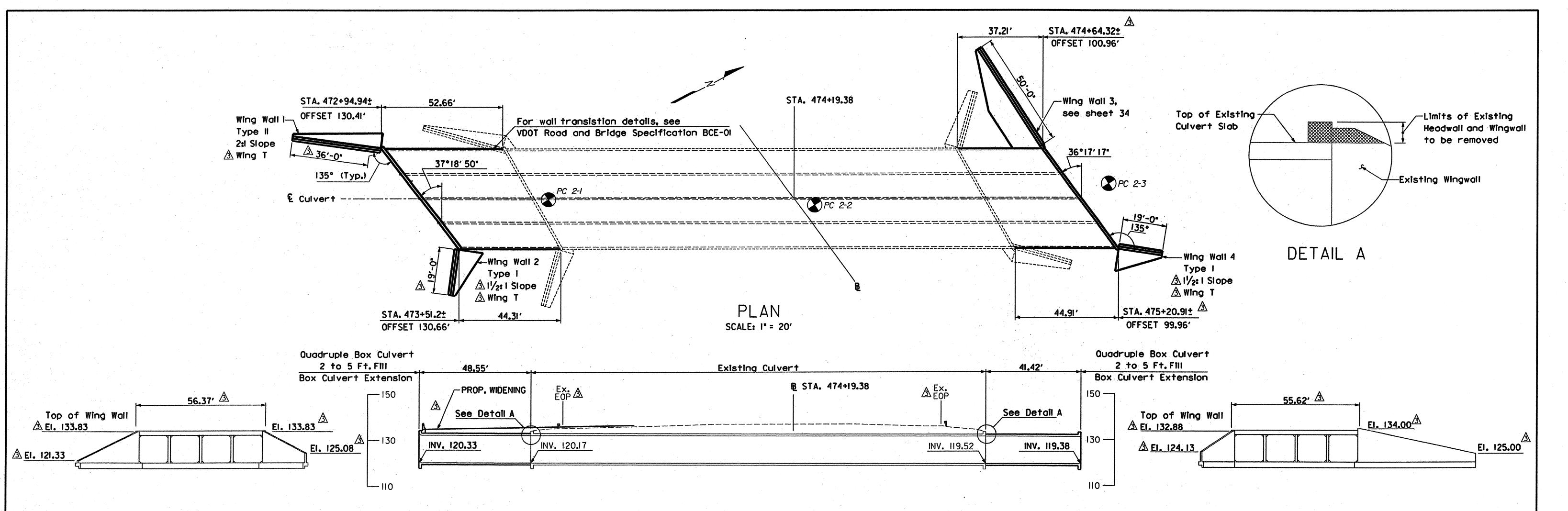
A 6. PROVIDE RUMBLE STRIP ON INSIDE SHOULDER R/W LINE FOLLOWS CENTERLINE OF CREEK to tester de stander de stander de Sioroga Sigd Overhong Stor 603 Shed 4 Woven Wire Feh Approximo of Box Culver 119.52 . THAT LIAZ Light pole PARKWA a source and the sour 15" RCP N\_81:131 26" E 122 S 123 NOSE DOWN CU 🔪 IN 2 FT. N 3,718,500 A SEE NOTE 4 Culvert. A  $\geq$ REMOVE AND SALVAGE LIGHT POLE (4 THIS SHEET) \* INCLUDED UNDER ITEM "HIGHWAY LIGHTING AND BRIDGE MOUNTED SIGN LIGHTING" PROPOSED FULL DEPTH PAVING CONFORMED SET PROPOSED TRAVEL LANE RESURFACING RICHMOND METROPOLITAN AUTHORITY A FLEXIBLE PAVEMENT PLANING AREA **RICHMOND EXPRESSWAY SYSTEM POWHITE CREEK AND PAVING PROJECT** HNTB PAVEMENT, GRADING & DRAINAGE PLAN STA. 600+00 TO STA. 615+50, CORPORATION POWHITE PARKWAY NB WIDENING 9175 GUILFORD ROAD, SUITE 100 COLUMBIA, MARYLAND 21046 Contract No.: PCPP-2004 Scale: Date: (301) 543-1000 1" = 50' 3/3/04





SILT FENCE STAGE I LINEAR FEET LOCATION STA. 473+07 TO STA. 474+17.RT 127 1. INSTALL CHANNEL DIVERSION DIKES TO DIVERT FLOW AS SHOWN. INSTALL DEWATERING BASIN. TEMPORARY SWALE A-2 A 2. CONSTRUCT TEMPORARY SWALE TO CHANNEL SURFACE LINEAR FEET LOCATION RUN-OFF AROUND CULVERT EXTENSION SITE. STA. 473+07 TO STA. 474+17, RT 127 ▲ 3. PLACE SILT FENCE AND TEMPORARY STONE OUTLET STRUCTURE AS SHOWN ON THE PLAN. A 4. REMOVE PORTION OF EXISTING HEADWALL AS REQUIRED TO PERFORM EXTENSIONS. A 5. CONSTRUCT PORTIONS OF CULVERT AND HEADWALL AS SHOWN. OWHITE LINEAR FEET 32 CHANNEL DIVERSION DIKE T'S PARKWAY 34-CULVERT TEMPORARY SWALE-CONSTRUCTED STABILIZED CONSTRUCTION ENTRANCE FOR THIS STAGE UPSTREAM EXTENSION SHALL BE PLACED IN LOCATION APPROVED BY THE ENGINEER. SILT FENCE STAGE II LOCATION LINEAR FEET STA. 469+78 TO STA. 474+09,RT 31 1. REDIVERT THE FLOW AS SHOWN BY STA. 474+25 TO STA. 474+72,LT 47 RELOCATING THE CHANNEL DIVERSION DIKES. STA. 472+69 TO STA. 473+18,LT | 113 A2. CONSTRUCT TEMPORARY SWALE TO CHANNEL SURFACE RUN-OFF AROUND CULVERT EXTENSION A TEMPORARY STONE OUTLET STRUCTURE STA. 474+17 ,LT SITE. SP. 3 TONS EROSION CONTROL STONE A 3. PLACE SILT FENCE AND TEMPORARY STONE CLASS AI RIPRAP\* OUTLET STRUCTURE AS SHOWN ON THE PLAN. - 7 SY GEOTEXTILE A4. REMOVE REMAINING PORTION OF EXISTING \* STONE QUANTITY INCLUDES COARSE HEADWALLS AS REQUIRED. AGGR. FACING AS SHOWN ON DETAIL. LINEAR FEET TEMPORARY SWALE A-2 ▲ 5. CONSTRUCT REMAINING PORTION OF BOX CULVERT AS SHOWN ON PLAN. LOCATION LINEAR FEI STA. 472+69 TO STA. 473+18,RT 113 AG. STARTING AT THE DOWNSTREAM LIMIT, REMOVE TEMPORARY PERIMETER CONTROLS. STABILIZE -138POWHITE ALL DISTURBED AREAS. CULVERT CONSTRUCTED THIS STAGE 7.5 TEMPORARY\ SWALE Silty Area. CHANNEL DIVERSION DIKE PARKWAY 34-3,0 STABILIZED CONSTRUCTION 0 ENTRANCES FOR THIS WORK SHALL BE PLACED IN LOCATIONS APPROVED BY THE ENGINEER.

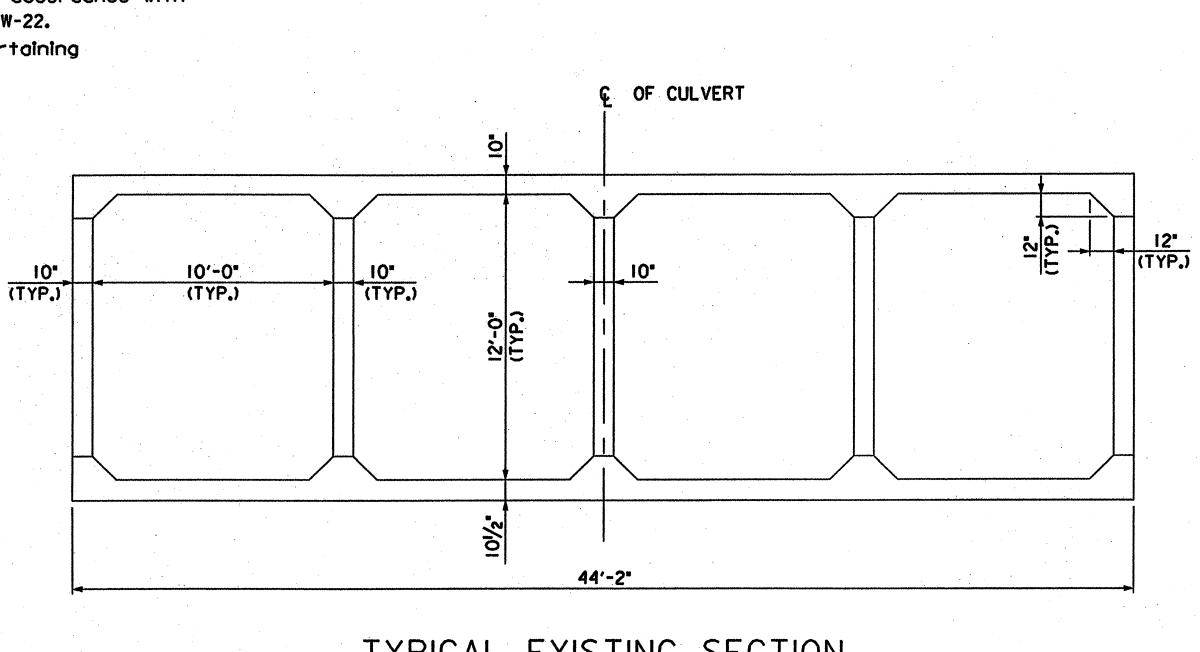




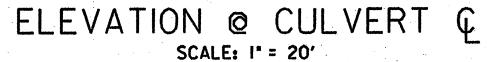
#### Notes:

- I. Box Culvert Extension materials and construction shall be in accordance with VDOT Road and Bridge Standards, BCO-5 10'x12'.
- 2. Wingwall materials and construction shall be in accordance with VDOT Road and Bridge Standards, BCW-II and BCW-22.
- 3. Contractor shall field verify all dimensions pertaining to existing box culvert.

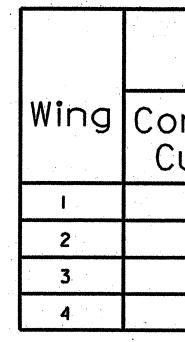
ntities xtension		
690 C.Y.		
390 C.Y.		
72.850 LBS.		
287 C.Y.		
630 TONS		



ADDENDUM NO.3: APR. 25, 2004 REVISIONS









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	ted Quantities Wing Wall
oncrete Cu.yd.	Reinforcing Steel Lbs.
22 👌	I,953 <u>A</u>
17 🛕	1,150 🖄
56	5,281
17 🛕	1,150 🛕

#### \*\* ISSUED FOR BID \*\* (NOT FOR CONSTRUCTION)

RICHMOND METROPOLITAN AUTHORITY **RICHMOND EXPRESSWAY SYSTEM POWHITE CREEK AND PAVING PROJECT** HNTB VDOT CULVERT NO. 1831 QUADRUPLE BOX CULVERT EXTENSION CORPORATION PLAN & ELEVATION 9175 GUILFORD ROAD, SUITE 100 COLUMBIA, MARYLAND 21046 (301) 543-1000 Scale: Sheet: Date: Contract No.: PCPP-2004 <u>33</u> of <u>42</u> 3/3/04 AS SHOWN